FINAL REPORT

## US 60 | US 70 | US 191 Corridor Profile Study

Apache Junction to Douglas
.......

MPD 022-21

## US 60 | US 70 | US 191 CORRIDOR PROFILE STUDY

Apache Junction to Douglas

ADOT WORK TASK NO. MPD-029-16
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| ACRONYMS \& ABBREVIATIONS |  | MPD | Multimodal Planning Division |
| :---: | :---: | :---: | :---: |
| AADT | Average Annual Daily Traffic | NACOG | Northern Arizona Council of Governments |
| ABISS | Arizona Bridge Information and Storage System | NB | Northbound |
| ADOT | Arizona Department of Transportation | NPV | Net Present Value |
| AGFD | Arizona Game and Fish Department | OP | Overpass |
| ASLD | Arizona State Land Department | P2P | Planning to Programming |
| AZTDM | Arizona Travel Demand Model | PA | Project Assessment |
| BLM | Bureau of Land Management | PARA | Planning Assistance for Rural Areas |
| BLM | Bureau of Land Management | PDI | Pavement Distress Index |
| BQAZ | Building a Quality Arizona | PES | Performance Effectiveness Score |
| CAG | Central Arizona Governments | PSR | Pavement Serviceability Rating |
| CCTV | Closed Circuit Television | RTP | Regional Transportation Plan |
| CPS | Corridor Profile Study | RWIS | Road Weather Information System |
| CR | Cracking Rating | SATS | Small Area Transportation Study |
| CYMPO | Central Yavapai Metropolitan Planning Organization | SB | Southbound |
| DCR | Design Concept Report | SEAGO | South Eastern Arizona Governments Organization |
| DMS | Dynamic Message Sign | SHSP | Strategic Highway Safety Plan |
| EB | Eastbound | SOV | Single Occupancy Vehicle |
| FHWA | Federal Highway Administration | SR | State Route |
| FY | Fiscal Year | SVMPO | Sierra Vista Metropolitan Planning Organization |
| HCRS | Highway Condition Reporting System | SWAP | State Wildlife Action Plan |
| HERE | Real time traffic conditions database produced by American Digital Cartography Inc. | TAC | Technical Advisory Committee |
| HPMS | Highway Performance Monitoring System | TI | Traffic Interchange |
| 1 | Interstate | TIP | Transportation Improvement Plan |
| IRI | International Roughness Index | TTTR | Truck Travel Time Reliability |
| ITS | Intelligent Transportation System | UP | Underpass |
| LCCA | Life-Cycle Cost Analysis | UPRR | Union Pacific Railroad |
| LOS | Level of Service | USDOT | United States Department of Transportation |
| LOTTR | Level of Travel Time Reliability | V/C | Volume to Capacity Ratio <br> Vehicle-Miles Travelled |
| LPOE | Land Point of Entry | VPD | Vehicles Per Day |
| LRTP | Long Range Transportation Plan | WB | Westbound |
| MAG | Maricopa Association of Governments | WIM | Weigh-in-Motion |
| MAP-21 | Moving Ahead for Progress in the 21st Century |  |  |
| MP | Milepost |  |  |



## EXECUTIVE SUMMARY

## INTRODUCTION

The Arizona Department of Transportation (ADOT) is the lead agency for this Corridor Profile Study (CPS) of US Route 60|US 70 from Apache Junction to the US 191 Junction and of US 191 from US 70 to the SR 80 Junction (US 60|US 70|US 191). This study examines key performance measures relative to the US 60|US 70|US 191 corridor, and the results of this performance evaluation are used to identify potential strategic improvements. The intent of the corridor profile program, and of ADOT's Planning-to-Programming (P2P) process, is to conduct performance-based planning to identify areas of need and make the most efficient use of available funding to provide an efficient transportation network.
ADOT has completed 21 original CPS within four separate groupings or rounds. In 2020, ADOT separated the previously studied corridors into six groupings to be updated and reassessed. The 60|US 70|US 191 corridor, depicted in Figure ES-1 along with all CPS corridors, is one of the strategic statewide corridors identified and the subject of this CPS Update

## Corridor Study Purpose, Goals and Objectives

The purpose of the CPS is to measure corridor performance to inform the development of strategic solutions that are cost-effective and account for potential risks. This purpose can be accomplished by following the process described below:

- Inventory past improvement recommendations
- Define corridor goals and objectives
- Assess existing performance based on quantifiable performance measures
- Propose various solutions to improve corridor performance
- Identify specific solutions that can provide quantifiable benefits relative to the performance measures
- Prioritize solutions for future implementation, accounting for performance effectiveness and risk analysis findings
The objective of the US 60|US 70|US 191 CPS is to identify a recommended set of prioritized potential solutions for consideration in future construction programs, derived from a transparent, defensible, logical, and replicable process. The US 60|US 70 UUS 191 CPS defines solutions and improvements for the corridor that are evaluated and ranked to determine which investments offer the greatest benefit to the corridor in terms of enhancing performance.

The following goals are identified as the outcome of this study:

- Link project decision-making and investments on key corridors to strategic goals
- Develop solutions that address identified corridor needs based on measured performance
- Prioritize improvements that cost-effectively preserve, modernize, and expand transportation infrastructure


Figure ES-1: Corridor Study Area

## Study Location and Corridor Segments

The US 60|US 70|US 191 CPS divides the corridor into twenty planning segments to facilitate analysis and evaluation. The corridor is segmented at logical breaks where the context changes due to differences in characteristics such as terrain, daily traffic volumes, or roadway typical sections. Corridor segments are shown in Figure ES-2.

## Figure ES-2: Corridor Location and Segments



## CORRIDOR PERFORMANCE

A series of performance measures are used to assess the US 60|US 70|US 191 corridor. The results of the performance evaluation are used to define corridor needs relative to the long-term goals and objectives for the corridor.

## Corridor Performance Framework

This study uses a performance-based process to define baseline corridor performance diagnose corridor needs, develop corridor solutions, and prioritize strategic corridor investments In support of this objective, a framework for the performance-based process was developed through a collaborative process involving ADOT and the CPS consultant teams.
Figure ES-3 illustrates the performance framework, which includes a two-tiered system of performance measures (primary and secondary) to evaluate baseline performance.


The following five performance areas guide the performance-based corridor analyses

- Pavement
- Bridge
- Mobility
- Safety
- Freight

The performance measures include five primary measures: Pavement Index, Bridge Index, Mobility Index, Safety Index, and Freight Index. Additionally, a set of secondary performance measures provides for a more detailed analysis of corridor performance. Table ES-1 provides the complete list of primary and secondary performance measures for each of the five performance areas.

## Table ES-1: Corridor Performance Measures

| Performance Area | Primary Measure | Secondary Measures |
| :---: | :---: | :---: |
| Pavement | Pavement Index <br> Based on a combination of International Roughness Index, cracking, and rutting | - Directional Pavement Serviceability <br> - Pavement Failure <br> - Pavement Hot Spots |
| Bridge | Bridge Index <br> Based on lowest of deck, substructure, superstructure and structural evaluation rating | - Bridge Sufficiency <br> - Bridge Rating <br> - Bridge Hot Spots |
| Mobility | Mobility Index <br> Based on combination of existing and future daily volume-to-capacity ratios | - Future Congestion <br> - Peak Congestion <br> - Travel Time Reliability <br> - Multimodal Opportunities |
| Safety | Safety Index <br> Based on frequency of fatal and incapacitating injury crashes | - Directional Safety Index <br> - Strategic Traffic Safety Plan Emphasis Areas <br> - Other Crash Unit Types <br> - Safety Hot Spots |
| Freight | Freight Index <br> Based on bi-directional truck planning time index | - Travel Time Reliability <br> - Bridge Vertical Clearance <br> - Bridge Vertical Clearance Hot Spots |

Each of the primary and secondary performance measures identified in the table above is comprised of one or more quantifiable indicators. A three-level scale was developed to standardize the performance scale across the five performance areas, with numerical thresholds specific to each performance measure

| Good/Above Average Performance | Rating is above the identified desirable/average range |
| :--- | :--- |
| Fair/Average Performance | Rating is within the identified desirable/average range |
| Poor/Below Average Performance | Rating is below the identified desirable/average range |

The terms "good", "fair", and "poor" apply to the Pavement, Bridge, Mobility, and Freight performance measures, which have defined thresholds. The terms "above average", "average", and "below average" apply to the Safety performance measures, which have thresholds referenced to statewide averages.

## Corridor Performance Summary

Table ES-2 shows a summary of corridor performance for all primary measures and secondary measure indicators for the US 60|US 70|US 191 corridor. A weighted corridor average rating (based on the length of the segment) was calculated for each primary and secondary measure as shown in Table ES-2

The five areas evaluated are split between "good" (40\%), "fair" (35\%), and "poor" (25\%) ratings The poorest performing segment is 60-14 which rates as "poor" in mobility, safety and freight, and "fair" in pavement \& mobility. The highest performing segment, 60-18, has only one performance metric that has "poor" performance which is Percent Area Failure

- Pavement Performance: The weighted average of the Pavement Index shows "fair" overall performance for the US 60|US 70|US 191 corridor; Segments 191-1,191-2,191-3,191-4,191-6,70-6, 70-7,70-8,70-9,70-11,70-12,70-13 and 60-18 shows "poor" or "fair performance for several Pavement performance areas. Due to the significant areas of roughness and pavement cracking, 17 of the 20 segments rate poorly for percentage o area in failure.
- Bridge Performance: The weighted average of the Bridge Index shows "fair" overal performance for the US 60|US 70|US 191 corridor; Segments 70|60-13 and 60E-14 show "fair" or "poor" performance for all Bridge performance area measures; the Lowest Bridge Rating measures shows "poor" performance; the weighted average for the Sufficiency Rating measure shows "good" performance; Segments 191-5, and 70-9 contain no bridges.
- Mobility Performance: The weighted average of the Mobility Index shows "good" overal performance for the US 60|US 70|US 191 corridor; the Future Daily V/C and Existing Peak Hour V/C measures show generally "good" performance for all segments along the corridor with exceptions of segments 60E-14,60E-19 and 60E-20, which show "poor" performance; the Directional Closure Extent measure show generally "good" or "fair" performance with exceptions of segments 60E-14 and 60E-20 which show "poor" performance ; the Directional LOTTR measure shows generally "good' or "fair" performance for all segments along the corridor; the weighted average for the Directional LOTTR measure shows "fair" in both the NB/WB and SB/EB direction; Segments 191-3 191-5, 191-6, 70-8 through 60E-14, and 60E-19 show "poor" performance for the \% Bicycle Accommodation measure and the weighted average for the corridor shows "fair" performance
- Safety Performance: The weighted average of the Safety Index and Directional Safety Indices show "fair" performance for the US 60|US 70|US 191 corridor; The crash unit type performance measures for crashes involving intersections, Pedestrians, Trucks and Bicycles had insufficient data to generate reliable performance ratings; The weighted average of the crash unit type performance measure involving Lane departures shows
"above average" performance; The Safety Index value for Segments 70-10 through 60E$14,60 \mathrm{E}-17$ and 60E-20 are "below average", meaning these segments have more crashes than the typical statewide for a similar operating environment; The Directional Safety Index value for NB/WB travel for Segments 70-11 through 60E-14, 60E-17, 60E19 and 60E-20 are "below average" and for SB/EB travel, 70-10 and 70-12 through 60E14 are "below average"
- Freight Performance: The weighted average of the Freight Index shows "poor" overal performance for US 60|US 70|US 191 corridor; All segments show "fair" or "poor" performance for the Freight Index and Directional Max TTTR (for NB/WB travel) measures except for Segment 60E-16; Directional Max TTTR in the SB/EB direction shows "fair" or "poor" for all segments except for segments 60E-17 and 60E-19 which show "good" performance. Segment 60E-14 in the SB/EB direction shows "poor" performance in the Closure Duration performance measure; Most of the segments show "fair" or "good" performance for the Closure Duration performance measure; two bridge vertical clearance hot spots exist in Segments 70/60-13 and 60E-14.

Table ES-2: Corridor Performance Summary by Segment and Performance Measure

| Segment \# | Segment Length (miles) | Pavement Performance Area |  |  |  | Bridge Performance Area |  |  | Mobility Performance Area |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Pavement Index | Directional PSR |  | \% Area Failure | Bridge Index | Sufficiency Rating | Lowest <br> Bridge <br> Rating | Mobility Index | Future Daily V/C | Existing Peak Hour V/C |  | Closure Extent (instances/ milepost/year/mile) |  | Directional Max LOTTR (all vehicles) |  | \% Bicycle <br> Accommodation | \% NonSingle Occupancy Vehicle (SOV) Trips |
|  |  |  | NB/EB | SB/WB |  |  |  |  |  |  | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB |  |  |
| 191-12* | 24 | 3.17 | 3.10 | 3.24 | 70.8\% | 6.0 | 87.80 | 6 | 0.16 | 0.18 | 0.13 | 0.13 | 0.04 | 0.02 | 1.40 | 1.39 | 66\% | 15.0\% |
| 191-2 ${ }^{2 *}$ | 43 | 2.89 | 3.44 | 3.38 | 55.8\% | 5.4 | 69.23 | 5 | 0.13 | 0.17 | 0.08 | 0.11 | 0.03 | 0.01 | Insu | Data | 100\% | 16.6\% |
| 191-3 ${ }^{\text {2 }}$ | 17 | 3.42 | 3.63 | 3.69 | 72.0\% | 5.5 | 93.81 | 5 | 0.05 | 0.05 | 0.03 | 0.03 | 0.02 | 0.00 | Insu | Data | 49\% | 8.8\% |
| 191-4 ${ }^{\text {^ }}$ | 12 | 3.44 | 3.29 | 3.32 | 41.7\% | 6.0 | 69.50 | 6 | 0.17 | 0.19 | 0.11 | 0.11 | 0.08 | 0.07 | Insu | Data | 97\% | 8.3\% |
| 191-5 ${ }^{\text {* }}$ | 5 | 3.10 | 3.16 | 3.07 | 80.0\% | No Bridges |  |  | 0.27 | 0.30 | 0.15 | 0.16 | 0.20 | 0.20 |  |  | 0.20 | 21.2\% |
| 70-6 ${ }^{1 *}$ | 9 | 3.23 | 3.15 | 3.25 | 60.0\% | 6.0 | 68.10 | 6 | 0.41 | 0.45 | 0.31 | 0.29 | 0.02 | 0.04 | Insu | Data | 46\% | 17.8\% |
| 70-7 ${ }^{\text {2 }}$ | 30 | 2.83 | 2.87 | 3.08 | 86.8\% | 5.7 | 70.25 | 5 | 0.18 | 0.20 | 0.11 | 0.10 | 0.04 | 0.01 | Insu | Data | 73\% | 15.8\% |
| $70-8^{\text {2 }}$ | 2 | 2.59 | 3.35 | 3.67 | 100.0\% | 6.0 | 73.00 | 6 | 0.11 | 0.12 | 0.08 | 0.05 | 0.10 | 0.00 | Insu | Data | 0\% | 12.8\% |
| $70-9^{\wedge}$ | 5 | 2.71 | 3.44 | 3.63 | 100.0\% | No Bridges |  |  | 0.24 | 0.26 | 0.16 | 0.12 | 0.04 | 0.04 | 0.04 |  | 0.04 | 11.2\% |
| $70-10^{2^{\wedge}}$ | 19 | 2.69 | 3.10 | 3.35 | 78.9\% | 7.0 | 80.00 | 7 | 0.15 | 0.17 | 0.11 | 0.08 | 0.07 | 0.05 | Insufficient Data |  | 4\% | 7.7\% |
| $70-11^{\text {2 }}$ | 4 | 2.40 | 3.27 | 3.28 | 87.5\% | 6.7 | 82.02 | 5 | 0.18 | 0.20 | 0.13 | 0.10 | 0.00 | 0.00 | Insufficient Data |  | 4\% | 11.3\% |
| $70-12^{2^{\wedge}}$ | 15 | 3.57 | 3.28 | 3.53 | 33.3\% | 6.0 | 52.90 | 6 | 0.24 | 0.27 | 0.16 | 0.17 | 0.17 | 0.00 | Insufficient Data |  | 23\% | 12.5\% |
| 70\|60-13 ${ }^{1 *}$ | 12 | 3.28 | 3.13 | 3.28 | 53.8\% | 5.2 | 78.01 | 4 | 0.40 | 0.45 | 0.26 | 0.25 | 0.22 | 0.35 | 1.16 | 1.15 | 54\% | 16.6\% |
| 60E-14* | 16 | 3.68 | 3.66 | 3.82 | 43.8\% | 5.5 | 68.13 | 3 | 1.42 | 1.71 | 0.79 | 1.14 | 0.67 | 1.84 | 1.12 | 1.17 | 49\% | 14.0\% |
| $60 \mathrm{E}-15^{2 \wedge}$ | 2 | 4.03 | 3.70 | 3.65 | 0.0\% | 6.3 | 84.08 | 6 | 2.80 | 3.90 | 1.13 | 1.12 | 0.00 | 0.90 | 1.18 | 1.14 | 95\% | 10.5\% |
| 60E-16 ${ }^{\text {2 }}$ | 2 | 4.50 | 4.22 | 4.15 | 0.0\% | 5.0 | 86.43 | 5 | 0.73 | 1.01 | 0.42 | 0.42 | 0.60 | 0.15 | 1.05 | 1.12 | 87\% | 7.7\% |
| $60 \mathrm{E}-17^{\text {2 }}$ | 11 | 3.51 | 3.93 | 3.99 | 76.2\% | 6.6 | 95.57 | 5 | 0.26 | 0.37 | 0.15 | 0.14 | 0.04 | 0.23 | 1.05 | 1.09 | 96\% | 8.9\% |
| $60 \mathrm{E}-18^{2^{\text {A }}}$ | 7 | 3.30 | 3.62 | 3.83 | 92.9\% | 5.9 | 90.24 | 5 | 0.53 | 0.66 | 0.30 | 0.32 | 0.00 | 0.23 | 1.12 | 1.05 | 100\% | 12.0\% |
| $60 \mathrm{E}-19^{1 *}$ | 6 | 3.57 | 3.57 | 3.65 | 33.3\% | 5.9 | 91.43 | 5 | 1.01 | 0.86 | 0.86 | 0.91 | 0.10 | 0.30 | 1.20 | 1.14 | 42\% | 17.8\% |
| $60 \mathrm{E}-20^{1 \wedge}$ | 5 | 4.17 | 3.87 | 3.83 | 0.0\% | 6.0 | 93.95 | 6 | 1.31 | 1.45 | 0.84 | 0.88 | 0.68 | 0.09 | 1.06 | 1.06 | 100\% | 17.2\% |
| Weighted Ave | Corridor ge | 3.18 | 3.33 | 3.44 | 63\% | 5.82 | 81.95 | 4.87 | 0.34 | 0.40 | 0.22 | 0.24 | 0.12 | 0.19 | 0.19 | 1.20 | 63\% | 13.7\% |
| SCALE |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Performa | ce Level | Non-Interstate |  |  |  | All |  |  | Urban Rural ${ }^{2}$ |  |  |  | All |  | Uninterrupted ^ Interrupted * |  |  | All |
| Good / Abo | Average | > 3.50 |  |  | < $5 \%$ | > 6.5 | $>80$ | > 6 | $\begin{aligned} & \leq 0.71 \text { (Urban) } \\ & \leq 0.56 \text { (Rural) } \end{aligned}$ |  |  |  | $<0.22$ |  | $\begin{aligned} & \leq 1.15 \\ & \leq 1.15 \end{aligned}$ |  | > 90\% | > 17\% |
| Fair / Average |  | 2.9-3.5 |  |  | 5\%-20\% | 5.0-6.5 | 50-80 | 5-6 | $0.56-0.76$ (Rural) |  |  |  | 0.22-0.62 |  | $\begin{array}{r} 1.15-1.5 \\ 1.15-1.5 \\ \hline \end{array}$ |  | 90\% - 60\% | 17\% - 11\% |
| Poor / Average |  | <2.90 |  |  | > 20\% | < 5.0 | < 50 | < 5 | $\begin{aligned} & >0.89 \text { (Urban) } \\ & >0.76 \text { (Rural) } \\ & \hline \end{aligned}$ |  |  |  | $\geq 0.62$ |  | $\begin{aligned} & \geq 1.5 \\ & \geq 1.5 \\ & \hline \end{aligned}$ |  | < 60\% | < 11\% |

[^0]Table ES-2: Corridor Performance Summary by Segment and Performance Measure (continued)

| $\underset{\#}{\text { Segment }}$ | Segment Length (miles) | Safety Performance Area |  |  |  |  |  |  |  | Freight Performance Area |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Safety Index | Directional Safety Index |  | \% of Fatal + Suspected Serious Injury Crashes at Intersections | \% of Fatal + <br> Suspected Serious Injury Crashes Involving Lane Departures | \% of Fatal + Suspected Serious Injury Crashes Involving Pedestrians | \% of Segment <br> Fatal + <br> Suspected <br> Serious Injury <br> Crashes <br> Involving Trucks | \% of Segment <br> Fatal + <br> Suspected Serious Injury Crashes Involving Bicycles | Freight TTTR | Directional Max TTTR |  | Combined Average Peak TTTR | Average Minutes Per Year Given Milepost Is Closed Per Segment Mile (NB/EB) |  | Bridge <br> Vertical <br> Clearance (feet) |
|  |  |  | NB/EB | SB/WB |  |  |  |  |  |  | NB/EB | $\begin{gathered} \text { SB/W } \\ \text { B } \end{gathered}$ |  | NB/EB | SB/WB |  |
| $191-1^{2^{*}}$ | 24 | 0.39 | 0.04 | 0.73 | $\begin{aligned} & \hline \text { Insufficient } \\ & \text { Data } \end{aligned}$ | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 2.26 | 2.52 | 2.00 | 2.26 | 3.02 | 1.00 | No UP |
| 191-2 ${ }^{2 *}$ | 43 | 0.49 | 0.54 | 0.44 | Insufficient Data | Insufficient Data | Insufficient Data | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | Insufficient Data | Insuffic Da |  | Insufficient Data | 2.67 | 1.78 | 22.04 |
| $191-3^{2^{\wedge}}$ | 17 | 0.59 | 0.00 | 1.18 | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | Insufficient Data | Insuffi Da |  | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | 2.47 | 0.00 | No UP |
| $191-4^{2^{\wedge}}$ | 12 | 0.58 | 1.06 | 0.11 | Insufficient Data | Insufficient Data | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | Insufficient Data | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | Insufficient Data | Insuffi Da |  | Insufficient Data | 12.23 | 5.00 | No UP |
| 191-5** | 5 | 0.06 | 0.12 | 0.00 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insuffi Da |  | Insufficient Data | 26.08 | 16.96 | None |
| $70-6{ }^{1 *}$ | 9 | 0.38 | 0.67 | 0.08 | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | 25\% | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insuffi Da |  | Insufficient Data | 1.33 | 4.67 | No UP |
| 70-7 ${ }^{\text {2 }}$ | 30 | 1.08 | 1.41 | 0.75 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insuffi D |  | Insufficient Data | 4.55 | 5.40 | 17.03 |
| $70-8^{\text {2 }}$ | 2 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | Insufficient Data | Insuffi Da |  | Insufficient Data | 14.30 | 0.00 | No UP |
| $70-9^{\text {2 }}$ | 5 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insuffi Da |  | Insufficient Data | 2.40 | 3.00 | None |
| $70-10^{2 \wedge}$ | 19 | 1.63 | 0.76 | 2.50 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insuffi Da |  | Insufficient Data | 8.63 | 2.51 | No UP |
| $70-11^{\text {^ }}$ | 4 | 3.37 | 6.74 | 0.00 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insuffi Da |  | Insufficient Data | 0.00 | 0.00 | No UP |
| $70-12^{2^{\wedge}}$ | 15 | 2.63 | 2.97 | 2.28 | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | 22\% | $\begin{gathered} \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | Insufficient Data | Insuffi Da |  | Insufficient Data | 17.39 | 0.00 | No UP |
| $\begin{gathered} 70 \mid 60- \\ 13^{1^{*}} \end{gathered}$ | 12 | 2.97 | 3.36 | 2.57 | Insufficient Data | 21\% | Insufficient Data | Insufficient Data | Insufficient Data | 1.58 | 1.67 | 1.49 | 1.58 | 22.75 | 26.52 | 15.84 |
| $\begin{aligned} & 60 \mathrm{E}- \\ & 14^{2^{\wedge}} \\ & \hline \end{aligned}$ | 16 | 1.78 | 1.50 | 2.07 | Insufficient Data | 81\% | Insufficient Data | Insufficient Data | Insufficient Data | 1.49 | 1.52 | 1.46 | 1.49 | 63.60 | 344.95 | 13.03 |
| $\begin{aligned} & \hline 60 \mathrm{E}- \\ & 15^{2^{\wedge}} \\ & \hline \end{aligned}$ | 2 | Insufficient Data | Insufficient Data | $\begin{gathered} \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | $\begin{gathered} \hline \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | Insufficient Data | 1.32 | 1.34 | 1.29 | 1.32 | 0.00 | 90.50 | 16.79 |
| $\begin{aligned} & 60 \mathrm{E}- \\ & 16^{2^{\wedge}} \\ & \hline \end{aligned}$ | 2 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 1.28 | 1.14 | 1.42 | 1.28 | 52.20 | 12.25 | No UP |

a 2 or 3 Lane Undivided
${ }^{\mathrm{b}} 2,3$ or 4 Lane Divided
${ }^{\circ} 4$ or 5 Lane Undivided
${ }^{\wedge}$ Uninterrupted

* Interrupted

Note: "Insufficient Data" indicates there was not enough data available to generate reliable performance ratings
"No UP" indicates no underpasses are present in the segment

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Table ES-2: Corridor Performance Summary by Segment and Performance Measure (continued)

| $\underset{\#}{\text { Segment }}$ | Segment Length (miles) | Safety Performance Area |  |  |  |  |  |  |  | Freight Performance Area |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Safety Index | Directional Safety Index |  | \% of Fatal + Suspected Serious Injury Crashes at Intersections | \% of Fatal + Suspected Serious Injury Crashes Involving Lane Departures | \% of Fatal + Suspected Serious Injury Crashes Involving Pedestrians | \% of Segment <br> Fatal + <br> Suspected Serious Injury Crashes Involving Trucks | \% of Segment <br> Fatal + <br> Suspected Serious Injury Crashes Involving Bicycles | Freight TTTR | Directional Max TTTR |  | Combined Average Peak TTTR | Average Minutes Per Year Given Milepost Is Closed Per Segment Mile (NB/EB) |  | Bridge Vertical Clearance (feet) |
|  |  |  | NB/EB | SB/WB |  |  |  |  |  |  | NB/EB | SB/WB |  | NBIEB | SB/WB |  |
| $\begin{aligned} & \hline 60 \mathrm{E}- \\ & 17^{2^{\wedge}} \\ & \hline \end{aligned}$ | 11 | 1.23 | 1.82 | 0.65 | $\begin{gathered} \hline \text { Insufficient } \\ \text { Data } \end{gathered}$ | 78\% | Insufficient Data | $\begin{gathered} \hline \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | Insufficient Data | 1.18 | 1.15 | 1.20 | 1.18 | 3.27 | 61.40 | No UP |
| $\begin{aligned} & 60 \mathrm{E}- \\ & 18^{2^{\wedge}} \\ & \hline \end{aligned}$ | 7 | 0.50 | 0.91 | 0.09 | Insufficient Data | 17\% | Insufficient Data | Insufficient Data | Insufficient Data | 1.22 | 1.32 | 1.13 | 1.22 | 0.00 | 22.29 | No UP |
| $\begin{aligned} & \hline 60 \mathrm{E}- \\ & 19^{1 *} \\ & \hline \end{aligned}$ | 6 | 0.95 | 1.62 | 0.27 | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | 60\% | $\begin{gathered} \hline \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | Insufficient Data | 1.63 | 1.74 | 1.52 | 1.63 | 14.00 | 20.30 | No UP |
| $\begin{aligned} & \hline 60 \mathrm{E}- \\ & 20^{1 \wedge} \\ & \hline \end{aligned}$ | 5 | 1.29 | 1.89 | 0.69 | Insufficient Data | 50\% | Insufficient Data | $\begin{gathered} \hline \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | Insufficient Data | 1.20 | 1.25 | 1.14 | 1.20 | 74.94 | 7.11 | No UP |
| Weig Corrid Ave | dod | 1.11 | 1.19 | 1.03 | Insufficient Data | 45\% | Insufficient Data | Insufficient Data | Insufficient Data | 1.64 | 1.75 | 1.54 | 1.64 | 12.16 | 30.69 | 18.90 |
| SCALES |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Perfor Le | mance el | 2 or 3 or 4 Lane Divided Highway 2 or 3 Undivided Highway 4 or 5 Undivided Highway Urban 4 Lane Freeway |  |  |  |  |  |  |  | Uninterrupted |  |  |  | All |  |  |
| Good Ave Perfor | Above age mance |  | $\begin{aligned} & <0.81 \\ & <0.92 \\ & <0.78 \\ & <0.73 \end{aligned}$ |  | $\begin{aligned} & <23.4 \% \\ & <11.2 \% \\ & <43.8 \% \\ & <0.0 \% \end{aligned}$ | $\begin{aligned} & <56.4 \% \\ & <66.9 \% \\ & <21.1 \% \\ & <60.6 \% \\ & \hline \end{aligned}$ | $\begin{aligned} & <16 \% \\ & <3.8 \% \\ & <8.8 \% \\ & <0.0 \% \end{aligned}$ | $\begin{aligned} & <3.7 \% \\ & <4.2 \% \\ & <0.8 \% \\ & <6.9 \% \end{aligned}$ | $\begin{aligned} & \hline<0 \% \\ & <0 \% \\ & <0.5 \% \\ & <0 \% \end{aligned}$ | < 1.15 |  |  |  | < 44.18 |  | > 16.5 |
| Fair/Average Performance |  |  | $\begin{gathered} \hline 0.81-1.19 \\ 0.92-1.08 \\ 0.78-1.22 \\ 0.73-1.27 \\ \hline \end{gathered}$ |  | $\begin{gathered} \hline 23.4 \%-29.3 \% \\ 11.2 \%-15.6 \% \\ 43.8 \%-49.5 \% \\ 0.0 \%-0.0 \% \\ \hline \end{gathered}$ | $\begin{gathered} 56.4 \%-65.0 \% \\ 66.9 \%-74.5 \% \\ 21.1 \%-32.1 \% \\ 60.6 \%-78.1 \% \\ \hline \end{gathered}$ | $\begin{gathered} 16 \%-26 \% \\ 3.8 \%-7.2 \% \\ 8.8 \%-13.5 \% \\ 0.0 \%-4.9 \% \\ \hline \end{gathered}$ | $\begin{gathered} 3.7 \%-9.9 \% \\ 4.2 \%-8.0 \% \\ 0.8 \%-5.5 \% \\ 6.9 \%-12.4 \% \\ \hline \end{gathered}$ | $\begin{gathered} \hline 0 \%-2 \% \\ 0 \%-3.3 \% \\ 0.5 \%-3.8 \% \\ 0.0 \%-0.0 \% \\ \hline \end{gathered}$ | 1.15-1.35 |  |  |  | 44.18-124.86 |  | $\begin{gathered} 16.0- \\ 16.5 \end{gathered}$ |
| Poor Ave Perfor | Below age mance |  | $\begin{array}{r} >1.19 \\ >1.08 \\ >1.22 \\ >1.27 \end{array}$ |  | $\begin{aligned} & >29.3 \% \\ & >15.6 \% \\ & >49.5 \% \\ & >0.0 \% \end{aligned}$ | > 65.0\% | $\begin{gathered} >26 \% \\ >7.2 \% \\ >13.5 \% \\ >4.9 \% \end{gathered}$ | $\begin{gathered} 9.90 \% \\ >8.0 \% \\ >5.5 \% \\ >12.4 \% \end{gathered}$ | $\begin{gathered} >2 \% \\ >3.3 \% \\ >3.8 \% \\ >0.0 \% \end{gathered}$ | > 1.35 |  |  |  | > 124.86 |  | < 16.0 |

b 2,3 or 4 Lane Divided
${ }^{\circ} 4$ or 5 Lane Undivided
${ }^{\wedge}$ Uninterrupted

* Interrupted

Note: "Insufficient Data" indicates there was not enough data available to generate reliable performance ratings "No UP" indicates no underpasses are present in the segment

## NEEDS ASSESSMENT

## Corridor Description

The US 60 UUS 70 UUS 191 corridor links the Mexico border at the City of Douglas and the Phoenix metropolitan area to agricultural, mining and recreational activity in southeastern Arizona. In general, all three highways are two-lane facilities designed for relatively modest traffic volumes in a rural setting. At the same time, the corridor offers some unique benefits within the Arizona circulation system that could be leveraged for increased usage as the need arises.
US 191 provides a link between Mexico and Interstate 10 (I-10), the primary east-west interstate corridor along the southern states. As a result, US 191 serves as a major freight corridor for goods moving between Mexico and the United States. Similarly, the combination of US 191 and US 70 between I-10 and Globe offers a critical connection to mining and agricultural interests located in the greater Safford and Globe areas of Graham and Pinal Counties. US 60 between Globe and SR 79 links activities within the corridor to the major population and commerce center of the Phoenix metropolitan area.

## Corridor Objectives

Statewide goals and performance measures were established by the ADOT Long-Range Transportation Plan (LRTP), 2010-2035. Statewide performance goals that are relevant to US 60|US $70 \mid$ US 191 performance areas were identified and corridor goals were then formulated for each of the five performance areas that aligned with the overall statewide goals established by the LRTP. Based on stakeholder input, corridor goals, corridor objectives, and performance results, three "emphasis areas" were identified for the US 60|US 70|US 191 corridor: Mobility, Safety and Freight.

Taking into account the corridor goals and identified emphasis areas, performance objectives were developed for each quantifiable performance measure that identify the desired level of performance based on the performance scale levels for the overall corridor and for each segment of the corridor For the performance emphasis areas, the corridor-wide weighted average performance objectives are identified with a higher standard than for the other performance areas.
Achieving corridor and segment performance objectives will help ensure that investments are targeted toward improvements that support the safe and efficient movement of travelers on the corridor. Corridor performance is measured against corridor and segment objectives to determine needs - the gap between observed performance and the performance objectives.

## Needs Assessment Process

The performance-based needs assessment evaluates the difference between the baseline performance and the performance objectives for each of the five performance areas used to characterize the health of the corridor: Pavement, Bridge, Mobility, Safety, and Freight. The performance-based needs assessment process is illustrated in Figure ES-4

The needs assessment compares baseline corridor performance with performance objectives to provide a starting point for the identification of performance needs. This mathematical comparison results in an initial need rating of None, Low, Medium, or High for each primary and secondary performance measure. An illustrative example of this process is shown in Figure ES-5.
The initial level of need for each segment is refined to account for hot spots and recently completed or under construction projects, resulting in a final level of need for each segment. The final levels of need for each primary and secondary performance measure are combined to produce a weighted final need rating for each segment. A detailed review of available data helps identify contributing factors to the need and if there is a high level of historical investment.

Figure ES-4: Needs Assessment Process

|  | STEP 1 | STEP 2 | STEP 3 | STEP 4 | STEP 5 |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Initial Need Identification | Need Refinement | Contributing Factors | Segment Review | Corridor Needs |
| $\begin{aligned} & \frac{2}{0} \\ & \frac{1}{5} \\ & \frac{8}{4} \end{aligned}$ | Compare results of performance baseline to performance objectives to identify initial performance need | Refine initial performance need based on recently completed projects and hotspots | Perform "drill-down" investigation of refined need to confirm need and to identify contributing factors | Summarize need on each segment | Identify overlapping, common, and contrasting contributing factors |
| 告 | Initial levels of need (none, low, medium, high) by performance area and segment | Refined needs by performance area and segment | Confirmed needs and contributing factors by performance area and segment | Numeric level of need for each segment | Actionable performance-based needs defined by location |

Figure ES-5: Initial Need Ratings in Relation to Baseline Performance (Bridge Example)

| Performance Thresholds | Performance Level | Initial Level of Need | Description |
| :---: | :---: | :---: | :---: |
| 6.5 | Good | None* | All levels of Good and top $1 / 3$ of Fair (>6.0) |
|  | Good |  |  |
|  | Good |  |  |
|  | Fair |  |  |
|  | Fair | Low | Middle 1/3 of Fair (5.5-6.0) |
| 5.0 | Fair | Medium | Lower 1/3 of Fair and top 1/3 of Poor (4.5-5.5) |
|  | Poor |  |  |
|  | Poor | High | Lower 2/3 of Poor (<4.5) |
|  | Poor |  |  | *A segment need rating of 'None' does not indicate a lack of needed improvements; rather, it indicates that the segment performance

score exceeds the established performance thresholds and strategic solutions for that segment will not be developed as part of this study.

Summary of Needs
Table ES-3 provides a summary of needs for each segment across all performance areas, and the average needs for each segment. A weighting factor of 1.5 is applied to the average need scores of the performance areas identified as emphasis areas (mobility, safety, and freight for the US 60|US 70|US 191 corridor). There are 10 segments with a high average need, seven segments with a medium average need, and 31 segments with a low average need. More information on the identified final needs in each performance area is provided below.

## Pavement Needs

- Seventeen segments (191-1 through 70|60-14 and 60-17 through 60-19) contain Pavement hot spots
- Segments 60-15, 60-16 and 60-20 have a final need of None, Segments 191-3, 191-4, 70-12, 60-14, 60-17 and 60-19 have a final need of Low, Segments 191-1, 191-5, 70-6, 70|60-13 and 60-18 have a final need of Medium, and Segments 191-2, and 70-7 through 70-11 show a final level need of High
- A high level of historical investment has occurred on Segments 191-3 and 70-9 and a medium level of historical investment has occurred through Segments 191-2, 191-4,191-5, 70-8, 7010 , and $70-12$ through 60-17


## Bridge Needs

- Bridge needs were identified on four segments of the corridor, 73 miles ( $30 \%$ ) with a "Medium" level of bridge need.
- Seven bridges showed potential repetitive investment issues and may be candidates for lifecycle cost analysis to evaluate alternative solutions.
- Three bridges have bridge ratings of 4: Pinal Creek Bridge (No. 266), Waterfall Canyon Bridge (No. 328), and Queen Creek Bridge (No. 406).
- Eleven bridges were defined as hot spots since they had multiple bridge ratings of 5 or less or one bridge rating of 4
- Of the eleven hot spot bridges, six also showed repetitive investment issues. These included the Holyoak Wash Bridge (No. 514), Pinal Creek Bridge (No. 549), Pinal Creek Bridge (No. 36), Pinal Creek Bridge (No. 266), Waterfall Canyon Bridge (No. 328) and Queen Creek Bridge (No. 406)
Mobility Needs
- Mobility Performance is an Emphasis Area for the US 60| US 70| US 191 corridor, giving it a heavier weight in the analysis
- Segments 60-14, 60-19 and 60-20 have a final segment need of High; all other segments on the corridor have a final segment need of Low or None
- There is lack of bicycle accommodation along $60 \%$ of the corridor
- Mobility needs are primarily due to mobility, future $\mathrm{v} / \mathrm{c}$, and existing $\mathrm{v} / \mathrm{c}$ issues Safety Needs
- Safety Performance is an Emphasis Area for the US 60| US $70 \mid$ US 191 corridor, giving it a heavier weight in the analysis.
- Segments 70-10 through 60-14, 60-17, 60-19 and 60-20 have a final segment need of Medium or High; all other segments on the corridor have a final segment of Low or None
- Safety hot spots exist in Segments 70-6, 60|70-13, 60-14, and 60-17 through 60-20
- There is insufficient data to generate reliable ratings for the secondary measures including crashes at intersections, involving pedestrians, involving trucks, or involving bicycles
Freight Needs
- Freight Performance is an Emphasis Area for the US 60| US 70| US 191 corridor, giving it a heavier weight in the analysis.
- Segments 191-1, 60-14 and 60-15 have a final segment need of Medium or High; all other segments on the corridor have a final segment need of Low or None
- Freight needs are primarily due to Freight TTTR, Closure Duration and Bridge Clearance
- There are two freight hot spots along the corridor: Pinal SR UP and Queen Creek Tunnel Overlapping Needs
Completing projects that address multiple needs presents the opportunity to more effectively improve overall performance. A summary of the overlapping needs that relate to locations with elevated levels of need is provided below.
- All segments on the corridor have overlapping needs. Traffic counters do not exist in Segments 191-2 through 70-12, approximately 161 miles or $66 \%$ of the corridor, resulting in insufficient data to calculate needs in the freight performance area for those locations.
- US $60 \mid 70$ MP 243 to MP 255 (Segment 70|60-13) and US 60 MP 227 to MP 243 (Segment 6014) have overlapping needs in all five performance areas. These segments comprised 28 of the 246 corridor miles.
- Segment $70 \mid 60-13$ has an overall Medium need score on the corridor. Some needs are site specific while others are characteristics of the segment. Medium bridge needs are related to the Bloody Tanks Bridge (No. 173), Pinal Creek Bridge (No. 36), Pinal Creek Bridge (No. 266), Pinal Creek Bridge (No. 549) and McMillen Wash Bridge (No. 1028) which are hot spots due to poor structural ratings and exhibit high repetitive investment. High safety needs are due to the number of fatal or suspected serious injury collisions exceeding the statewide average which are due to failure to yield right-of-way and involve vehicles running off the road (left). Low freight needs are due to the bridge vertical clearance for the Pinal SPRR UP (No. 0562)
- Segment 60-14 has an overall High need and the highest need score in the corridor. This segment has significant grades and subsequently suffers from freight, safety and mobility needs related to delay and incidents/accidents associated with the grade along with speeding too fast for conditions. The segment includes 2 hot spot bridges, both which do not have repetitive investment histories. The Queen Creek Tunnel, also located in the segment, affects bridge and freight needs with low vertical clearance.
- Segments 60-19 registers an overall Medium need score on the corridor with overlapping mobility and safety needs. Medium safety needs are due to the number of fatal or suspected serious injury collisions exceeding the statewide average which are due to dark-lighted conditions and involve vehicles running of the road (left). High mobility needs are due to poor bicycle accommodation and poor existing peak hour volume-to-capacity ratio.

Table ES-3: Summary of Needs by Segment

| Performance Area | Segment Number and Mileposts (MP) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 191-1 | 191-2 | 191-3 | 191-4 | 191-5 | 70-6 | 70-7 | 70-8 | 70-9 | 70-10 | 70-11 | 70-12 | 70\|60-13 | 60-14 | 60-15 | 60-16 | 60-17 | 60-18 | 60-19 | 60-20 |
|  | $\begin{aligned} & \text { MP } \\ & 0-24 \end{aligned}$ | $\begin{gathered} \text { MP } \\ 24-67 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 87-104 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 104-116 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 116-121 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 339-330 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 330-300 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 300-298 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 298-293 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 293-274 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 274-270 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 270-255 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 255-243 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 243-227 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 227-225 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 225-223 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 223-212 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 212-205 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 205-199 \end{gathered}$ | $\begin{gathered} \hline \text { MP } \\ 199- \\ 194.3 \\ \hline \end{gathered}$ |
| Pavement | Medium | High | Low | Low | Medium | Medium | High | High | High | High | High | Low | Medium | Low | None* | None* | Low | Medium | Low | None* |
| Bridge | None* | Medium | Low | Low | N/A | Low | Low | None* | N/A | None* | Low | Low | Medium | Medium | None* | Medium | Low | Low | Low | None* |
| Mobility+ | Low | None* | Low | None* | Low | Low | Low | Low | Low | Low | Low | Low | Low | High | Low | Low | None* | Low | High | High |
| Safety+ | None* | None* | Low | Low | None* | Low | None* | N/A\# | N/A\# | High | High | High | High | High | N/A\# | N/A\# | High | Low | Medium | Medium |
| Freight+ | High | N/A ${ }^{\text {\# }}$ | N/A ${ }^{\#}$ | N/A ${ }^{\text {\# }}$ | N/A ${ }^{\text {\# }}$ | N/A ${ }^{\text {\# }}$ | N/A ${ }^{\text {\# }}$ | N/A ${ }^{\text {\# }}$ | N/A ${ }^{\#}$ | N/A ${ }^{\text {\# }}$ | N/A ${ }^{\text {\# }}$ | N/A ${ }^{\#}$ | Low | High | Medium | Low | None* | Low | Low | Low |
| Average Need | 1.23 | 0.77 | 0.77 | 0.54 | 0.54 | 0.92 | 0.85 | 0.69 | 0.69 | 1.38 | 1.54 | 1.23 | 1.77 | 2.54 | 0.69 | 0.77 | 1.00 | 1.15 | 1.69 | 1.38 |

*A segment need rating of 'None' does not indicate a lack of needed improvements; rather, it indicates that the segment performance score exceeds the established performance
thresholds and strategic solutions for that segment will not be developed as part of this study.

+ Identified as an emphasis area for the US 60|US 70|US 191 corridor
\# N/A indicates insufficient or no data available to determine level of need

| Average Need Scale |  |
| :---: | :---: |
| None $^{\star}$ | $<0$ |
| Low | $0.1-1.0$ |
| Medium | $1.0-2.0$ |
| High | $>2.0$ |

## STRATEGIC SOLUTIONS

The principal objective of the CPS is to identify strategic solutions (investments) that are performance-based to ensure that available funding resources are used to maximize the performance of the State's key transportation corridors. One of the first steps in the development of strategic solutions is to identify areas of elevated levels of need as addressing these needs will have the greatest effect on corridor performance. Segments with Medium or High needs and specific locations of hot spots are considered strategic investment areas for which strategic solutions should be developed. Segments with lower levels of need or without identified hot spots are not considered candidates for strategic investment and are expected to be addressed through other ADOT programming processes. US 60|US 70|US 191 strategic investment areas (resulting from the elevated needs) are shown in Figure ES-6

## Screening Process

In some cases, needs that are identified do not advance to solutions development and are screened out from further consideration because they have been or will be addressed through other measures including:

- A project is programmed to address this need
- The need is a result of a Pavement or Bridge hot spot that does not show historical investment or rating issues; these hot spots will likely be addressed through other ADOT programming means
- A bridge is not a hot spot but is located within a segment with a Medium or High level of need; this bridge will likely be addressed through current ADOT bridge maintenance and preservation programming processes
- The need is determined to be non-actionable (i.e., cannot be addressed through an ADOT project)
- The conditions/characteristics of the location have changed since the performance data was collected that was used to identify the need


## Candidate Solutions

For each elevated need within a strategic investment area that is not screened out, a candidate solution is developed to address the identified need. Each candidate solution is assigned to one of the following three P2P investment categories based on the scope of the solution:

- Preservation
- Modernization
- Expansion

Documented performance needs serve as the foundation for developing candidate solutions for corridor preservation, modernization, and expansion. Candidate solutions are not intended to be a substitute or replacement for traditional ADOT project development processes where various ADOT technical groups and districts develop candidate projects for consideration in the performance-based programming in the P2P process. Rather, these candidate solutions are intended to complement ADOT's traditional project development processes through a performance-based process to address needs in one or more of the five performance areas of Pavement, Bridge, Mobility, Safety, and Freight. Candidate solutions developed for the US 60|US 70|US 191 corridor will be considered along with other candidate projects in the ADOT statewide programming process.

## Candidate solutions include some or all of the following characteristics:

- Do not recreate or replace results from normal programming processes
- May include programs or initiatives, areas for further study, and infrastructure projects
- Address elevated levels of need (High or Medium) and hot spots
- Focus on investments in modernization projects (to optimize current infrastructure)
- Address overlapping needs
- Reduce costly repetitive maintenance
- Extend operational life of system and delay expansion
- Leverage programmed projects that can be expanded to address other strategic elements
- Provide measurable benefit

Candidate solutions developed to address an elevated need in the Pavement or Bridge performance areas include two options; rehabilitation or full replacement. These solutions are initially evaluated through a Life-Cycle Cost Analysis (LCCA) to provide insights into the costeffectiveness of these options so a recommended approach can be identified. Candidate solutions developed to address an elevated need in the Mobility, Safety, or Freight performance areas are advanced directly to the Performance Effectiveness Evaluation. In some cases, there may be multiple solutions identified to address the same area of need

Candidate solutions that are recommended to expand or modify the scope of an already programmed project are noted and are not advanced to solution evaluation and prioritization These solutions are directly recommended for programming.

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## Figure ES-6: Strategic Investment Areas



## SOLUTION EVALUATION AND PRIORITIZATION

Candidate solutions are evaluated using the following steps: LCCA (where applicable), Performance Effectiveness Evaluation, Solution Risk Analysis, and Candidate Solution Prioritization. The methodology and approach to this evaluation are shown in Figure ES-7 and described more fully below.

## Life-Cycle Cost Analysis

All Pavement and Bridge candidate solutions have two options: rehabilitation/repair or reconstruction. These options are evaluated through an LCCA to determine the best approach for each location where a Pavement or Bridge solution is recommended. The LCCA eliminates options from further consideration and identify which options should be carried forward for further evaluation.

All Mobility, Safety, and Freight strategic investment areas that result in multiple independent candidate solutions are advanced directly to the Performance Effectiveness Evaluation.

## Performance Effectiveness Evaluation

After completing the LCCA process, all remaining candidate solutions are evaluated based on their performance effectiveness. This process includes determining a Performance Effectiveness Score (PES) based on how much each solution impacts the existing performance and needs scores for each segment. This evaluation also includes a Performance Area Risk Analysis to help differentiate between similar solutions based on factors that are not directly addressed in the performance system.

## Solution Risk Analysis

All candidate solutions advanced through the Performance Effectiveness Evaluation are also evaluated through a Solution Risk Analysis process. A solution risk probability and consequence analysis is conducted to develop a solution-level risk weighting factor. This risk analysis is a numeric scoring system to help address the risk of not implementing a solution based on the likelihood and severity of the performance failure.

## Candidate Solution Prioritization

The PES, weighted risk factor, and segment average need score are combined to create a prioritization score. The candidate solutions are sorted by prioritization score from highest to lowest. The highest prioritization score indicates the candidate solution that is recommended as the highest priority. Solutions that address multiple performance areas tend to score higher in this process.

Figure ES-7: Candidate Solution Evaluation Process


## SUMMARY OF CORRIDOR RECOMMENDATIONS

Table ES-4 and Figure ES-8 show the prioritized candidate solutions recommended for the US 60|US70|US 191 corridor. These solutions will increase the performance of the US 60|US70|US 191 corridor primarily in the Freight Performance Area. Solutions that address multiple performance areas tend to score higher in this process. Other findings include:

- Most of the anticipated improvements in performance are in the Mobility, Safety, and Freight performance areas
- The highest ranking solutions tended to have overlapping benefits in the Mobility, Safety, and Freight performance areas
- The highest priority solutions address needs in the US 60 Superior to Miami area


## Other Corridor Recommendations

As part of the investigation of strategic investment areas and candidate solutions, other corridor solutions were also identified that are compatible with the long range vision to increase safety and support truck and freight movements:

- Road Safety Assessments are recommended in Peridot, Cutter and Globe to identify safety improvements, specifically pedestrian circulation and access needs in Peridot.
- Access Control Studies in Peridot (MP 270 - 274) and Globe-Miami (MP 243-255) are recommended to reduce friction and improve safety
- Recommend Superior to Globe Design Concept Study
- Recommend San Carlos Area (MP 268 - 292) Superelevation Study


## Policy and Initiative Recommendations

In addition to location-specific needs, general corridor and system-wide needs have also been identified through the CPS process. While these needs are more overarching and cannot be individually evaluated through the CPS process, it is important to document them. A list of recommended policies and initiatives was developed for consideration when programming future projects not only on the SR 90/SR 80 corridor, but across the entire state highway system where conditions are applicable. The following list, which is in no particular order of priority, was derived from the four CPS rounds:

- Install Intelligent Transportation System (ITS) conduit with all new infrastructure projects
- Prepare strategic plans for Closed Circuit Television (CCTV) camera and Road Weather Information System (RWIS) locations statewide
- Leverage power and communication at existing weigh-in-motion (WIM), dynamic messaging signs (DMS), and call box locations to expand ITS applications across the state
- Consider solar power for lighting and ITS where applicable
- Investigate ice formation prediction technology where applicable
- Conduct highway safety manual evaluation for all future programmed projects
- Develop infrastructure maintenance and preservation plans (including schedule and funding) for all pavement and bridge infrastructure replacement or expansion projects
- Develop standardized bridge maintenance procedures so districts can do routine maintenance work
- Review historical ratings and level of previous investment during scoping of pavement and bridge projects; in pavement locations that warrant further investigation, conduct subsurface investigations during project scoping to determine if full replacement is warranted
- For pavement rehabilitation projects, enhance the amount/level of geotechnical investigations to address issues specific to the varying conditions along the project
- Expand programmed and future pavement projects as necessary to include shoulders
- Expand median cable barrier guidelines to account for safety performance
- Install CCTV cameras with all DMS
- In locations with limited communications, use CCTV cameras to provide still images rather than streaming video
- Develop statewide program for pavement replacement
- Install additional continuous permanent count stations along strategic corridors to enhance traffic count data
- When reconstruction or rehabilitation activities will affect existing bridge vertical clearance, the dimension of the new bridge vertical clearance should be a minimum of 16.25 feet where feasible
- All new or reconstructed roadway/shoulder edges adjacent to an unpaved surface should be constructed with a Safety Edge
- Collision data on tribal lands may be incomplete or inconsistent; additional coordination for data on tribal lands is recommended to ensure adequate reflection of safety issues
- Expand data collection devices statewide to measure freight delay
- Evaluate and accommodate potential changes in freight and goods movement trends that may result from improvements and expansions to the state roadway network
- At traffic interchanges with existing communication connectivity to the ADOT Traffic Operations Center, consideration should be given to adding thermal detection cameras for vehicle detection with the capability for wrong way vehicle detection
- Improved vehicle detection systems, as recommended by ADOT Systems Technology group, should be deployed at traffic interchanges for improved traffic control


## Next Steps

The candidate solutions recommended in this study are not intended to be a substitute or replacement for traditional ADOT project development processes where various ADOT technical groups and districts develop candidate projects for consideration in the performance-based programming in the P2P process. Rather, these candidate solutions are intended to complement ADOT's traditional project development processes through a performance-based process to address needs in one or more of the five performance areas of Pavement, Bridge, Mobility, Safety, and Freight. Candidate solutions developed for the US 60|US 70|US 191 corridor will be considered along with other candidate projects in the ADOT statewide programming process.

It is important to note that the candidate solutions are intended to represent strategic solutions to address existing performance needs related to the Pavement, Bridge, Mobility, Safety, and Freight performance areas. Therefore, the strategic solutions are not intended to preclude recommendations related to the ultimate vision for the corridor that may have been defined in the context of prior planning studies and/or design concept reports. Recommendations from such studies are still relevant to addressing the ultimate corridor objectives.

These results will be incorporated into a summary document comparing all corridors that is expected to provide a performance-based review of statewide needs and candidate solutions.

## CPS Program Refinements

This CPS assessment is an update to the originally produced CPS assessments conducted between 2017 and 2019. Due to changes in state and federal reporting standards as well as data availability, the original methodology has been adapted to produce comparable and relatable performance, need, and evaluation results. The methodology changes include:

- Pavement performance now includes the addition of rutting as a component of the Pavement Distress measure
- Bridge performance no longer includes the \% Functionally Obsolete secondary measure
- Safety performance includes updated secondary measure-categories; and is evaluated against updated statewide averages.
- Mobility and Freight performance are evaluated using updated reliability measures based on Level of Travel Time Reliability and Truck Travel Time Reliability, which are new federal standard measure adapted from the previous Travel Time Index and Planning Time Index measures

Table ES-4: Prioritized Recommended Solutions

| Rank | Candidate Solution \# | Option | Solution Name and Location | Description / Scope | Estimated Cost (in millions) | Investment Category (Preservation [P], Modernization [M], Expansion [E]) | Prioritization Score |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 60.14 | - | Apache Junction Area Safety Improvements | Install inside and edge line rumble strips through entire segment Consider installing speed feedback sign MP 195 | \$0.3 | M | 362 |
| 2 | 60.8 | - | Superior Area East Safety Improvements | Consider installing speed feedback signs at MP 229.9, MP 236, MP 241 Install centerline rumble strips at MP 229-231 <br> Install high visibility striping and delineators MP 228-228.3 and MP 241-242 <br> Install edge line rumble strips EB MP 228.17-228.3, MP 229.2-229.26, and MP 247247.26 | \$17.0 | M | 227 |
| 3 | 60.10 | - | Superior Area Safety Improvements | Install lighting at N Queen Valley Road and US 60 intersection Consider installing speed feedback sign MP 212.5 Install chevrons or curve warning sign at MP 219.33 | \$0.4 | M | 191 |
| 4 | 60.13 | - | Apache Junction Area Mobility Improvements | Add through lane in NB/WB direction | \$24.7 | M | 102 |
| 5 | 60.12 | - | Gold Canyon Area Mobility and Safety Improvements | Add SB/EB through lane MP 199.12 to 206 Consider installing speed feedback sign at MP 201 Widen shoulders MP 199.12 to 205 Install lighting MP 201-202 | \$44.0 | E | 101 |
| 6 | 60.9 | B | Superior Area East Freight Improvements | Reprofile mainline to increase vertical clearance | \$1.9 | E | 100 |
| 7 | 70\|60.6 | - | Globe Area Safety Improvements | Consider installing speed feedback signs (2 EB and 2 WB between MP 246-250) High visibility striping Install signal ahead warning signs with beacons in advance of SR 188 intersection Construct passing lane in each direction from MP 243-243.25 and MP 253.6-255 | \$22.6 | M | 44 |
| 8 | 70\|60.7 | B | Globe Area Freight Improvements | Reprofile mainline to increase vertical clearance | \$2.1 | E | 39 |
| 9 | 70.5 | - | East of Globe Safety Improvements | Widen shoulders MP 255-270 <br> Install centerline rumble strips MP 255-270 <br> Install improved lighting from milepost 269-270 <br> Construct passing lane in each direction (MP 255-256) <br> Improve existing pedestrian and speed warning signs to include flashing beacons and speed feedback signs (MP 269.25) | \$31.1 | M | 23 |
| 10 | 60.11 | - | US-60 SW of Gold Canyon Safety Improvements | Install lighting MP 205-207 <br> Widen inside shoulder 208.3-212 <br> Consider installing speed feedback sign | \$3.9 | M | 13 |

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Table ES-4: Prioritized Recommended Solutions (continued)

| Rank | Candidate Solution \# | $\begin{aligned} & \text { Optio } \\ & \mathrm{n} \end{aligned}$ | Solution Name and Location | Description / Scope | Estimated Cost (in millions) | Investment Category (Preservation [P], Modernization [M], Expansion [E]) | Prioritization Score |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 11 | 70\|60.7 | A | Globe Area Freight Improvements | Reconstruct Pinal SPRR UP to increase vertical clearance | \$8.2 | E | 4 |
| 12 | 70.2 | - | East Safford Safety Improvements | Provide flashing traffic signal warning signs at Milepost 337.82 and Milepost 338.03. Consider installing feedback signs in both directions at 20th Avenue | \$0.1 | M | 3 |
| 13 | 60.9 | A | Superior Area East Freight Improvements | Reconstruct Queen Creek Tunnel to increase vertical clearance | \$33.3 | E | 3 |
| 14 | 70.4 | - | Bylas to Peridot Safety Improvements | Widen shoulders Milepost 274-278 <br> Install centerline rumble strips MP 275.5-276.5,MP 279.5-287.5 <br> Install shoulder rumble strips MP 275.5-276.5,MP 279.5-287.5 <br> Install high visibility striping and delineators from milepost 274-278 <br> Improve existing pedestrian / speed warning signs to also include flashing beacons and speed feedback signs (MP 292,MP 280, MP 278.5) <br> Construct passing lanes (WB MP 288.2-289.6) <br> Formalize pullouts (signage, ROW for pullouts) (WB MP 274.5, EB MP 279, EB MP 289, WB 292) | \$15.1 | M | 2 |
| 15 | 191.1 | B | US191 Pavement Preservation South of Safford | Replace pavement | \$200.3 | M | 0 |

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Figure ES-8: Prioritized Recommended Solutions


Final Report

### 1.0 INTRODUCTION

The Arizona Department of Transportation (ADOT) is the lead agency for this Corridor Profile Study (CPS) of US Route (US) 60|US 70: State Route (SR) 79 to US 191 and US 191: US 70 to SR 80 (US 60|US 70|US 191). The study examines key performance measures relative to the US 60|US 70|US 191 corridor, and the results of this performance evaluation are used to identify potential strategic improvements. The intent of the corridor profile program, and of ADOT's Planning-to-Programming (P2P) process, is to conduct performance-based planning to identify areas of need and make the most efficient use of available funding to provide an efficient transportation network.

ADOT has completed 21 original CPS within four separate groupings or rounds. In 2020, ADOT separated the previously studied corridors into six groupings to be updated and reassessed: Northeast, Northcentral, Northwest, Southeast, Southcentral, and Southwest. The 8 corridor studies within the three southern groupings began in Spring 2022 and include:

## Southeast

- US 60: Meridian Road to US 70; US 70: US 60 to 191 (1 ${ }^{\text {ST }}$ Avenue); US 191: US 70 to I10 and SR 80 to l-10
- SR 90: SR 80 to I-10; SR 80: US 191 to SR 90


## Southcentral

- SR 347: SR 84 to Peters and Nall Road; SR 84: I-8 to SR 347
- I-10E: MP 187 to NM border
- I-19: Mexico border to I-10


## Southwest

- I-8: California border to I-10
- I-10W: California border to SR 85; SR 85: I-10 to I-8
- SR 95: I-10 to I-40; US 95: I-8 to I-10

The studies under this program assess the overall health, or performance, of the state's strategic highways. The CPS will identify candidate solutions for consideration in the Multimodal Planning Division's (MPD) P2P project prioritization process, providing information to guide corridorspecific project selection and programming decisions.

The US 60|US 70|US 191 corridor, depicted in Figure 1, along with all CPS corridors, is one of the strategic statewide corridors identified and the subject of this CPS Update.


### 1.1 Corridor Study Purpose

The purpose of the CPS is to measure corridor performance to inform the development of strategic solutions that are cost-effective and account for potential risks. This purpose can be accomplished by following the process described below:

- Inventory past improvement recommendations
- Define corridor goals and objectives
- Assess existing performance based on quantifiable performance measures
- Propose various solutions to improve corridor performance
- Identify specific solutions that can provide quantifiable benefits relative to the performance measures
- Prioritize solutions for future implementation, accounting for performance effectiveness and risk analysis findings


### 1.2 Study Goals and Objectives

The objective of this study is to identify a recommended set of prioritized potential solutions for consideration in future construction programs, derived from a transparent, defensible, logical, and replicable process. The US 60|US 70 UUS 191 CPS defines solutions and improvements for the corridor that are evaluated and ranked to determine which investments offer the greatest benefit to the corridor in terms of enhancing performance. Corridor benefits can be categorized by the following three investment types:

- Preservation: Activities that protect transportation infrastructure by sustaining asset condition or extending asset service life
- Modernization: Highway improvements that upgrade efficiency, functionality, and safety without adding capacity
- Expansion: Improvements that add transportation capacity through the addition of new facilities and/or services

This study identifies potential actions to improve the performance of the US 60|US 70|US 191 corridor. Proposed actions are compared based on their likelihood of achieving desired performance levels, life-cycle costs, and cost effectiveness to produce a prioritized list of solutions that help achieve corridor goals.

The following goals are identified as the desired outcome of this study:

- Link project decision-making and investments on key corridors to strategic goals
- Develop solutions that address identified corridor needs based on measured performance
- Prioritize improvements that cost-effectively preserve, modernize, and expand transportation infrastructure


### 1.3 Corridor Overview and Location

The US 60|US 70|US 191 corridor links the Mexico border at the City of Douglas and the Phoenix metropolitan area to agricultural, mining and recreational activity in southeastern Arizona. The US 60|US 70|US 191 Corridor Profile Study limits extend along US 191 from Douglas to I-10, continuing along US 191 from I-10 to Safford to the junction with US 70, then following US 70 from Safford, passing through the San Carlos Apache Reservation to Globe, and transitioning to the US 60 from Globe, through Superior to Apache Junction at the US 60|Meridian Road intersection. In general, all three highways are mostly two-lane facilities designed for relatively modest traffic volumes in a rural setting. At the same time, the corridor offers some unique benefits within the Arizona circulation system that could be leveraged for increased usage as the need arises.

US 191 provides a link between Mexico and Interstate $10(\mathrm{I}-10)$, the primary east-west interstate corridor along the southern states. As a result, US 191 serves as a major freight corridor for goods moving between Mexico and the United States. Similarly, the combination of US 191 and US 70 between $\mathrm{I}-10$ and Globe offers a critical connection to mining and agricultural interests located in the greater Safford and Globe areas of Graham and Pinal Counties. US 60 between Globe and Apache Junction links activities within the corridor to the major population and commerce center of the Phoenix metropolitan area.
The combination of all three highways (US 60|US 70|US 191) creates a potentially significant alternative to $\mathrm{I}-10$ and $\mathrm{I}-19$ for travel in the eastern reaches of Arizona. A seamless connection among the three routes as a reliever could have major implications for improving international, interstate and intrastate trade along with opening access to financial and commercial distribution centers in the Phoenix area. It would also provide enhanced accessibility to tourist and recreational opportunities in southeastern Arizona.

### 1.4 Corridor Segments

The US 60|US 70|US 191 Corridor is divided into twenty planning segments for analysis and evaluation. These planning segments allow the corridor to be analyzed at a detailed level so that location-specific needs can be readily identified and compared to other segments on this or other corridors. Segmentation by similar characteristics (e.g., urban/rural surroundings, road width, traffic volumes) allowed the analysis to highlight anomalies or instances of poor performance within the context of each segment. The corridor is segmented at logical breaks where context changes such as terrain, daily traffic volumes, or roadway typical sections. Additional segment breaks may occur at major intersections or junctions, where the corridor transitions from rural to urban environments, other similar operating environments, maintenance sections, and at jurisdictional changes. Corridor segments are described in Table 1 and are shown in Figure 2.

Table 1: US 60|US 70|US 191 Corridor Segments

| Segment \# | Route | Begin | End | Approximate Begin Milepost | Approximate End Milepost | Approximate Length (miles) | Typical Through Lanes (NB/EB, SB/WB) | 2020 (2040) Average Annual Daily Traffic Volume (vpd) | Character Description |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 191-1 | US 191 | US 191B Junction | Elfrida | 0 | 24 | 24 | 1,1 | 2,093 (2,885) | Starting from MP 0 along US 191, this segment is primarily rural in nature, but is the only route to the Bisbee-Douglas International Airport. |
| 191-2 | US 191 | Elfrida | I-10 | 24 | 67 | 43 | 1,1 | 1,534 (2,759) | Beginning in Elfrida, a census-designated place, this segment connects smaller agricultural communities to each other and I-10. |
| 191-3 | US 191 | I-10 | SR 266 | 87 | 104 | 17 | 2,2 | 2,617 (3,316) | No known developments exist along this segment however, it does connect the Arizona State Prison at Fort Grant to l-10 via SR 266. |
| 191-4 | US 191 | SR 266 | Safford City Limit | 104 | 116 | 12 | 1,1 | 4,343 (5,566) | Land along this segment is primarily owned by the Bureau of Reclamation and is therefore undeveloped. The segment begins at SR 266 and ends at approximately the southern limits of Safford. Traffic numbers in this segment increase due to the development south of Safford. |
| 191-5 | US 191 | Safford City Limit | US 70 Junction | 116 | 121 | 5 | 2,2 | 7,903 (9,942) | This segment starts at approximately the southern limits of Safford and ends at the junction with US 70 . The segment is differentiated by jurisdiction and change in route along the corridor rather than any changes in terrain or traffic. |
| 70-6 | US 70 | US 191 Junction | Pima | 339 | 330 | 9 | 2,2 | 11,553 (14,390) | Beginning at the junction with US 191 in Safford and ending at the northern limit of Pima, this segment has very high traffic volumes which can be attributed to the higher density of surrounding communities and agricultural/mining operations. A large majority of the land abutting the route is privately owned. |
| 70-7 | US 70* | Pima | San Carlos Apache Reservation | 330 | 300 | 19 | 1,1 | 3,116 $(3,909)$ | This segment connects the western limit of Pima to the eastern edge of the San Carlos Apache Reservation. A majority of the land abutting US 70 is privately owned and used for agricultural purposes. Milepost equation MP 314.21 Back = MP 325.31 Ahead occurs within this segment. |
| 70-8 | US 70 | San Carlos Apache Reservation | Bylas | 300 | 298 | 2 | 1,1 | 2,749 (3,473) | Beginning at the eastern limits of the San Carlos Apache Reservation, this short segment terminates at the eastern limits of Bylas. |
| 70-9 | US 70 | Bylas | Bylas | 298 | 293 | 5 | 1,1 | 2,749 (3,434) | Bylas is a census-designated place within the San Carlos Apache Reservation. The boundary of this segment was determined by the extent of development and not necessarily the jurisdictional limits. |
| 70-10 | US 70 | Bylas | Peridot | 293 | 274 | 19 | 1,1 | 2,749 (3,468) | This segment begins at the western extent of development in Bylas and extends to the eastern limits of development in Peridot. The segment is within the San Carlos Reservation and has low traffic volume. |

AADT = Average Annual Daily Traffic
vpd = vehicles per day
*Milepost equation MP 314.21Back $=$ MP 325.31

Table 1: US 60|US 70|US 191 Corridor Segments (continued)

| Segment \# | Route | Begin | End | Approximate Begin Milepost | Approximate End Milepost | Approximate Length (miles) | Typical <br> Through <br> Lanes <br> (NB/EB, <br> SB/WB) | 2020 (2040) Average Annual Daily Traffic Volume (vpd) | Character Description |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 70-11 | US 70 | Peridot | Peridot | 274 | 270 | 4 | 1,1 | 2,900 (3,500) | The segment starts at the new medical center at the eastern limits of Peridot and extends west to the high school. It is differentiated by Graham/Gila County jurisdiction rather than changes in terrain or traffic. |
| 70-12 | US 70 | Peridot | San Carlos Apache Reservation | 270 | 255 | 15 | 1,1 | 5,800 (7,200) | Beginning at the Peridot High School and continuing to the western limit of the San Carlos Apache Reservation, this segment is differentiated by jurisdiction rather than any changes in terrain or traffic. |
| 70\|60-13 | US 70/US 60 | San Carlos <br> Apache <br> Reservation | Miami | 255 | 243 | 12 | 2,2 | 11,100 (14,400) | Beginning at the western limits of the San Carlos Apache Reservation, this segment goes through the City of Globe, Claypool and Miami. Although this segment includes US 70 and US 60, there is no change in cross section therefore, the segment is differentiated by jurisdiction rather than any other changes. Higher traffic counts are due to the junction of US 60 and US 70 along with higher traffic counts and the proximity of large mines. |
| 60-14 | US 60 | Miami | Superior | 243 | 227 | 16 | 1,1 | 10,100 (15,400) | Beginning at the western limits of Miami and extending to the eastern limits of Superior, this segment bisects the Tonto National Forest. The high traffic volume can be attributed to a significant number of regular commuters in both directions (Valley to Globe) and tourist traffic. |
| 60-15 | US 60 | Superior | Superior | 227 | 225 | 2 | 1,1 | 7,500 (17,200) | This segment starts and ends at approximately the eastern and western limits of Superior. This segment is differentiated by jurisdiction rather than any changes in terrain or traffic. |
| 60-16 | US 60 | Superior | Forest Road 357 | 225 | 223 | 2 | 2,2 | 10,700 (24,600) | This segment is bounded by the Tonto National Forest and is differentiated by the number of thru east and west lanes rather than changes in terrain or jurisdiction. |
| 60-17 | US 60 | Forest Road 357 | SR 79 | 223 | 212 | 11 | 2,2 | 11,000 (25,500) | Although this segment is generally flat in nature, it is differentiated by the number of thru lanes, compared to 60-16. Beginning at State Forest Road 357, this segment terminates at the interchange with SR 79. |
| 60-18 | US 60 | SR 79 | Arizona Renaissance Festival Grounds | 212 | 205 | 7 | 2,2 | 14,000 (22,900) | The segment starts at the Florence Junction and terminates near the Arizona Renaissance Festival Grounds, just south of Gold Canyon. |
| 60-19 | US 60 | Arizona Renaissance Festival Grounds | South Mountain View Road | 205 | 199 | 6 | 2,2 | 18,700 (13,900) | This segment begins near Arizona Renaissance Festival Grounds, passes through Gold Canyon, and terminates at the eastern limits of Apache Junction. |
| 60-20 | US 60 | South Mountain View Road | Meridian Road | 199 | 194.3 | 5 | 2,2 | 48,500 (59,500) | Beginning at the eastern limits of Apache Junction and ending at the western limits of Apache Junction, this segment is widely used by people traveling to and from the Phoenix Metropolitan area. |

AADT = Average Annual Daily Traffic
vpd $=$ vehicles per day ${ }^{*}$ Milepost equation MP 314.21Back $=$ MP 325.31

Figure 2: Corridor Location and Segments


### 1.5 Corridor Characteristics

The US 60|US $70 \mid$ US 191 corridor provides primary access to agriculture, mining and recreation areas in the southeastern part of Arizona. The corridor intersects I-10, which provides east and west access to and from the corridor. Beginning in Douglas, just north of the international border, the corridor extends northwest through Safford to Apache Junction, at the edge of the Phoenix metropolitan region, providing a key economic and recreational link in the region and state.

National Context
The southern and northern portions of the corridor both provide connectivity to the national transportation network. The southern portion of the corridor, US 191 south of I-10, provides a link between Mexico and I-10, the main east-west corridor along the southern states. As a result, US 191 serves as a major freight corridor for goods moving between Mexico and the US. The portion of the corridor north of I-10 provides connectivity between major mining and agricultural areas, linking to $\mathrm{I}-10$ for national distribution.

Regional Connectivity
The combination of US 191 and US 70 between I-10 and Globe offers a critical connection to mining and agricultural interests located in the greater Safford and Globe areas of Graham and Pinal Counties. US 60 between Globe and Apache Junction ties all the activities within the corridor, along with additional mining and recreational opportunities along US 60, to the major population and commerce center of the Phoenix metropolitan area

Commercial Truck Traffic
The US 60|US $70 \mid$ US 191 corridor serves as an important route for agricultural products grown in the Gila River Valley, and for large mining operations near Safford, Miami and Superior. According to ADOT's 2020 Highway Performance Monitoring System (HPMS) data, the average daily commercial truck volumes along the corridor range from $9 \%$ to $27 \%$ of the total traffic flow on the corridor. Segments with higher truck percentages include Segments 191-2,191-3 and Segment 70-7 through Segment 70-11. The high volume of trucks on these segments can be attributed to the large active mines in the Safford and Globe areas, as well as agricultural shipments. Due to the nature of truck traffic, oversize loads are common on this corridor.

The Douglas Port of Entry (POE) is located at the southern end of the corridor. In 2020, this crossing was the second busiest port in Arizona in terms of total number of loaded truck containers, accounting for approximately $9 \%$ of all truck crossings within the State. One inspection station is located adjacent to northbound US 191 at MP 1 and includes a weigh-inmotion scale. One permanent border checkpoint is located just north of Elfrida, on northbound US 191 in Segment 191-2. This location requires all vehicles to stop for inspection, which can create some delay with commercial truck traffic

## Commuter Traffic

Commuter traffic on US 60|US 70|US 191 occurs mostly within the urbanized areas of Safford,

Globe and Superior, which are the primary economic centers along the corridor. According to the most recent traffic volume data maintained by ADOT, traffic volumes range from approximately 1,500 vehicles per day in the El Frida/Sunsites/Cochise area to approximately 48,500 vehicles per day in the Apache Junction area.
According to the 2020 5-Year American Community Survey data from the US Census Bureau, $82 \%$ of the workforce in the City of Safford, $82 \%$ of the workforce in the City of Globe and $78 \%$ of the workforce in the Town of Superior drove alone for their daily commutes. Carpooling accounted for $9 \%-15 \%$ of daily commuters. As there are limited transit options in this area, less than $1 \%$ of daily commuters used public transportation as a means to get to work. The average commute travel time for these areas is 16-31 minutes. In the less populated areas of Bylas, Elfrida and Miami, there is a lower percentage of the population commuting to work alone, averaging $70 \%$. In Bylas, $0.8 \%$ of commuters used public transportation. Twenty two percent of daily commuters in Elfrida and 20\% of commuters in Miami carpooled. The average commute trave time for these less populated areas is similar to the larger urban areas, 16-31 minutes.

Recreation and Tourism
US 60|US 70|US 191 provides access to many recreational opportunities within the southeastern area of the state, including National Forest, wildlife areas, tribal recreation areas, and parks. The corridor provides access to both the Coronado and Tonto National Forests. The Coronado National Forest is broken up by the National Forest Service into different Ecosystem Managemen Areas, defined by each mountain range. The Dragoon, Pinaleno and Santa Teresa Ecosystem Management Areas are primarily accessed via US 191 or US 70. Segments 70|60-13 through 60-17 bisect the Tonto National Forest and can be used to access the Salt River and Superstition Mountains via SR 188

There are numerous other natural areas and parks along the corridor. The Leslie Canyon Nationa Wildlife Refuge is located east of US 191 between Douglas and Elfrida and encompasses over 2,700 acres. The Refuge was established in 1988 to protect two native fish species of the Rio Yaqui watershed. Located west of US 191 between Douglas and Elfrida is the Whitewater Draw Wildlife Area, which is comprised of 1,500 acres and home to over 20,000 Sandhill Cranes during the winter. The park is open from October 15 through March 15.

Mount Graham is located southwest of Safford and is accessible via US 191. Recreational activities include hiking, rock climbing and cross country skiing. In addition to these opportunities, the Mount Graham is home to the University of Arizona Steward Observatory. The observatory was established in 1916 however construction was delayed due to World War I. By 1963, the original 36 " diameter telescope was replaced with a smaller one due to the increased ligh pollution from the expanding Tucson area.
Coolidge Dam and San Carlos Lake are located west of US 70 just south of Peridot. Built between 1924 and 1928, the Coolidge Dam was part of the San Carlos Irrigation Project and is responsible for irrigating 100,000 acres of agricultural land. Recreational uses within the area include fishing
and boating on San Carlos Lake and hiking/biking on a 13 mile route along the dam's access road.

The Apache Gold Casino and Resort in Globe is located along Highway. Owned by the San Carlos Apache Tribe the casino includes 600 slot machines, and 200 -seat bingo hall, a golf course, 145-room resort with a conference center and an RV park

The Boyce Thompson Arboretum and State Park is located off of US 60 in Superior. Founded in the 1920's, the park is Arizona's oldest and largest botanical garden encompassing 323-acres and includes over three miles of paths and trails
In addition to the recreational amenities already mentioned, there are numerous trailheads along the corridor which are accessible through informal pull off areas.

## Multimodal Uses

Besides commuter and freight traffic, as previously discussed, the US 60 US 70 US 191 corridor also accommodates alternative modes of transportation. The following section will discuss the existing multimodal options connecting communities along the corridor to each other and the surrounding region

Freight Rail
The Arizona Eastern Railroad (AZER) extends from Miami to Lordsburg, New Mexico and Clifton to Lordsburg. The line from Miami to Lordsburg follows the Gila River until Bylas, then parallels US 70 into the Safford area, extending from Segment 191-5 through US 70|60-13. There are three at grade crossings along the corridor. The crossings are located at US 191 near MP 121 in Safford, and MP 246 and MP 247 near Miami. Commodities transported include copper, chemicals, and agricultural and forest products.

There is one additional at grade rail road crossing along the corridor. The Magma Arizona Railroad crosses US 60 near MP 215

Passenger Rail
The "Copper Spike Extension", which traveled from Globe to the Apache Gold Casino Resort on the San Carlos Indian Reservation, was previously used for passenger train service. In 2011, ownership of the line transferred and the line was abandoned

Bicycles/Pedestrians
Cyclists may use state highways unless specifically prohibited, although a majority of the corridor has an effective shoulder width of less than 10 feet on either side. Only Segments 191-3 and 6017 have shoulder widths greater than 10 feet. Sidewalks are located along portions of the corridor within the urbanized areas. A pedestrian bridge at Fort Thomas provides a grade separated crossing of US 70. Additionally, within the areas of Bylas and Peridot, pedestrian facilities are not continuous on both sides of the roadway and drainage features create discontinuity in the informal, unpaved pedestrian network in these areas. Also, fencing along the roadway in Bylas
and Peridot limits pedestrian crossing opportunities, although there are breaks in the fencing Unpaved trails can also be found along the corridor and are served by informal pullouts.

## Bus/Transit

Within the study area there are limited public transit opportunities. There are two local public transportation service providers along the US 60|US 70|US 191 corridor. The San Carlos Apache Nnee Bich'o Nii Public Transit Service provides buses between Safford and Globe with stops in Thatcher, Pima, Fort Thomas, Bylas, Peridot and Globe. There are three routes with an additional Casino Employee Shuttle. Fares range from $\$ 2.00-\$ 10.00$ round trip. The second service provider is the Cobre Valley Community Transit which serves Miami, Globe and unincorporated portions of Gila County. There are two routes between Miami and Globe, operating Monday through Friday $6: 30 \mathrm{am}$ to $6: 00 \mathrm{pm}$. One way fares are $\$ 1.00$. The transit provider also offers a Dial-a-Ride service with fares ranging from $\$ 1.00-\$ 4.00$, depending on distance

While existing public transportation service providers may currently be limited, several recent planning documents and studies have identified the need to increase intercity and intracity public transit options along the corridor.
No Greyhound or Amtrak stations are located along the corridor. Private shuttle service provides transportation from Safford to Willcox, Benson, Tucson International Airport and Phoenix Sky Harbor Airport.

## Aviation

Municipal airports along the corridor are located in Douglas, Safford, San Carlos, and Superior. The Bisbee Douglas International Airport located along US 191 in Douglas is owned by Cochise County and averages 54 aircraft operations per day. Thirty percent of daily operations are military-related and the remainder is general aviation. The Safford Regional Airport is located northeast of the corridor within Safford city limits. The airfield averages 38 aircraft operations per day. The San Carlos Apache Airport is located along US 70 in Globe. It is owned by the San Carlos Apache tribe and averages 36 aircraft operations per week. The Superior Municipa Airfield is located along US 60 near the western boundaries of the town. The airfield averages 200 aircraft operations per year.
Land Ownership, Land Uses and Jurisdictions
As shown Table 2, the corridor crosses multiple jurisdictions and land holdings throughout Cochise, Graham, Gila and Pinal Counties. A majority of the land directly abutting the corridor is privately owned. In the vicinity of the corridor, but not immediately adjacent to it, there are significant Bureau of Reclamation, State Trust and National Forest lands

## Population Centers

The major population centers within the US 60|US 70|US 191 corridor are centered around the urbanized areas of Douglas, Safford, Globe, and Superior. Table 2 provides a summary of the U.S. Census population for the communities along the corridor. The local municipalities saw little
change in population between 2010 and 2020, where several of these municipalities actually decreased in population during the same timeframe. At the county level, the population shift was more noticeable, especially for Cochise and Pinal County. The populations in the communities along the corridor fluctuate significantly with market demands related to mining and agriculture activities. Looking at the projected 2040 population, Douglas, Safford and Bylas will experience the greatest growth. During the same time period, Cochise and Pinal County will also see a large population shift. However, the growth is not focused in the areas along the study corridor.

## Major Traffic Generators

Along the corridor, major traffic generators are related to mining and agriculture activities, as well as recreation and local commuter traffic in the urbanized areas of Douglas, Safford, Globe and Superior. Outside of the study area, major traffic generators include the Douglas Port of Entry, which generates significant freight traffic that utilizes US 191 to access I-10. Traffic generated from agricultural activities fluctuates seasonally. Mining related traffic experiences significant fluctuations as mining activity varies based on the global price of copper.
There are currently operational mines in Superior, Globe-Miami, and north of Safford, with plans for increases in mining activity in the vicinity of Superior. These mining activities generate traffic related to employment, and induced activity related to the increase in population in the local communities. In some cases, shift workers may live temporarily in housing near the mine while their families live in another community, where the mine workers commute home on off days. Due to the shift work related to the mines, there are not typical peak-hour and weekday commute patterns. The mines also generate significant truck traffic, including oversized loads related to mining equipment.

## Tribes

Segments 70-8 through 70-12 bisect the San Carlos Apache Reservation.

## Wildlife Linkages

The Arizona State Wildlife Action Plan (SWAP) provides a 10 -year vision for the entire state, identifying wildlife and habitats in need of conservation, insight regarding the stressors to those resources, and suggested actions that can be taken to alleviate those stressors. Using the HabiMap Tool developed by Arizona Game and Fish Department (AGFD), which is an interactive database of the information included in the SWAP, the following wildlife considerations were identified in relation to the US 60|US 70|US 191 corridor:

- Wildlife waters are located to the north of US 60 near Superior and on both sides of US 191 between Safford and I-10

Table 2: Current and Future Population

| Community | $\mathbf{2 0 1 0}$ <br> Population | $\mathbf{2 0 2 0}$ <br> Population | $\mathbf{2 0 4 0}$ <br> Population | \% Change 2010- <br> $\mathbf{2 0 4 0}$ | Total <br> Growth |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Cochise County | $\mathbf{1 3 1 , 3 4 6}$ | $\mathbf{1 3 1 , 2 7 7}$ | $\mathbf{1 3 0 , 4 5 6}$ | $\mathbf{- 1 \%}$ | $\mathbf{- 8 9 0}$ |
| Douglas | 17,378 | 16,416 | 15,448 | $-11 \%$ | $-1,930$ |
| Elfrida | 459 | 293 | - | - | - |
| Graham County | $\mathbf{3 7 , 2 2 0}$ | $\mathbf{3 8 , 6 1 4}$ | $\mathbf{4 5 , 3 3 1}$ | $\mathbf{2 2 \%}$ | $\mathbf{8 , 1 1 1}$ |
| Safford | 9,566 | 9,974 | 10,950 | $14 \%$ | 1,384 |
| Pima | 2,387 | 2,520 | 2,870 | $20 \%$ | 483 |
| Bylas | 1,962 | 2,005 | 2,459 | $25 \%$ | 497 |
| Peridot | 973 | 994 | 1,220 | $25 \%$ | 247 |
| Gila County | $\mathbf{5 3 , 5 6 5}$ | $\mathbf{5 5 , 1 0 0}$ | $\mathbf{5 4 , 6 1 7}$ | $\mathbf{2 \%}$ | $\mathbf{1 , 0 5 2}$ |
| San Carlos | 4,038 | 5,434 | 5,387 | $33 \%$ | 1,349 |
| Globe | 7,533 | 7,460 | 7,457 | $-1 \%$ | -76 |
| Miami | 1,837 | 1,817 | 1,821 | $-1 \%$ | -16 |
| Pinal County | $\mathbf{3 7 6 , 3 6 9}$ | $\mathbf{4 6 6 , 1 7 5}$ | $\mathbf{8 2 0 , 8 7 7}$ | $\mathbf{1 1 8 \%}$ | $\mathbf{4 4 4 , 5 0 8}$ |
| Superior | 2,835 | 3,161 | $\mathbf{3 , 1 8 4}$ | $\mathbf{1 2 \%}$ | 349 |

Source: U.S. Census, Arizona Department of Administration - Employment and Population Statistics

- Willcox Playa/Cochise Important Bird Area is located along the eastern side of US 191 from approximately MP 60 continuing north to I-10
- A majority of the US 60|US 70|US 191 corridor bisects allotments/pastures, except along US 70 on the San Carlos Reservation and along US 191 south of US 181
- State Land holdings exist within the corridor, primarily along US 191 between Safford and I-10
- US Forest Service Land is located along US 60 and US 70 between SR 79 and SR 77
- Potential Wildlife Linkages exist along US 60 between SR 79 and SR 77 and along US 191 between SR 366 and I-10
- The Species and Habitat Conservation Guide indicates sensitive habitats exist along the corridor except a portion of US 70 which bisects the San Carlos Reservation
- "Species of Greatest Conservation Need" are identified along the corridor except a portion of US 70 which bisects the San Carlos Reservation
- A moderate level of "Species of Economic and Recreational Importance" are identified along the corridor except a portion of US 70 that bisects the San Carlos Reservation

Corridor Assets
The US 60|US 70|US 191 corridor links regionally important communities in the southwestern part of the state to Mexico, I-10 and the Phoenix metropolitan area. The southern portion of US 191 connects the Douglas Port of Entry to I-10 and is an important route for freight. The corridor is also a vital route between the large mining and agricultural activities within the Gila River Valley and the rest of the state. The Transportation Assets Map (Figure 3) shows key features that are available to the travelling public today.

Limited public transportation services are offered within the region. These services either don't span the entire corridor or are only operated on a limited basis. While population changes have not been significant over the last few years, numerous transportation studies have identified a need for intercity and intracity transit services along the corridor.

The majority of assets are located along the most densely populated portions of the corridor near the Safford and Globe areas. In addition to the one Border Patrol check point, one weigh-inmotion scale and four public rest stops already discussed, there are three permanent traffic counters along the corridor, located at MP 337 and MP 254 on US 70 and MP 252 on US 60. There is one short climbing/passing lane for eastbound traffic on US 70 in Segment 70-12, while Segment 60-14 has numerous climbing and passing lanes for both directions. There are several grade-separated crossroads and at-grade railroad crossings along the corridor but they are primarily located near the urbanized areas.
Along the US 60|US 70|US 191 corridor ADOT operates four rest areas. The Douglas Rest Area is located at the southwest corner of US 191 and SR 80 at MP 0. The Safford Park Rest Area is located along the east side of US 70 at MP 338. The third rest area is the Bylas Rest Area along the west side US 70 at MP 296. The fourth rest area is the Superior Rest Area located along the east side of US 60 at MP 226 and serves the eastbound traffic. There are also a number of informal pullouts along the corridor.
There is one closed circuit television (CCTV) camera located along US 70 east of Globe to monitor traffic, as well as one dynamic message sign in the same vicinity currently in design

Figure 3: Corridor Assets


### 1.6 Corridor Stakeholders and Input Process

A Technical Advisory Committee (TAC) was created, which was comprised of representatives from key stakeholders. TAC meetings were held at key milestones to present results and obtain feedback. In addition, several meetings were also conducted with key stakeholders between April 2022 and March 2023.

Key stakeholders for this study include:

- South Eastern Arizona Governments Organization (SEAGO)
- Central Arizona Governments (CAG)
- ADOT Southeast District
- ADOT South Central District
- ADOT Technical Groups
- Arizona Game and Fish Department (AGFD)
- Arizona State Land Department (ASLD)
- Federal Highway Administration (FHWA)

Several Working Papers were developed during the course of the CPS. The Working Papers were provided to the TAC for review and comment.

### 1.7 Prior Studies and Recommendations

This section provides a summary of previous studies and plans and their recommendations that are relevant to the IUS 60|US 70|US 191 CPS.

## Framework and Statewide Studies

- ADOT Bicycle and Pedestrian Plan Update
- ADOT Five-Year Transportation Facilities Construction Program 2016-2020
- ADOT Climbing and Passing Lane Prioritization Study
- Arizona Key Commerce Corridors
- Arizona Multimodal Freight Analysis Study
- Arizona Ports of Entry Study
- Arizona State Airports System Plan
- Arizona State Rail Plan
- Arizona Statewide Dynamic Message Sign Master Plan
- Arizona Statewide Rail Framework Study
- Arizona Statewide Shoulders Study
- Arizona Roadway Departure Safety Implementation Plan (RDSIP)
- Arizona Wildlife Action Plan / Arizona Wildlife Linkages Assessment
- Building a Quality Arizona (BQAZ)
- Eastern Arizona Framework Study
- FHWA Freight Analysis Framework
- MAG 2035 RTP
- What Moves You Arizona? Long-Range Transportation Plan 2010-2035

Regional Planning Studies

- Arizona - Sonora Border Master Plan
- Bi-National Border Transportation Infrastructure Needs Study
- Gila County Rail Passenger Study
- Graham County Transit Feasibility Study
- Pinal County Comprehensive Plan Update
- Pinal County Open Space and Trails Master Plan
- Pinal County Regionally Significant Routes for Safety and Mobility Study
- Pinal County Transit Feasibility Study
- Pinal Creek Trail Conceptual Plan
- Safford General Plan
- SEAGO Transportation Coordination plan Update
- SR 80 \& US 191 Oversized Load Study

Planning Assistance for Rural Areas (PARA) and Small Area Transportation Studies (SATS)

- Cobre Valley Comprehensive Transportation Study
- City of Douglas Small Area Transportation Study
- Gila County Small Area Transportation Study
- Gila County Transportation Study
- Graham County Alternate Route Study
- Graham County/ Safford/ Thatcher/ Pima Small Area Transportation Study
- San Carlos Apache Tribe Transit Feasibility Study

Design Concept Reports (DCR) and Project Assessments (PA)

- US 60 Florence Junction - Superior DCR
- US 60 Superior - Globe Feasibility Study
- US 60 Superior - Globe Scoping (MP 222 - MP 258)
- US 70 Bylas Road Safety Assessment
- US 70 Segment 1 Pima - Thatcher Final DCR
- US 70 Segment 2 Thatcher - Safford Final DCR
- US 191 Douglas to I-10 Final DCR
- US 191 I-10 to SR 266 Final DCR
- US 191 Jct SR 266 to US 70 Final Corridor Selection Report
- US 191 Whitewater Draw to Thompson Rd Final DCR
- US 60 Passing Lanes (Miami-Superior) Final PA

Table 3: Corridor Recommendations from Previous Studies

| Map Key Ref. \# | Begin MP | End MP | Length (miles) | Project Description | Investment Category (Preservation [P], Modernization[M], Expansion [E]) |  |  | Status of Recommendation |  |  | Name of Study |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | P | M | E | Program Year | Project No. | Environmental Documentation (Y/N) |  |
| 1 | 2 | 2 | 0 | DMS sign north and southbound |  | $\checkmark$ |  |  |  | N | Arizona Statewide DMS Plan |
| 2 | 7 | N/A | N/A | Bisbee Douglas International Airport improvements | $\checkmark$ |  |  | 2017-2019 |  | N | ADOT Five Year Program |
| 3 | 67.5 | 67.5 | 0 | Reconstruct interchange with I-10 |  | $\checkmark$ |  |  |  | N | Arizona Key Commerce Corridors |
| 4 | 87 | 121 | 34 | Reconstruct to 4 lane divided highway l-10 to US 70 |  |  | $\checkmark$ |  |  | N | BQAZ <br> Eastern Arizona Framework Study |
| 5 | 90 | 90 | 0 | DMS sign southbound |  | $\checkmark$ |  |  |  | N | Arizona Statewide DMS Plan |
| 6 | 104 | 121 | 17 | Alternate Route |  |  | $\checkmark$ |  |  | N | Graham County SATS/US 191 Alternative Route Study/US 191 Jct. SR 266 to US 70 Corridor Selection |
| 7 | 104.6 | 121 | 16.4 | Local public transit service |  | $\checkmark$ |  |  |  | N | Graham County SATS |
| 8 | 110.9 | 116 | 5.1 | Restripe to 5 lanes between Atresia Road and Lebanon Road |  |  | $\checkmark$ | 2018-2023 |  | $N$ | Graham County SATS |
| 9 | 110.9 | 118 | 4.4 | Widen to 4 lanes between Artesia Road and Armory Road |  |  | $\checkmark$ | 2008-2013 |  | $N$ | Graham County SATS |
| 10 | 114 | 114 | 0 | SR 366 and Swift Trail Road Intersection Improvement |  | $\checkmark$ |  | 2008-2013 |  | $N$ | Graham County SATS |
| 11 | 114 | 118 | 4 | Pavement preservation | $\checkmark$ |  |  | 2016 |  | Y | ADOT Five Year Program |
| 12 | 116 | 116 | 0 | DMS sign northbound |  | $\checkmark$ |  |  |  | N | Arizona Statewide DMS Plan |
| 13 | 118 | 118 | 0 | Armory Road Intersection Improvement |  | $\checkmark$ |  | 2008-2013 |  | N | Graham County SATS |
| 14 | 119 | 119 | 0 | Discovery Park Boulevard Intersection Improvement |  | $\checkmark$ |  | 2008-2013 |  | N | Graham County SATS |
| 15 | 120 | 121 | 1 | Restripe to 5 lanes between $11^{\text {th }}$ Street and US 70 |  |  | $\checkmark$ | 2008-2013 |  | N | Graham County SATS |
| 16 | 121 | N/A | N/A | Extend Highway North US 70 to $8^{\text {th }}$ Street |  |  | $\checkmark$ | 2018-2023 |  | $N$ | Graham County SATS |
| 17 | 121 | N/A | N/A | Safford Regional Airport improvements | $\checkmark$ | $\checkmark$ | $\checkmark$ | 2016-2020 |  | $N$ | ADOT Five Year Program |

Table 3: Corridor Recommendations from Previous Studies (continued)

| Map Key Ref. \# | Begin MP | End MP | Length (miles) | Project Description | Investment Category (Preservation [P], Modernization[M], Expansion [E]) |  |  | Status of Recommendation |  |  | Document |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | P | M | E | Program Year | Project No. | Environmental Documentation (Y/N) |  |
| 18 | 339 | 339 | 0 | Intersection Improvement |  | $\checkmark$ |  | 2008-2013 |  | N | Graham County SATS |
| 19 | 339 | 338 | 1 | Safety /Intersection Improvements |  | $\checkmark$ |  | 2018 |  | N | ADOT Five Year Program |
| 20 | 339 | 328 | 11 | Provide enhanced local transit in Safford/Pima/Thatcher |  |  | $\checkmark$ |  |  | N | Eastern Arizona Framework Study Graham County Transit Feasibility Study |
| 21 | 339 | 328 | 11 | Provide Complete Streets in Safford/Pima/Thatcher |  | $\checkmark$ |  |  |  | N | Eastern Arizona Framework Study |
| 22 | 339 | 253 | 86 | Widen roadway to 4 lanes between US 191 and Globe |  |  | $\checkmark$ |  |  | N | Eastern Arizona Framework Study/BQAZ |
| 23 | 337 | 337 | 0 | Intersection Improvement |  | $\checkmark$ |  | 2008-2013 |  | N | Graham County SATS |
| 24 | 335.8 | 335.8 | 0 | Intersection Improvement |  | $\checkmark$ |  | 2008-2013 |  | N | Graham County SATS |
| 25 | 335.7 | 335.7 | 0 | Intersection Improvement |  | $\checkmark$ |  | 2008-2013 |  | N | Graham County SATS |
| 26 | 335.6 | 335.6 | 0 | Intersection Improvement |  | $\checkmark$ |  | 2008-2013 |  | N | Graham County SATS |
| 27 | 335.5 | 335.5 | 0 | Traffic signal or roundabout |  | $\checkmark$ |  | 2008-2013 |  | N | Graham County SATS |
| 28 | 330 | 329 | 1 | Construct Pedestrian Bridge Extension |  | $\checkmark$ |  | 2017 | H8397 01C | Y | ADOT Five Year Program |
| 29 | 312.25 | 312.25 | 0 | Add Center Turn Lane BryceEden Road |  |  | $\checkmark$ |  |  | N | Graham County SATS |
| 30 | 300 | 299 | 1 | Bridge Replacement and Rehabilitation | $\checkmark$ |  |  | 2016 | H8547 01C | Y | ADOT Five Year Program |
| 31 | 300 | 291 | 9 | Pathway, entry monument and intersection improvements |  | $\checkmark$ |  | 2016 | $\begin{aligned} & \hline \text { H8031 01C } \\ & \text { H7637 01C } \end{aligned}$ | Y | ADOT Five Year Program |
| 32 | 298 | 294 | 4 | Construct continuous two-way left turn lane |  |  | $\checkmark$ |  |  | N | Road Safety Assessment US 70 |
| 33 | 298 | 294 | 4 | Install street name signs for all intersections |  | $\checkmark$ |  |  |  | N | Road Safety Assessment US 70 |
| 34 | 298 | 294 | 4 | Evaluate 50 MPH speed limit |  | $\checkmark$ |  |  |  | N | Road Safety Assessment US 70 |

Table 3: Corridor Recommendations from Previous Studies (continued)

| Map Key Ref. \# | Begin MP | End MP | Length (miles) | Project Description | Investment Category (Preservation [P], Modernization[M], Expansion [E]) |  |  | Status of Recommendation |  |  | Document |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | P | M | E | Program Year | Project No. | Environmental Documentation (Y/N) |  |
| 35 | 298 | 294 | 4 | Pedestrian Safety improvements - Pedestrian crossings, warning signs/flashing lights, ADA compliant pedestrian gates |  | $\checkmark$ |  |  |  | N | Road Safety Assessment US 70 |
| 36 | 297.7 | 296.5 | 1.1 | Eliminate passing zone through Bylas |  | $\checkmark$ |  |  |  | N | Road Safety Assessment US 70 |
| 37 | 297 | 294 | 3 | Repair 4 street lights west of rest area, 3 lights between MP 294 and 295 and 1 between MP 267 and 297 |  | $\checkmark$ |  |  |  | N | Road Safety Assessment US 70 |
| 38 | 296.5 | 296.5 | 0 | Curb installation on north side of US 70 |  | $\sqrt{ }$ |  |  |  | N | Road Safety Assessment US 70 |
| 39 | 296.5 | 296.5 | 0 | Realign intersection |  | $\checkmark$ |  |  |  | $N$ | Road Safety Assessment US 70 |
| 40 | 295.5 | 294.6 | 0.9 | Eliminate passing zone through Bylas |  | $\checkmark$ |  |  |  | N | Road Safety Assessment US 70 |
| 41 | 288 | 282 | 6 | Tier 2 priority westbound climbing lane |  | $\checkmark$ |  |  |  | N | ADOT Climbing and Passing Lane Prioritization Study |
| 42 | 288 | 281 | 7 | Tier 2 priority westbound passing lane |  | $\checkmark$ |  |  |  | N | ADOT Climbing and Passing Lane Prioritization Study |
| 43 | 271 | 269 | 2 | Construct passing lanes |  | $\checkmark$ |  | 2018 |  | N | ADOT Five Year Program |
| 44 | 271 | 251 | 20 | Passenger rail service along Arizona Eastern Railway from Globe to San Carlos |  |  | $\checkmark$ |  |  | N | Gila County Rail Passenger Study |
| 45 | 270 | 267 | 3 | Tier 2 priority east and westbound passing lane |  | $\checkmark$ |  |  |  | N | ADOT Climbing and Passing Lane Prioritization Study |
| 46 | 264 | 262 | 2 | Tier 2 priority eastbound climbing lane |  | $\checkmark$ |  |  |  | N | ADOT Climbing and Passing Lane Prioritization Study |
| 47 | 259 | 259 | 0 | San Carlos Apache Airport improvements | $\checkmark$ | $\checkmark$ | $\checkmark$ | 2016-2020 |  | N | ADOT Five Year Program |
| 48 | 254 | 254 | 0 | Intersection Study at SR 70 and SR 77 |  | $\checkmark$ |  | 2015 |  | N | Cobre Valley Comprehensive Transportation Study |
| 49 | 254 | 235.5 | 0.5 | Widen to four-lane roadway |  | $\checkmark$ | $\sqrt{ }$ | 2020 |  | N | Cobre Valley Comprehensive Transportation Study |
| 50 | 253.75 | 253.75 | 0 | Rehabilitate Southern Pacific bridge |  | $\sqrt{ }$ |  | 2020 |  | N | Cobre Valley Comprehensive Transportation Study |

Table 3: Corridor Recommendations from Previous Studies (continued)

| Map Key Ref. \# | Begin MP | End MP | Length (miles) | Project Description | Investment Category (Preservation [P], Modernization[M], Expansion [E]) |  |  | Status of Recommendation |  |  | Document |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | P | M | E | Program Year | Project No. | Environmental Documentation (Y/N) |  |
| 51 | 253 | 253 | 0 | DMS sign eastbound |  | $\checkmark$ |  |  |  | N | Arizona Statewide DMS Plan |
| 52 | 252 | 243 | 9 | Speed Limit Study |  | $\checkmark$ |  | 2015 |  | N | Cobre Valley Comprehensive Transportation Study |
| 53 | 252 | 243 | 9 | Construct new sidewalks on north side |  | $\checkmark$ |  | 2020 |  | N | Cobre Valley Comprehensive Transportation Study |
| 54 | 252 | 212 | 40 | Construct alternative alignment/Widen to 4 lanes |  |  | $\checkmark$ | 2030 |  | N | Cobre Valley Comprehensive Transportation Study /BQAZ |
| 55 | 252 | 227 | 25 | Priority Paved Shoulder Opportunity |  | $\checkmark$ |  |  |  | N | ADOT Statewide Bicycle and Pedestrian Plan Update |
| 56 | 251 | 246 | 5 | Passenger rail service along Arizona Eastern Railway from Miami to Globe |  |  | $\checkmark$ |  |  | N | Gila County Rail Passenger Study |
| 57 | 250.75 | 250.75 | 0 | Replace Maple Street Bridge |  | $\checkmark$ |  | 2020 |  | N | Cobre Valley Comprehensive Transportation Study |
| 58 | 249.9 | 249.9 | 0 | Rehabilitate Pinal Creek bridge |  | $\checkmark$ |  | 2020 |  | N | Cobre Valley Comprehensive Transportation Study |
| 59 | 247 | 246.5 | 0.5 | Access Management Study |  | $\checkmark$ |  | 2015 |  | N | Cobre Valley Comprehensive Transportation Study |
| 60 | 247 | 247 | 0 | DMS Sign Eastbound |  | $\checkmark$ |  |  |  | N | Arizona Statewide DMS Plan |
| 61 | 245.5 | 243 | 2.5 | Implement access management through Miami |  | $\checkmark$ |  | 2030 |  | N | Cobre Valley Comprehensive Transportation Study |
| 62 | 244.6 | 244.6 | 0 | Intersection improvements at Latham Boulevard |  | $\checkmark$ |  | 2020 |  | N | Cobre Valley Comprehensive Transportation Study |
| 63 | 244.5 | 244.5 | 0 | Add exclusive turn lanes on US 60 |  | $\checkmark$ |  | 2020 |  | N | Cobre Valley Comprehensive Transportation Study |
| 64 | 244.25 | 244 | 0.25 | Restripe to a five-lane section |  | $\checkmark$ |  | 2020 |  | N | Cobre Valley Comprehensive Transportation Study |
| 65 | 243.75 | 243.75 | 0 | Rehabilitate Bloody Tanks Wash bridge |  | $\checkmark$ |  | 2020 |  | N | Cobre Valley Comprehensive Transportation Study |
| 66 | 242 | 242 | 0 | Re-align intersection |  | $\checkmark$ |  | 2030 |  | N | Cobre Valley Comprehensive Transportation Study |
| 67 | 242 | 227 | 15 | East and Westbound Shoulder Improvement |  | $\checkmark$ |  |  |  | N | Statewide Shoulders Study |

Table 3: Corridor Recommendations from Previous Studies (continued)

| Map Key Ref. \# | Begin MP | End MP | Length (miles) | Project Description | Investment Category (Preservation [P], Modernization[M], Expansion [E]) |  |  | Status of Recommendation |  |  | Document |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | P | M | E | Program Year | Project No. | Environmental Documentation (Y/N) |  |
| 68 | 226 | 213 | 13 | Regional part-time bus service between Florence Junction and Superior; park-and-ride in the vicinity of Florence Junction |  |  | $\checkmark$ |  |  | N | Pinal County Transit Feasibility Study |
| 69 | 222.3 | 219.9 | 2.4 | Picket Post- Construct new EB lanes parallel to existing, between Reymert Wash and Queen Creek |  |  | $\checkmark$ |  |  | Y | US 60 Florence Jct - Superior DCR and EA |
| 70 | 219.9 | 216.3 | 3.6 | Gonzales Pass- Construct new EB lanes west of the summit, construct new WB lanes east of the summit |  |  | $\checkmark$ |  |  | Y | US 60 Florence Jct - Superior DCR and EA |
| 71 | 215 | 214 | 1 | Queen Valley TI- Construct full access controlled, gradeseparated interchange over Queen Valley Rd and the Arizona Magma RR |  |  | $\checkmark$ |  |  | Y | US 60 Florence Jct - Superior DCR and EA |
| - | N/A | N/A | 0 | Bridge Infrastructure Improvements East of SR 177 | $\checkmark$ |  |  |  |  | N | Arizona Key Commerce Corridor |
| - | N/A | N/A | 0 | Bridge Infrastructure Improvements between SR 177 and SR 77 | $\checkmark$ |  |  |  |  | N | Arizona Key Commerce Corridor |
| - | N/A | N/A | 0 | Bridge Infrastructure Improvements at Globe | $\checkmark$ |  |  |  |  | N | Arizona Key Commerce Corridor |

Figure 4: Corridor Recommendations from Previous Studies


### 2.0 CORRIDOR PERFORMANCE

This chapter describes the evaluation of the existing performance of the US 60|US 70|US 191 corridor. A series of performance measures are used to assess the corridor. The results of the performance evaluation are used to define corridor needs relative to the long-term goals and objectives for the corridor.

### 2.1 Corridor Performance Framework

This study uses a performance-based process to define baseline corridor performance, diagnose corridor needs, develop corridor solutions, and prioritize strategic corridor investments. In support of this objective, a framework for the performance-based process was developed through a collaborative process involving ADOT and the CPS consultant teams.
Figure 5 illustrates the performance framework, which includes a two-tiered system of performance measures (primary and secondary) to evaluate baseline performance. The primary measures in each of five performance areas are used to define the overall health of the corridor, while the secondary measures identify locations that warrant further diagnostic investigation to delineate needs. Needs are defined as the difference between baseline corridor performance and established performance objectives.

Figure 5: Corridor Profile Performance Framework


The following five performance areas guide the performance-based corridor analyses:

- Pavement
- Bridge
- Mobility
- Safety
- Freight

These performance areas reflect national performance goals stated in Moving Ahead for Progress in the $21^{\text {st }}$ Century (MAP-21):

- Safety: To achieve a significant reduction in traffic fatalities and serious injuries on all public roads
- Infrastructure Condition: To maintain the highway infrastructure asset system in a state of good repair
- Congestion Reduction: To achieve a significant reduction in congestion on the National Highway System
- System Reliability: To improve the efficiency of the surface transportation system
- Freight Movement and Economic Vitality: To improve the national freight network, strengthen the ability of rural communities to access national and international trade markets, and support regional economic development
- Environmental Sustainability: To enhance the performance of the transportation system while protecting and enhancing the natural environment
- Reduced Project Delivery Delays: To reduce project costs, promote jobs and the economy, and expedite the movement of people and goods by accelerating project completion
In 2015, the Fixing America's Surface Transportation Act (FAST Act) was passed. The FAST Act continued to emphasize the performance management approach identified in MAP-21 but included additional provisions for meeting established performance targets.
The MAP-21 and FAST Act performance areas were considered in the development of ADOT's P2P process, which integrates transportation planning with capital improvement programming and project delivery. Because the P2P program requires the preparation of annual transportation system performance reports using the five performance areas, consistency is achieved among various ADOT processes by using these same performance areas.
While these performance areas were established prior to the earlier rounds of the CPS program, several related federal and ADOT reporting measures and targets were not yet in place at that time. These measures and targets have since been established (subsequent to completion of the prior CPS rounds). As such, it became necessary to revisit and revise the CPS performance measures to be more consistent with the latest federal and ADOT reporting measures and targets.
The performance measures include five primary measures: Pavement Index, Bridge Index, Mobility Index, Safety Index, and Freight Index. Additionally, a set of secondary performance measures provides for a more detailed analysis of corridor performance.
Each of the primary and secondary performance measures is comprised of one or more quantifiable indicators. A three-level scale was developed to standardize the performance scale


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across the five performance areas, with numerical thresholds specific to each performance measure:
Good/Above Average Performance

Fair/Average Performance
Poor/Below Average Performance

Rating is above the identified desirable/average range Rating is within the identified desirable/average range

Rating is below the identified desirable/average range

Table 4 provides the complete list of primary and secondary performance measures for each of the five performance areas.

Table 4: Corridor Performance Measures

| Performance Area | Primary Measure | Secondary Measures |
| :---: | :---: | :---: |
| Pavement | Pavement Index <br> Based on a combination of International Roughness Index, cracking, and rutting | - Directional Pavement Serviceability <br> - Pavement Failure <br> - Pavement Hot Spots |
| Bridge | Bridge Index <br> Based on lowest of deck, substructure, <br> superstructure and structural evaluation rating | - Bridge Sufficiency <br> - Bridge Rating <br> - Bridge Hot Spots |
| Mobility | Mobility Index <br> Based on combination of existing and future daily volume-to-capacity ratios | - Future Congestion <br> - Peak Congestion <br> - Travel Time Reliability <br> - Multimodal Opportunities |
| Safety | Safety Index <br> Based on frequency of fatal and suspected serious injury crashes | - Directional Safety Index <br> - Strategic Traffic Safety Plan Emphasis Areas <br> - Other Crash Unit Types <br> - Safety Hot Spots |
| Freight | Freight Index Based on bi-directional truck travel time reliability | - Travel Time Reliability <br> - Bridge Vertical Clearance <br> Bridge Vertical Clearance Hot Spots |

The general template for each performance area is illustrated in Figure 6.
The guidelines for performance measure development are:

- Indicators and performance measures for each performance area should be developed for relatively homogeneous corridor segments
- Performance measures for each performance area should be tiered, consisting of primary measure(s) and secondary measure(s)
- Primary and secondary measures should assist in identifying those corridor segments that warrant in-depth diagnostic analyses to identify performance-based needs and a range of corrective actions known as solution sets
- One or more primary performance measures should be used to develop a Performance Index to communicate the overall health of a corridor and its segments for each performance area; the Performance Index should be a single numerical index that is quantifiable, repeatable, scalable, and capable of being mapped; primary performance measures should be transformed into a Performance Index using mathematical or statistical methods to combine one or more data fields from an available ADOT database
- One or more secondary performance measure indicators should be used to provide additional details to define corridor locations that warrant further diagnostic analysis; secondary performance measures may include the individual indicators used to calculate the Performance Index and/or "hot spot" features

Figure 6: Performance Area Template


### 2.2 Pavement Performance Area

The Pavement performance area consisted of a primary measure (Pavement Index) and three secondary measures, as shown in Figure 7. These measures assess the condition of the existing pavement along the US 60|US 70|US 191 corridor. The detailed calculations and equations developed for each measure are available in Appendix B and the performance data for this corridor is contained in Appendix C.
This CPS is an update to a previously completed report. The performance measures and performance thresholds have been revised from the previous version. For the Pavement performance area, the new methodology includes the use of Rutting data and the performance thresholds have been slightly modified.

## Figure 7: Pavement Performance Measures



## Primary Pavement Index

The Pavement Index is calculated using two pavement condition ratings: the Pavement Serviceability Rating (PSR) and the Pavement Distress Index (PDI).
The PSR is extracted from the International Roughness Index (IRI), a measurement of pavement roughness based on field-measured longitudinal roadway profiles. The PDI is extracted from the Cracking Rating (CR) and Rutting Rating, field-measured samples from each mile of highway.

Both the PSR and PDI use a 0 to 5 scale with 0 representing the lowest performance and 5 representing the highest. The Pavement Index for each segment is a weighted average of the directional ratings based on the number of travel lanes. Therefore, the condition of a section with more travel lanes will have a greater influence on the resulting segment Pavement Index than the condition of a section with fewer travel lanes.
Each corridor segment is rated on a scale with other segments in similar operating environments. Within the Pavement performance area, the relevant operating environments are designated as interstate and non-interstate segments. For US 60|US 70|US 191, all segments are considered the non-interstate operating environment.

## Secondary Pavement Measures

Three secondary measures provide an in-depth evaluation of the different characteristics of pavement performance.

Directional Pavement Serviceability

- Weighted average (based on number of lanes) of the PSR for the pavement in each direction of travel


## Pavement Failure

- Percentage of pavement area rated above failure thresholds for IRI, Cracking or Rutting Pavement Hot Spots
- A Pavement "hot spot" exists where a given one-mile section of roadway rates as being in "poor" condition
- Highlights problem areas that may be under-represented in a segment average. This measure is recorded and mapped, but not included in the Pavement performance area rating calculations

Pavement Performance Results
The Pavement Performance Index provides a high-level assessment of the pavement condition for the corridor and for each segment. The three secondary measures provide more detailed information to assess pavement performance.
Based on the results of the analysis, the following pavement conditions were observed on US 60|US 70|US 191:

- Based on the weighted average of the Pavement Index, the pavement is in "poor" condition on 6 of the 20 segments studied, "fair" condition for 7 of the segments and "good" condition for the other 7 segments.
- Hot spots are present in all segments except for Segments 60E-15, 60E-16, and 60E-20.
- Pavement Failure evaluation assesses the percentage of lane miles considered in failure throughout the corridor. All segments show "poor" performance in area failure except for Segments 60E-15, 60E-16, and 60E-20.
- The Directional Pavement Serviceability shows "fair" or "good" performance except for Segment 70-7 which shows "poor" performance for the NB/WB travel
- Segment 191-2 yielded the lowest Pavement Index

Table 5 summarizes the Pavement performance results for the US 60|US 70|US 191 corridor. Figure 8 illustrates the primary Pavement Index performance and locations of pavement hot spots along the US 60|US 70|US 191 corridor. Maps for each secondary measure can be found in Appendix A.

Table 5: Pavement Performance

| Segment | Segment Length (miles) | Pavement Index | Pavement Serviceability |  | \% Area Failure |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $\begin{gathered} \text { Dir } 1 \\ \text { (NB/WB) } \end{gathered}$ | $\begin{gathered} \text { Dir } 2 \\ \text { (SB/EB) } \end{gathered}$ |  |
| 191-1 | 24 | 3.17 | 3.10 | 3.24 | 71\% |
| 191-2 | 43 | 2.89 | 3.44 | 3.38 | 56\% |
| 191-3 | 17 | 3.42 | 3.63 | 3.69 | 72\% |
| 191-4 | 12 | 3.44 | 3.29 | 3.32 | 42\% |
| 191-5 | 5 | 3.10 | 3.16 | 3.07 | 80\% |
| 70-6 | 9 | 3.23 | 3.15 | 3.25 | 60\% |
| 70-7 | 19 | 2.83 | 2.87 | 3.08 | 87\% |
| 70-8 | 2 | 2.59 | 3.35 | 3.67 | 100\% |
| 70-9 | 5 | 2.71 | 3.44 | 3.63 | 100\% |
| 70-10 | 19 | 2.69 | 3.10 | 3.35 | 79\% |
| 70-11 | 4 | 2.40 | 3.27 | 3.28 | 88\% |
| 70-12 | 15 | 3.57 | 3.28 | 3.53 | 33\% |
| 70\|60-13 | 12 | 3.28 | 3.13 | 3.28 | 54\% |
| 60E-14 | 16 | 3.68 | 3.66 | 3.82 | 44\% |
| 60E-15 | 2 | 4.03 | 3.70 | 3.65 | 0\% |
| 60E-16 | 2 | 4.50 | 4.22 | 4.15 | 0\% |
| 60E-17 | 11 | 3.51 | 3.93 | 3.99 | 76\% |
| 60E-18 | 7 | 3.30 | 3.62 | 3.83 | 93\% |
| 60E-19 | 6 | 3.57 | 3.57 | 3.65 | 33\% |
| 60E-20 | 5 | 4.17 | 3.87 | 3.83 | 0\% |
| Weighted Corridor Average |  | 3.17 | 3.34 | 3.45 | 62\% |
| SCALES |  |  |  |  |  |
| Performance Level |  | Non-Interstate |  |  |  |
| Good |  | > 3.50 | > 3.50 |  | < 5\% |
| Fair |  | 2.90-3.50 | 2.90-3.50 |  | 5\%-20\% |
| Poor |  | <2.90 | <2.90 |  | > $20 \%$ |
| Performance Level |  | Interstate |  |  |  |
| Good |  | > 3.75 | > 3.75 |  | < 5\% |
| Fair |  | 3.20-3.75 | 3.40-3.75 |  | 5\%-20\% |
| Poor |  | < 3.20 | < 3.4 |  | > 20\% |

Moving Ahead for Progress in the 21st Century Act of 2012 (MAP-21), identified national transportation system goals. The transportation asset management regulations associated with the infrastructure condition goals required the development of a Transportation Asset Management Plan (TAMP) covering National Highway System (NHS) bridges and pavements. As part of the statewide TAMP, ADOT developed pavement performance metrics and thresholds in compliance with federal tracking and reporting requirements, as shown in Table 6. The thresholds shown in Table 6 are the basis for the TAMP and ADOT's federal reporting and are different than those used in this CPS, which are based on ADOT's Pavement Management System, as shown in Table 5. The TAMP reports asset condition information in the aggregate at the statewide level and applying the thresholds shown in Table 6 would result in different segment-level performance than shown in Table 5.

Table 6: Statewide TAMP Metrics

| Metric | Good | Fair | Poor |
| :--- | :---: | :---: | :---: |
| IRI (in./mile) | $<95$ | $95-170$ | $>170$ |
| Cracking (\%) | $<5$ | $5-20$ (asphalt) | $>20$ |
|  |  | $5-15$ (jointed concrete) | $>15$ |
| Rutting (in.) | $<0.20$ | $5-10$ (cont. reinforced concrete) | $>10$ |
| Faulting (in.) | $<0.10$ | $0.20-0.40$ | $>0.40$ |

Figure 8: Pavement Performance


### 2.3 Bridge Performance Area

The Bridge performance area consists of a primary measure (Bridge Index) and three secondary measures, as shown in Figure 9. These measures assess the condition of the existing bridges along the US 60|US 70|US 191 corridor. Only bridges that carry mainline traffic or bridges that cross the mainline are included in the calculation. The detailed calculations and equations developed for each measure are available in Appendix B and the performance data for this corridor is contained in Appendix C.
This CPS is an update to a previously completed report. The performance measures and performance thresholds have been revised from the previous version. For the Bridge performance area, the new methodology does not include the performance metric related to Functionally Obsolete bridges, which was used in the previous methodology.

Figure 9: Bridge Performance Measures


Primary Bridge Index
The Bridge Index is calculated based on the use of four different bridge condition ratings from the ADOT Bridge Database, also known as the Arizona Bridge Information and Storage System (ABISS). The four ratings are the Deck Rating, Substructure Rating, Superstructure Rating, and Structural Evaluation Rating. These ratings are based on inspection reports and establish the structural adequacy of each bridge. The performance of each individual bridge is established by using the lowest of these four ratings. The use of these ratings, and the use of the lowest rating, is consistent with the approach used by the ADOT Bridge Group to assess the need for bridge
rehabilitation. The Bridge Index is calculated as a weighted average for each segment based on deck area.

Secondary Bridge Measures
Three secondary measures provide an in-depth evaluation of the characteristics of each bridge: Bridge Sufficiency

- Multipart rating includes structural adequacy and safety factors as well as functional aspects such as traffic volume and length of detour
- Rates the structural and functional sufficiency of each bridge on a 100 -point scale


## Bridge Rating

- The lowest rating of the four bridge condition ratings (substructure, superstructure, deck, and structural evaluation) on each segment
- Identifies lowest performing evaluation factor on each bridge Bridge Hot Spots
- A Bridge "hot spot" is identified where a given bridge has a bridge rating of 4 or lower or multiple ratings of 5 between the deck, superstructure, and substructure ratings
- Identifies particularly low-performing bridges or those that may decline to low performance in the immediate future

Bridge Performance Results
The Bridge Index provides a top-level assessment of the structural condition of bridges on the US 60|US 70|US 191 corridor, and for each corridor segment. The three secondary measures provide more detailed information to assess the bridge condition for each segment. A total of 48 major structures classified as bridges were included in the analysis. Major structures that are classified as Reinforced Concrete Box Culverts (RCBC) were not considered. Overall, based on the Bridge Index, all segments show "fair" performance.

- All the segments show "good" or "fair" performance ratings for the Bridge Index, which consists of the deck, substructure, superstructure and structural ratings. The ratings ranged from 5.0 to 7.0
- Segments 191-5 and 70-9 do not have any bridges.
- Bridge Sufficiency ratings per segment are either "good" or "fair". The weighted averaged values range from 52.90 to 95.57 out of 100 .
- Eleven bridges have been rated as structurally deficient with five of the bridges being in Segment 70|60-13.
- Queen Creek Tunnel (MP 228.47, No. 407) located on US 60 approximately 1.6 miles east of the SR 177 junction is a major feature on the corridor that was not evaluated within the performance framework for structural integrity (it is considered in freight performance for the vertical clearance secondary measure). This unique feature (located within Segment

60-14) will require isolated consideration throughout the Corridor Profile Study process to include its contribution to corridor condition and needs.

Table 7 summarizes the bridge performance results for the US 60|US $70 \mid$ US 191 corridor. Figure 10 illustrates the primary bridge index performance and locations of bridge hot spots along US 60|US 70|US 191. Maps for each secondary measure can be found in Appendix A.

Table 7: Bridge Performance

| Segment \# | Segment Length (miles) | \# of Bridges | Bridge Index | Sufficiency Rating | Lowest Bridge Rating |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 191-1 | 24 | 1 | 6.0 | 87.80 | 6 |
| 191-2 | 43 | 2 | 5.4 | 69.23 | 5 |
| 191-3 | 17 | 2 | 5.5 | 93.81 | 5 |
| 191-4 | 12 | 1 | 6.0 | 69.50 | 6 |
| 191-5 | 5 | 0 |  | No Bridges |  |
| 70-6 | 9 | 1 | 6.0 | 68.10 | 6 |
| 70-7 | 30 | 8 | 5.7 | 70.25 | 5 |
| 70-8 | 2 | 1 | 6.0 | 73.00 | 6 |
| 70-9 | 5 | 0 |  | No Bridges |  |
| 70-10 | 19 | 1 | 7.0 | 80.00 | 7 |
| 70-11 | 4 | 2 | 6.7 | 82.02 | 5 |
| 70-12 | 15 | 1 | 6.0 | 52.90 | 6 |
| 70\|60-13 | 12 | 11 | 5.2 | 78.01 | 4 |
| $60 \mathrm{E}-14$ | 16 | 6 | 5.5 | 68.13 | 3 |
| 60E-15 | 2 | 3 | 6.3 | 84.08 | 6 |
| 60E-16 | 2 | 2 | 5.0 | 86.43 | 5 |
| 60E-17 | 11 | 7 | 6.6 | 95.57 | 5 |
| 60E-18 | 7 | 8 | 5.9 | 90.24 | 5 |
| $60 \mathrm{E}-19$ | 6 | 6 | 5.9 | 91.43 | 5 |
| 60E-20 | 5 | 4 | 6.0 | 93.95 | 6 |
| Weighted Corridor Average |  |  | 5.82 | 81.95 | 4.87 |
| Scales |  |  |  |  |  |
| Performance Level |  |  | All |  |  |
| Good |  |  | > 6.5 | $>80$ | $>6$ |
| Fair |  |  | 5.0-6.5 | 50-80 | 5-6 |
| Poor |  |  | < 5.0 | < 50 | <5 |



### 2.4 Mobility Performance Area

The Mobility performance area consists of a primary measure (Mobility Index) and four secondary measures, as shown in Figure 11. These measures assess the condition of existing mobility along the US 60|US 70|US 191 corridor. The detailed calculations and equations developed for each measure are available in Appendix B and the performance data for this corridor is contained in Appendix C.

Figure 11: Mobility Performance Measures

## Mobility Performance Area



## Primary Mobility Index

The Mobility Index is an average of the existing (2019) daily volume-to-capacity (V/C) ratio and the future ( 2040 AZTDM) daily V/C ratio for each segment of the corridor. The V/C ratio is an indicator of the level of congestion. This measure compares the average annual daily traffic (AADT) volume to the capacity of the corridor segment as defined by the service volume for level of service (LOS) E. By using the average of the existing and future year daily volumes, this index measures the level of daily congestion projected to occur in approximately ten years (2030) if no capacity improvements are made to the corridor.
Each corridor segment is rated on a scale with other segments in similar operating environments. Within the Mobility performance area, the relevant operating environments are urban vs. rural setting and interrupted flow (e.g., signalized at-grade intersections are present) vs. uninterrupted flow (e.g., controlled access grade-separated conditions such as a freeway or interstate highway). For US 60|US 70|US 191, the following operating environments were identified:

Multimodal Opportunities - Three multimodal opportunity indicators reflect the characteristics of the corridor that promote alternate modes to the single occupancy vehicle (SOV) for trips along the corridor:

- \% Bicycle Accommodation
- Percentage of the segment that accommodates bicycle travel; bicycle accommodation on the roadway or on shoulders varies depending on traffic volumes, speed limits, and surface type
- Encouraging bicycle travel has the potential to reduce automobile travel, especially on non-interstate highways
- \% Non-SOV Trips:
- The percentage of trips (less than 50 miles in length) by non-SOVs
- The percentage of non-SOV trips in a corridor gives an indication of travel patterns along a section of roadway that could benefit from additional multimodal options
- \% Transit Dependency:
- The percentage of households that have zero or one automobile and households where the total income level is below the federally defined poverty level
- Used to track the level of need among those who are considered transit dependent and more likely to utilize transit if it is available


## Mobility Performance Results

The Mobility Index provides a high-level assessment of mobility conditions for the corridor and for each segment. The four secondary measures provide more detailed information to assess mobility performance.

Each corridor segment is rated on a scale with other segments in similar operating environments Within the mobility performance area, the relevant operating environments included urban or rural locations, as well as interrupted flow (where signalized at-grade intersections are present) and uninterrupted flow (grade-separated).
Based on the results of this analysis, the following observations were made:

- Overall, based on the weighted average of the Mobility Index, the traffic operations are in "good" condition. Segment 60-14, 60-19 and 60-20 are rated "poor" due to high V/C ratios.
- Existing peak hour traffic operations are "good" throughout the corridor, except for Segment 60-14, 60-19 and 60-20 which are rated as "poor" or "fair".
- Future traffic operations are anticipated to be "good" throughout the corridor, with the exception being "poor" or "fair" in Segment 60-14 and 60-18 through 60-20.
- Most of the corridor performed "good" in measuring closures for travel time reliability.

Segment 60-16 showed "fair" performance in the westbound direction, and Segments 70 60-13 and Segments 60-17 through 60-19 showed "fair" performance in the eastbound direction. Segment 60-15 showed "poor" performance in the eastbound direction, and Segment 60-14 showed "poor" performance in both the eastbound and westbound directions; Segment 60-20 showed "poor" performance in the westbound direction; Segments 60-14 and 60-15 showed "poor" performance in the eastbound direction

- The Directional Max LOTTR shows both directions perform at "good" or "fair". Segment 191-2 through Segment 70-12 have insufficient data.
- A majority of the corridor shows "poor" or "fair" performance for non-SOV trips meaning that many vehicles carry only a single occupant.
- Eleven segments show "poor" performance for accommodation of bicycles due to lack of sufficient shoulder width. Bicycle accommodation is "good" on Segments 191-2, 191-4, 60-15, 60-17, 60-18, 60-20 and "fair" for Segments 191-1, 70-7 and 60-16.

Table 8 summarizes the Mobility performance results for the US 60|US 70|US 191 corridor. Figure 12 illustrates the primary Mobility Index performance along the US 60|US 70|US 191 corridor. Maps for each secondary measure can be found in Appendix A.

Table 8: Mobility Performance

| Segment \# | Length (mi) | Mobility Index | Future V/C | Existing Peak Hour V/C |  | Closure Extent (instances/milepost/year/mile) |  | Directional LOTTR (all vehicles) |  | \% Bicycle Accommodation | \% Non-Single Occupancy Vehicle (SOV) Trips |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | NB/WB | SB/EB | NB/WB | SB/EB | NB/WB | SB/EB |  |  |
| 191-1 ${ }^{2^{*}}$ | 24 | 0.16 | 0.18 | 0.13 | 0.13 | 0.04 | 0.02 | 1.40 | 1.39 | 66\% | 15.0\% |
| 191-2 ${ }^{2^{*}}$ | 43 | 0.13 | 0.17 | 0.08 | 0.11 | 0.03 | 0.01 | N/A | N/A | 100\% | 16.6\% |
| $191-3^{2^{\wedge}}$ | 17 | 0.05 | 0.05 | 0.03 | 0.03 | 0.02 | 0.00 | N/A | N/A | 49\% | 8.8\% |
| 191-4 ${ }^{\text {^ }}$ | 12 | 0.17 | 0.19 | 0.11 | 0.11 | 0.08 | 0.07 | N/A | N/A | 97\% | 8.3\% |
| 191-5** | 5 | 0.27 | 0.30 | 0.15 | 0.16 | 0.20 | 0.20 | N/A | N/A | 27\% | 21.2\% |
| $70-6{ }^{1 *}$ | 9 | 0.41 | 0.45 | 0.31 | 0.29 | 0.02 | 0.04 | N/A | N/A | 46\% | 17.8\% |
| $70-7^{2 \wedge}$ | 30 | 0.18 | 0.20 | 0.11 | 0.10 | 0.04 | 0.01 | N/A | N/A | 73\% | 15.8\% |
| $70-8^{\text {2 }}$ | 2 | 0.11 | 0.12 | 0.08 | 0.05 | 0.10 | 0.00 | N/A | N/A | 0\% | 12.8\% |
| $70-9^{2^{\wedge}}$ | 5 | 0.24 | 0.26 | 0.16 | 0.12 | 0.04 | 0.04 | N/A | N/A | 26\% | 11.2\% |
| $70-10^{2^{\wedge}}$ | 19 | 0.15 | 0.17 | 0.11 | 0.08 | 0.07 | 0.05 | N/A | N/A | 4\% | 7.7\% |
| $70-11^{2^{\wedge}}$ | 4 | 0.18 | 0.20 | 0.13 | 0.10 | 0.00 | 0.00 | N/A | N/A | 4\% | 11.3\% |
| $70-12^{2^{\wedge}}$ | 15 | 0.24 | 0.27 | 0.16 | 0.17 | 0.17 | 0.00 | N/A | N/A | 23\% | 12.5\% |
| 70/60E-13 ${ }^{1 *}$ | 12 | 0.40 | 0.45 | 0.26 | 0.25 | 0.22 | 0.35 | 1.16 | 1.15 | 54\% | 16.6\% |
| $60 \mathrm{E}-14^{2 \wedge}$ | 16 | 1.42 | 1.71 | 0.79 | 1.14 | 0.67 | 1.84 | 1.12 | 1.17 | 49\% | 14.0\% |
| $60 \mathrm{E}-15^{2 \wedge}$ | 2 | 0.27 | 0.37 | 0.11 | 0.11 | 0.00 | 0.90 | 1.18 | 1.14 | 95\% | 10.5\% |
| $60 \mathrm{E}-16^{2^{\wedge}}$ | 2 | 0.27 | 0.38 | 0.16 | 0.16 | 0.60 | 0.15 | 1.05 | 1.12 | 87\% | 7.7\% |
| $60 \mathrm{E}-17^{2^{\wedge}}$ | 11 | 0.26 | 0.37 | 0.15 | 0.14 | 0.04 | 0.23 | 1.05 | 1.09 | 96\% | 8.9\% |
| $60 \mathrm{E}-18^{2^{\wedge}}$ | 7 | 0.53 | 0.66 | 0.30 | 0.32 | 0.00 | 0.23 | 1.12 | 1.05 | 100\% | 12.0\% |
| 60E-19 ${ }^{1 *}$ | 6 | 1.01 | 0.86 | 0.86 | 0.91 | 0.10 | 0.30 | 1.20 | 1.14 | 42\% | 17.8\% |
| 60E-20 ${ }^{1 \wedge}$ | 5 | 1.31 | 1.45 | 0.84 | 0.88 | 0.68 | 0.09 | 1.06 | 1.06 | 100\% | 17.2\% |
| Weighted Corridor Average |  | 0.32 | 0.37 | 0.20 | 0.23 | 0.12 | 0.19 | 1.20 | 1.20 | 63\% | 14\% |
| SCALES |  |  |  |  |  |  |  |  |  |  |  |
| Performance Level |  | Urban Rural |  |  |  | All |  | Uninterrupted Interrupted |  | All |  |
| Good |  | $\begin{aligned} & <0.71 \\ & <0.56 \end{aligned}$ |  |  |  | $<0.22$ |  | $\begin{aligned} & <1.15 \\ & <1.20 \end{aligned}$ |  | > 17\% | 90\% |
| Fair |  | $\begin{aligned} & 0.71-0.89 \\ & 0.56-0.76 \end{aligned}$ |  |  |  | 0.22-0.62 |  | $\begin{aligned} & 1.15-1.33 \\ & 1.30-2.00 \end{aligned}$ |  | 11\%-17\% | 90\%-60\% |
| Poor |  | $\begin{aligned} & >0.89 \\ & >0.76 \end{aligned}$ |  |  |  | $>0.62$ |  | $\begin{array}{r} >1.33 \\ >2.00 \end{array}$ |  | < 11\% | 60\% |

Urban Operating Environment ^ Uninterrupted
${ }^{2}$ Rural Operating Environment *Interrupted

Figure 12: Mobility Performance


### 2.5 Safety Performance Area

The Safety performance area consists of a primary measure (Safety Index) and four secondary measures, as illustrated in Figure 13. All measures relate to crashes that result in fatal and suspected serious injuries, as these types of crashes are the emphasis of the ADOT Strategic Traffic Safety Plan (STSP), FHWA, and MAP-21. The detailed calculations and equations developed for each measure are available in Appendix B and the performance data for this corridor is contained in Appendix C.

Figure 13: Safety Performance Measures


Primary Safety Index
The Safety Index is based on the bi-directional frequency and rate of fatal and suspected serious injury crashes, the relative cost of those types of crashes, and crash occurrences on similar roadways in Arizona. According to ADOT’s 2018 Highway Safety Improvement Program Application, fatal crashes have an estimated cost that is 17.3 times the estimated cost of incapacitating injury crashes ( $\$ 9.5$ million compared to $\$ 555,000$ ).
Each corridor segment is rated on a scale by comparing the segment score with the average statewide score for similar operating environments. Since crash frequencies and rates vary depending on the operating environment of a particular roadway, statewide values were developed for similar operating environments defined by functional classification, urban vs. rural setting, number of travel lanes, and traffic volumes.

For US 60|US 70|US 191, the following operating environments were identified:

- 2 or 3 Lane Undivided Highway (Segments 1-2; 4; 7-12; 14-16)
- 4 or 5 Lane Undivided Highway (Segments 5-6; 13)
- 2, 3 or 4 Lane Divided Highway (Segments 3; 17-19)
- 4 Lane Freeway (Segment 20)


## Secondary Measures

Four secondary measures provide an in-depth evaluation of the different characteristics of safety performance:

Directional Safety Index

- This measure is based on the directional frequency and rate of fatal and incapacitating injury crashes

STSP Emphasis Areas
ADOT's 2019 STSP identified several emphasis areas for reducing fatal and suspected serious injury crashes. This measure compared rates of crashes in three STSP emphasis areas to other corridors with a similar operating environment. The three STSP emphasis areas related to crashes involving:

- Intersections
- Lane departures
- Pedestrians

Other Crash Unit Types

- The percentage of total fatal and suspected serious injury crashes that involves crash unit types of trucks and bicycles is compared to the statewide average on roads with similar operating environments


## Safety Hot Spots

- The hot spot analysis identifies abnormally high concentrations of fatal and suspected serious injury crashes along the study corridor by direction of travel
For the Safety Index and the secondary safety measures, any segment that has too small of a sample size to generate statistically reliable performance ratings for a particular performance measure is considered to have "insufficient data" and is excluded from the safety performance evaluation for that particular performance measure.

The Safety Index provides a high-level assessment of safety performance for the corridor and for each segment. The four secondary measures provide more detailed information to assess safety performance.
The scale for ratings for all of the Safety performance measures depend on the crash history on similar statewide operating environments. Based on the results of this analysis, the following observations were made:

- Overall, based on the weighted average of the Safety Index, the corridor rates in "average performance" condition
- Four segments have insufficient data to determine the Safety Index.
- Eight of the segments perform above average and the remaining are "below average performance" or "average performance" in the Safety Index.
- Nine of the segments perform "below average" or "average" in the NB/WB Directional Safety Index.
- Five of the segments perform "below average" or "average" in the SB/EB Directional Safety Index.
- Most of the segments have insufficient data to assess the percent of fatal and incapacitating injury crashes per crash unit type except for crashes involving Lane Departures which shows the weighted corridor average is "above average."
- Segments 60-14 and 60-17 show the Percentage of Fatal and Incapacitating Crashes Involving Lane Departures was "above average."

Table 9 summarizes the Safety performance results for the US 60|US $70 \mid$ US 191 corridor. Figure 14 illustrates the primary Safety Index performance and locations of Safety hot spots along the US 60|US 70|US 191 corridor. Maps for each secondary measure can be found in Appendix A.

Table 9: Safety Performance

| Segment | Segment Length (miles) | Overall Safety Index | NB/WB Directional Safety Index | SB/EB Directional Safety Index | \% of Fatal + Suspected Serious Injury Crashes at Intersections | \% of Fatal + Suspected Serious Injury Crashes Involving Lane Departures | \% of Fatal + Suspected Serious Injury Crashes Involving Pedestrians | \% of Segment Fatal + Suspected Serious Injury Crashes Involving Trucks | \% of Segment Fatal <br> + Suspected Serious Injury Crashes Involving Bicycles |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 191-1 | 24 | 0.39 | 0.04 | 0.73 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data |
| 191-2 | 43 | 0.49 | 0.54 | 0.44 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data |
| 191-3 | 17 | 0.59 | 0.00 | 1.18 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data |
| 191-4 | 12 | 0.58 | 1.06 | 0.11 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data |
| 191-5 | 5 | 0.06 | 0.12 | 0.00 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data |
| 70-6 | 9 | 0.38 | 0.67 | 0.08 | Insufficient Data | 25\% | Insufficient Data | Insufficient Data | Insufficient Data |
| 70-7 | 30 | 0.68 | 0.89 | 0.48 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data |
| 70-8 | 2 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data |
| 70-9 | 5 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data |
| 70-10 | 19 | 1.63 | 0.76 | 2.50 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data |
| 70-11 | 4 | 3.37 | 6.74 | 0.00 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data |
| 70-12 | 15 | 2.63 | 2.97 | 2.28 | Insufficient Data | 22\% | Insufficient Data | Insufficient Data | Insufficient Data |
| 60\|70-13 | 12 | 2.97 | 3.36 | 2.57 | Insufficient Data | 21\% | Insufficient Data | Insufficient Data | Insufficient Data |
| 60E-14 | 16 | 1.78 | 1.50 | 2.07 | Insufficient Data | 81\% | Insufficient Data | Insufficient Data | Insufficient Data |
| 60E-15 | 2 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data |
| 60E-16 | 2 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data |
| 60E-17 | 11 | 1.23 | 1.82 | 0.65 | Insufficient Data | 78\% | Insufficient Data | Insufficient Data | Insufficient Data |
| 60E-18 | 7 | 0.50 | 0.91 | 0.09 | Insufficient Data | 17\% | Insufficient Data | Insufficient Data | Insufficient Data |
| 60E-19 | 6 | 0.95 | 1.62 | 0.27 | Insufficient Data | 60\% | Insufficient Data | Insufficient Data | Insufficient Data |
| 60E-20 | 5 | 1.29 | 1.89 | 0.69 | Insufficient Data | 50\% | Insufficient Data | Insufficient Data | Insufficient Data |
| Weighted | Average | 1.06 | 1.13 | 0.99 | Insufficient Data | 45\% | Insufficient Data | Insufficient Data | Insufficient Data |
| SCALES |  |  |  |  |  |  |  |  |  |
| Performance LevelGood/Above Average Performance |  | 2 or 3 or 4 Lane Divided Highway |  |  |  |  |  |  |  |
|  |  |  | <0.81 |  | <23.4\% | < 56.4\% | <2.4\% | <3.7\% | < 0\% |
| Fair/Ave | mance |  | 0.81-1.19 |  | 23.4\%-29.3\% | 56.4\%-65.0\% | 2.4\%-3.6\% | 3.7\%-3.9\% | 0\%-2.2\% |
| Poor/Below | formance |  | > 1.19 |  | > 29.3\% | > 65.0\% | > 3.6\% | 3.90\% | $>2.2 \%$ |
| Performance Level |  | 2 or 3 Lane Undivided Highway |  |  |  |  |  |  |  |
| Good/Above Average Performance |  | < 0.92 |  |  | < 11.2\% | < 66.9\% | < 3.8\% | <4.2\% | < 0\% |
| Fair/Average Performance |  | 0.92-1.08 |  |  | 11.2\%-15.6\% | 66.9\%-74.5\% | 3.8\%-7.2\% | 4.2\%-8.0\% | 0\%-3.3\% |
| Poor/Below Average Performance |  | > 1.08 |  |  | > 15.6\% | > 74.5\% | > 7.2\% | > 8.0\% | > $3.3 \%$ |
| Performance Level |  | 4 or 5 Undivided Highway |  |  |  |  |  |  |  |
| Good/Above Average Performance |  | < 0.78 |  |  | < 43.8\% | <21.1\% | < 8.8\% | <0.8\% | <0.5\% |
| Fair/Average Performance |  | 0.78-1.22 |  |  | 43.8\% - 49.5\% | 21.1\%-32.1\% | 8.8\%-13.5\% | 0.8\%-5.5\% | 0.5\%-3.8\% |
| Poor/Below Average Performance |  | >1.22 |  |  | > 49.5\% | $>32.1 \%$ | > 13.5\% | > 5.5\% | > $3.8 \%$ |
| Performance Level |  | Urban 4 Lane Freeway |  |  |  |  |  |  |  |
| Good/Above Average Performance |  | < 0.73 |  |  | = 0.0\% | < 60.6\% | = 0\% | <6.9\% | = 0.0\% |
| Fair/Average Performance |  |  |  |  |  | 60.6\% - 78.1\% | 0.0\% - 4.9\% | 6.9\%-12.4\% | $>0.0 \%$ |
| Poor/Below | rformance | $\begin{gathered} 0.13-1.27 \\ >1.27 \end{gathered}$ |  |  | > 0.0\% | > 78.1\% | > 4.9\% | > 12.4\% |  |

64 or 5 Lane Undivided Highway, ${ }^{\text {c } 2}$ or 3 Lane Undivided Highway
Note: "Insufficient Data" indicates there was not enough data available to generate reliable performance ratings

## Figure 14: Safety Performance



### 2.6 Freight Performance Area

The Freight performance area consists of a single primary measure (Freight Index) and three secondary measures, as illustrated in Figure 15. All measures related to the reliability of truck travel as measured by observed truck travel time speed and delays to truck travel from freeway closures or physical restrictions to truck travel. The detailed calculations and equations developed for each measure are available in Appendix B and the performance data for this corridor is contained in Appendix C.

Figure 15: Freight Performance Measures


Primary Freight Index
The Freight Index is a reliability performance measure based on the travel time reliability for truck travel. The Truck Travel Time Reliability (TTTR) is the ratio of the $95^{\text {th }}$ percentile truck travel time to average ( $50^{\text {th }}$ percentile) truck travel time. The TTTR reflects the extra buffer time needed for on-time delivery while accounting for delay resulting from circumstances such as recurring congestion, crashes, inclement weather, and construction activities.
Each corridor segment is rated on a scale with other segments in similar operating environments. Within the Freight performance area, the relevant operating environments are interrupted flow (e.g., signalized at-grade intersections are present) and uninterrupted flow (e.g., controlled access grade-separated conditions such as a freeway or interstate highway).
For US 60|US 70|US 191, the following operating environments were identified:

- Urban Interrupted (Segments 5-6; 13, 19)
- Urban Uninterrupted (Segment 20)
- Rural Uninterrupted (Segments 3-4; 7-12; 14-18)
- Rural Interrupted (Segments 1-2)

The Freight performance area includes three secondary measures that provide an in-depth evaluation of the different characteristics of freight performance:
Travel Time Reliability - Two separate travel time reliability indicators together provide a comprehensive picture of how much time may be required to travel within the corridor:

- Directional Truck Travel Time Reliability (TTTR):
- The ratio of the $95^{\text {th }}$ percentile truck travel time to average ( $50^{\text {th }}$ percentile) truck travel time for a given corridor segment in a specific direction; as corridor segments were often comprised of multiple roadway sections for which TTTR was reported, a weighted average was applied to each section based on the section length in order to arrive at the segment TTTR
- Directional Closure Duration
- The average time (in minutes) a particular milepost is closed per year per mile on a given segment of the corridor in a specific direction of travel; a weighted average is applied to each closure that takes into account the distance over which the closure occurs


## Bridge Vertical Clearance

- The minimum vertical clearance (in feet) over the travel lanes for underpass structures on each segment

Bridge Vertical Clearance Hot Spots

- A Bridge vertical clearance "hot spot" exists where the underpass vertical clearance over the mainline travel lanes is less than 16.25 feet and no exit/entrance ramps exist to allow vehicles to bypass the low clearance location
- If a location with a vertical clearance less than 16.25 feet can be avoided by using immediately adjacent exit/entrance ramps rather than the mainline, it is not considered a hot spot
Freight Performance Results
The Freight Index provides a high-level assessment of freight mobility for the corridor and for each segment. The three secondary measures provide more detailed information to assess freight performance.
Each corridor segment is rated on a scale with other segments in similar operating environments. Within the freight performance area, the relevant operating environments included interrupted
flow (where signalized at-grade intersections are present) and uninterrupted flow (roads with only controlled access grade-separated conditions such as a freeway or interstate highway).

Based on the results of this analysis, the following observations were made:

- Overall, based on the weighted average of the Freight Index, the freight mobility is in "poor" condition, although eleven segments did not have a calculated Freight Index due to lack of data. All segments show "fair" performance in Freight TTTR except for Segments 1911 and 60-14 which show "poor" performance.
- The Directional Max TTTR shows the weighted corridor average for NB/WB travel has "poor" performance while the SB/EB travel has "fair" performance.
- For Closure Duration, most of the corridor performed "good" with just a few Segments showing "fair" performance. Only 60-14 showed "poor" performance, and only in the westbound direction, having the highest average durations of closures.
- Two locations have vertical clearance restrictions that cannot be by-passed, including one bridge in Segment 70|60-13 and the Queen Creek Tunnel in Segment 60-14
Table 10 summarizes the Freight performance results for the US 60|US 70|US 191 corridor. Figure 16 illustrates the primary Freight Index performance and locations of freight hot spots along US 60|US 70|US 191. Maps for each secondary measure can be found in Appendix A.

Table 10: Freight Performance

| Segment \# | Freight TTTR | Directional Max TTTR |  | Combined Average Peak TTTR | Average Minutes Per Year Given Milepost Is Closed Per Segment Mile |  | Bridge Vertical Clearance (feet) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | NB/WB | SB/EB |  | NB/WB | SB/EB |  |
| 191-1* | 2.26 | 2.52 | 2.00 | 2.26 | 3.02 | 1.00 | No UP |
| 191-2* | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 2.67 | 1.78 | 22.04 |
| 191-3^ | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 2.47 | 0.00 | No UP |
| 191-4^ | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 12.23 | 5.00 | No UP |
| 191-5* | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 26.08 | 16.96 | None |
| 70-6* | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 1.33 | 4.67 | No UP |
| 70-7^ | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 4.55 | 5.40 | 17.03 |
| 70-8^ | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 14.30 | 0.00 | No UP |
| 70-9^ | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 2.40 | 3.00 | None |
| 70-10^ | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 8.63 | 2.51 | No UP |
| 70-11^ | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 0.00 | 0.00 | No UP |
| 70-12^ | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 17.39 | 0.00 | No UP |
| 70/60E-13* | 1.58 | 1.67 | 1.49 | 1.58 | 22.75 | 26.52 | 15.84 |
| 60E-14^ | 1.49 | 1.52 | 1.46 | 1.49 | 63.60 | 344.95 | 13.03 |
| 60E-15^ | 1.32 | 1.34 | 1.29 | 1.32 | 0.00 | 90.50 | 16.79 |
| 60E-16^ | 1.28 | 1.14 | 1.42 | 1.28 | 52.20 | 12.25 | No UP |
| 60E-17^ | 1.18 | 1.15 | 1.20 | 1.18 | 3.27 | 61.40 | No UP |
| $60 \mathrm{E}-18^{\wedge}$ | 1.22 | 1.32 | 1.13 | 1.22 | 0.00 | 22.29 | No UP |
| 60E-19* | 1.63 | 1.74 | 1.52 | 1.63 | 14.00 | 20.30 | No UP |
| $60 \mathrm{E}-20^{\wedge}$ | 1.20 | 1.25 | 1.14 | 1.20 | 74.94 | 7.11 | No UP |
| Weighted Corridor Average | 1.64 | 1.75 | 1.54 | 1.64 | 12.16 | 30.69 | 18.96 |
| SCALE |  |  |  |  |  |  |  |
| $\begin{array}{\|c\|} \hline \text { Performance } \\ \text { Level } \end{array}$ | Uninterrupted: Segments 3-4; 7-12; 14-18,20 Interrupted: Segments 1-2; 5-6; 13,19 |  |  |  | All |  |  |
| Good | $\begin{aligned} & <1.15 \\ & <1.45 \\ & \hline \end{aligned}$ |  |  |  | < 44.18 |  | > 16.5 |
| Fair | $\begin{aligned} & 1.15-1.35 \\ & 1.45-1.85 \end{aligned}$ |  |  |  | 44.18-124.86 |  | 16.0-16.5 |
| Poor | $\begin{aligned} & >1.35 \\ & >1.85 \end{aligned}$ |  |  |  | > 124.86 |  | < 16.0 |
| ${ }^{\wedge}$ Uninterrupted ${ }^{*}$ Interrupted |  |  |  |  |  |  |  |

Figure 16: Freight Performance


### 2.7 Corridor Performance Summary

Based on the results presented in the preceding sections, the following general observations were made related to the performance of the US 60|US 70|US 191 corridor:

- Overall performance within all five areas evaluated is split between "good" (40\%), "fair" (35\%) and "poor" (25\%) ratings.
- Pavement Performance: The Pavement Index shows a mix of "poor," "fair" and "good" performance. Fifty-seven out of 246 miles of the corridor show Pavement Index performance as "good."
- Bridge Performance: A total of 67 bridges were included in the evaluation. Eleven bridges on US 60 are considered structurally deficient in which five fall in Segment 70|60 - 13.
- Mobility Performance: US 60 |US $70 \mid$ US 191 corridor is considered to have two operating environments for evaluating Mobility. These include Urban/Fringe Highway and Rural Highway. Both the current and future capacity is considered "good" with the exception of Segments 60-14 and 60-18 through 60-20.
- Safety Performance: Safety performance utilizes the four operating environments for analysis that compare fatal and incapacitating injury crashes to other similar routes statewide. The US 60|US $70 \mid$ US 191 corridor is mixed of "good," "fair" and "poor" ratings. The Safety Index for Segments 70-7, 70-10 through 70-14, 60-17 and 60-20 show "poor" performance.
- Freight Performance: The performance of freight mobility is overall "poor" within the US 60|US 70|US 191 corridor. This is primarily due to the high TTTR. Traffic counters do not exist in 11 of the 20 segments, which does not allow for the performance to be measured for TTTR for much of the corridor.
- Poorest Performing Segment: Segment 60-14 rated lower in performance than the other segments in the corridor. Mobility, Safety and Freight measures all rated as "poor" performance. Pavement and Bridge had mixes of "good," "fair" and poor performance.
- Highest Performing Segments: Segment 60-18 has only one performance metric that has "poor" performance which is Percent Area Failure.

Figure 17 shows the percentage of the US 60|US 70|US 191 corridor that rates either "good/above average performance", "fair/average performance", or "poor/below average performance" in each Index.

Table 11 shows a summary of all primary and secondary performance measures for the US 60|US 70 US 191 corridor. A weighted average rating (based on the length of the segment) was calculated for each primary and secondary measure - this is shown in the last row of Table 11 The weighted average ratings are summarized in Figure 18 which also provides a brief description of each performance measure. Figure 18 represents the average for the entire corridor, and any given segment or location could have a higher or lower rating than the corridor average.

Figure 17: Performance Summary by Primary Measure


Figure 18: Corridor Performance Summary by Performance Measure

| Pavement | Bridge | Mobility | Safety | Freight |
| :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |
| Pavement Index (PI): based on three pavement condition ratings from the ADOT Pavement Database; the three ratings are the International Roughness Index (IRI), the Cracking Rating, and the Rutting Rating | Bridge Index (BI): based on four bridge condition ratings from the ADOT Bridge Database; the four ratings are the Deck Rating, Substructure Rating, Superstructure Rating, and Structural Evaluation Rating | Mobility Index (MI): an average of the existing daily volume-to-capacity (V/C) ratio and the projected long-term future daily V/C ratio | Safety Index (SI): combines the bidirectional frequency and rate of fatal and suspected serious injury crashes, compared to crash occurrences on roads with similar operating environments in Arizona | Freight Index (FI): a reliability performance measure based on the bi-directional Truck Travel Time Reliability (TTTR) for truck travel |
| > Directional Pavement Serviceability Rating (PSR) - the weighted average (based on number of lanes) of the PSR for the pavement in each direction of travel <br> > \% Area Failure - the percentage of pavement area rated above failure thresholds for IRI or Cracking | > Sufficiency Rating- multipart rating includes structural adequacy and safety factors as well as functional aspects such as traffic volume and length of detour <br> > Lowest Bridge Rating -the lowest rating of the four bridge condition ratings on each segment | > Future Daily V/C - the future daily V/C ratio provides a measure of future congestion if no capacity improvements are made to the corridor <br> > Existing Peak Hour V/C - the existing peak hour V/C ratio for each direction of travel provides a measure of existing peak hour congestion during typical weekdays <br> > Closure Extent - the average number of instances a particular milepost is closed per year per mile on a given segment of the corridor in a specific direction of travel <br> - Directional Level of Travel Time Reliability (LOTTR) - the ratio of the $80^{\text {th }}$ percentile peak period travel time to the $50^{\text {th }}$ percentile peak period travel time for all vehicles <br> > \% Bicycle Accommodation - the percentage of a segment that accommodates bicycle travel <br> > \% Non-Single Occupancy Vehicle (NonSOV) Trips -the percentage of trips that are taken by vehicles carrying more than one occupant | > Directional Safety Index - the combination of the directional frequency and rate of fatal and suspected serious injury crashes, compared to crash occurrences on roads with similar operating environments in Arizona \% of Fatal + Suspected Serious Injury Crashes Involving Lane Departures the percentage of total fatal and suspected serious injury crashes involving lane departures compared to the statewide average percentage on roads with similar operating environments | $>$ Directional TTTR - the ratio of the $95^{\text {th }}$ percentile peak period travel time to the $50^{\text {th }}$ percentile peak period travel time for trucks <br> > Closure Duration - the average time a particular milepost is closed per year per mile on a given segment of the corridor in a specific direction of travel <br> > Bridge Vertical Clearance - the minimum vertical clearance over the travel lanes for underpass structures on each segment. |

Table 11: Corridor Performance Summary by Segment and Performance Measure

| Segment \# | Segment Length (miles) | Pavement Performance Area |  |  |  | Bridge Performance Area |  |  | Mobility Performance Area |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Pavement Index | Directional PSR |  | \% Area Failure | Bridge Index | Sufficiency Rating | Lowest <br> Bridge <br> Rating | Mobility Index | Future Daily V/C | Existing Peak Hour V/C |  | Closure Extent (instances/ milepost/year/mile) |  | Directional Max LOTTR (all vehicles) |  | \% Bicycle Accommodation | \% Non-SingleOccupancyVehicle(SOV) Trips |
|  |  |  | NB/EB | SB/WB |  |  |  |  |  |  | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB |  |  |
| 191-12* | 24 | 3.17 | 3.10 | 3.24 | 70.8\% | 6.0 | 87.80 | 6 | 0.16 | 0.18 | 0.13 | 0.13 | 0.04 | 0.02 | 1.40 | 1.39 | 66\% | 15.0\% |
| 191-2 ${ }^{2^{*}}$ | 43 | 2.89 | 3.44 | 3.38 | 55.8\% | 5.4 | 69.23 | 5 | 0.13 | 0.17 | 0.08 | 0.11 | 0.03 | 0.01 | Insuf | Data | 100\% | 16.6\% |
| 191-3 ${ }^{\text { }}$ | 17 | 3.42 | 3.63 | 3.69 | 72.0\% | 5.5 | 93.81 | 5 | 0.05 | 0.05 | 0.03 | 0.03 | 0.02 | 0.00 | Insuff | Data | 49\% | 8.8\% |
| $191-4^{\text {2 }}$ | 12 | 3.44 | 3.29 | 3.32 | 41.7\% | 6.0 | 69.50 | 6 | 0.17 | 0.19 | 0.11 | 0.11 | 0.08 | 0.07 | Insuff | Data | 97\% | 8.3\% |
| 191-5 ${ }^{\text {* }}$ | 5 | 3.10 | 3.16 | 3.07 | 80.0\% | No Bridges |  |  | 0.27 | 0.30 | 0.15 | 0.16 | 0.20 | 0.20 |  |  | 0.20 | 21.2\% |
| $70-6{ }^{1 *}$ | 9 | 3.23 | 3.15 | 3.25 | 60.0\% | 6.0 | 68.10 | 6 | 0.41 | 0.45 | 0.31 | 0.29 | 0.02 | 0.04 | Insuff | Data | 46\% | 17.8\% |
| $70-7^{\text {2 }}$ | 30 | 2.83 | 2.87 | 3.08 | 86.8\% | 5.7 | 70.25 | 5 | 0.18 | 0.20 | 0.11 | 0.10 | 0.04 | 0.01 | Insuff | Data | 73\% | 15.8\% |
| $70-8^{\text {2 }}$ | 2 | 2.59 | 3.35 | 3.67 | 100.0\% | 6.0 | 73.00 | 6 | 0.11 | 0.12 | 0.08 | 0.05 | 0.10 | 0.00 | Insuff | Data | 0\% | 12.8\% |
| $70-9^{\text {2 }}$ | 5 | 2.71 | 3.44 | 3.63 | 100.0\% | No Bridges |  |  | 0.24 | 0.26 | 0.16 | 0.12 | 0.04 | 0.04 | 0.04 |  | 0.04 | 11.2\% |
| $70-10^{2 \wedge}$ | 19 | 2.69 | 3.10 | 3.35 | 78.9\% | 7.0 | 80.00 | 7 | 0.15 | 0.17 | 0.11 | 0.08 | 0.07 | 0.05 | Insufficient Data |  | 4\% | 7.7\% |
| $70-11^{2^{\wedge}}$ | 4 | 2.40 | 3.27 | 3.28 | 87.5\% | 6.7 | 82.02 | 5 | 0.18 | 0.20 | 0.13 | 0.10 | 0.00 | 0.00 | Insufficient Data |  | 4\% | 11.3\% |
| $70-12^{2^{\wedge}}$ | 15 | 3.57 | 3.28 | 3.53 | 33.3\% | 6.0 | 52.90 | 6 | 0.24 | 0.27 | 0.16 | 0.17 | 0.17 | 0.00 | Insufficient Data |  | 23\% | 12.5\% |
| 70\|60-13 ${ }^{\text {** }}$ | 12 | 3.28 | 3.13 | 3.28 | 53.8\% | 5.2 | 78.01 | 4 | 0.40 | 0.45 | 0.26 | 0.25 | 0.22 | 0.35 | 1.16 | 1.15 | 54\% | 16.6\% |
| $60 \mathrm{E}-14^{2^{\wedge}}$ | 16 | 3.68 | 3.66 | 3.82 | 43.8\% | 5.5 | 68.13 | 3 | 1.42 | 1.71 | 0.79 | 1.14 | 0.67 | 1.84 | 1.12 | 1.17 | 49\% | 14.0\% |
| $60 \mathrm{E}-15^{2^{\wedge}}$ | 2 | 4.03 | 3.70 | 3.65 | 0.0\% | 6.3 | 84.08 | 6 | 2.80 | 3.90 | 1.13 | 1.12 | 0.00 | 0.90 | 1.18 | 1.14 | 95\% | 10.5\% |
| $60 \mathrm{E}-16^{2^{\wedge}}$ | 2 | 4.50 | 4.22 | 4.15 | 0.0\% | 5.0 | 86.43 | 5 | 0.73 | 1.01 | 0.42 | 0.42 | 0.60 | 0.15 | 1.05 | 1.12 | 87\% | 7.7\% |
| $60 \mathrm{E}-17^{2 \wedge}$ | 11 | 3.51 | 3.93 | 3.99 | 76.2\% | 6.6 | 95.57 | 5 | 0.26 | 0.37 | 0.15 | 0.14 | 0.04 | 0.23 | 1.05 | 1.09 | 96\% | 8.9\% |
| $60 \mathrm{E}-18^{2^{\wedge}}$ | 7 | 3.30 | 3.62 | 3.83 | 92.9\% | 5.9 | 90.24 | 5 | 0.53 | 0.66 | 0.30 | 0.32 | 0.00 | 0.23 | 1.12 | 1.05 | 100\% | 12.0\% |
| $60 \mathrm{E}-19^{1 *}$ | 6 | 3.57 | 3.57 | 3.65 | 33.3\% | 5.9 | 91.43 | 5 | 1.01 | 0.86 | 0.86 | 0.91 | 0.10 | 0.30 | 1.20 | 1.14 | 42\% | 17.8\% |
| $60 \mathrm{E}-20^{1 \wedge}$ | 5 | 4.17 | 3.87 | 3.83 | 0.0\% | 6.0 | 93.95 | 6 | 1.31 | 1.45 | 0.84 | 0.88 | 0.68 | 0.09 | 1.06 | 1.06 | 100\% | 17.2\% |
| Weighted <br> Aver | Corridor age | 3.18 | 3.33 | 3.44 | 63\% | 5.82 | 81.95 | 4.87 | 0.34 | 0.40 | 0.22 | 0.24 | 0.12 | 0.19 | 0.19 | 1.20 | 63\% | 13.7\% |
| SCALE |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Performan | ce Level | Non-Interstate |  |  |  | All |  |  | Urban Rural ${ }^{2}$ |  |  |  | All |  | Uninterrupted ^ Interrupted |  |  | All |
| Good / Abo | e Average | > 3.50 |  |  | < $5 \%$ | > 6.5 | $>80$ | > 6 | $\leq 0.71$ (Urban)$<0.56$ (Rural) |  |  |  | < 0.22 |  | $\begin{aligned} & \leq 1.15 \\ & \leq 1.15 \end{aligned}$ |  | > 90\% | > 17\% |
| Fair / Average |  | 2.9-3.5 |  |  | 5\%-20\% | 5.0-6.5 | 50-80 | 5-6 | 0.71-0.89 (Urban) <br> 0.56-0.76 (Rural) |  |  |  | 0.22-0.62 |  | $\begin{aligned} & 1.15-1.5 \\ & 1.15-1.5 \end{aligned}$ |  | 90\% - 60\% | 17\% - 11\% |
| Poor / Average |  | <2.90 |  |  | > 20\% | < 5.0 | < 50 | < 5 | $\begin{aligned} & >0.89 \text { (Urban) } \\ & >0.76 \text { (Rural) } \end{aligned}$ |  |  |  | $\geq 0.62$ |  | $\begin{array}{r} \geq 1.5 \\ \geq 1.5 \\ \hline \end{array}$ |  | < 60\% | < 11\% |

${ }^{1}$ Urban or Fringe Urban Operating Environment
${ }^{2}$ Rural Operating Environment
${ }^{\wedge}$ Uninterrupted

* Interrupted

Table 11: Corridor Performance Summary by Segment and Performance Measure (continued)

| Segment <br> \# | Segment Length (miles) | Safety Performance Area |  |  |  |  |  |  |  | Freight Performance Area |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Safety Index | Directional Safety Index |  | \% of Fatal + Suspected Serious Injury Crashes at Intersections | \% of Fatal + <br> Suspected Serious Injury Crashes Involving Lane Departures | \% of Fatal + Suspected Serious Injury Crashes Involving Pedestrians | $\%$ of Segment <br> Fatal + <br> Suspected <br> Serious Injury <br> Crashes <br> Involving Trucks | \% of Segment <br> Fatal + <br> Suspected <br> Serious Injury <br> Crashes <br> Involving <br> Bicycles | Freight TTTR | Directional Max TTTR |  | Combined Average Peak TTTR | Average Minutes Per Year Given Milepost Is Closed Per Segment Mile (NB/EB) |  | Bridge Vertical Clearance (feet) |
|  |  |  | NB/EB | SB/WB |  |  |  |  |  |  | NB/EB | $\begin{gathered} \text { SB/W } \\ \text { B } \end{gathered}$ |  | NB/EB | SB/WB |  |
| 191-12* | 24 | 0.39 | 0.04 | 0.73 | $\begin{gathered} \hline \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 2.26 | 2.52 | 2.00 | 2.26 | 3.02 | 1.00 | No UP |
| $191-2^{2^{*}}$ | 43 | 0.49 | 0.54 | 0.44 | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | Insufficient Data | Insufficient Data | Insufficient Data | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | Insufficient Data | $\begin{array}{r} \text { Insu } \\ \hline \end{array}$ |  | Insufficient Data | 2.67 | 1.78 | 22.04 |
| $191-3^{2^{\wedge}}$ | 17 | 0.59 | 0.00 | 1.18 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insuffic |  | Insufficient Data | 2.47 | 0.00 | No UP |
| $191-4^{2^{\wedge}}$ | 12 | 0.58 | 1.06 | 0.11 | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | Insufficient Data | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | Insufficient Data | Insufficient Data | Insuffi Da |  | Insufficient Data | 12.23 | 5.00 | No UP |
| 191-5 ${ }^{1 *}$ | 5 | 0.06 | 0.12 | 0.00 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | Insufficient Data | Insuffi |  | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | 26.08 | 16.96 | None |
| $70-6{ }^{1 *}$ | 9 | 0.38 | 0.67 | 0.08 | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \\ & \hline \end{aligned}$ | 25\% | Insufficient Data | $\begin{gathered} \hline \text { Insufficient } \\ \text { Data } \end{gathered}$ | $\begin{gathered} \hline \text { Insufficient } \\ \text { Data } \end{gathered}$ | Insufficient Data | Insuffi Da |  | Insufficient Data | 1.33 | 4.67 | No UP |
| 70-7 ${ }^{\text {2 }}$ | 30 | 1.08 | 1.41 | 0.75 | $\begin{gathered} \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | Insufficient Data | Insufficient Data | Insufficient Data | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | Insufficient Data | $\begin{gathered} \text { Insuffi } \\ \text { Da } \end{gathered}$ |  | $\begin{gathered} \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | 4.55 | 5.40 | 17.03 |
| 70-8 ${ }^{\text {2^ }}$ | 2 | Insufficient Data | $\qquad$ | $\begin{gathered} \hline \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | $\begin{gathered} \hline \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | Insufficient Data | Insufficient Data | $\begin{array}{r} \text { Insuffi } \\ \mathrm{Da} \end{array}$ |  | Insufficient Data | 14.30 | 0.00 | No UP |
| $70-9^{\text {2 }}$ | 5 | $\begin{gathered} \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | Insufficient Data | $\begin{gathered} \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | Insufficient Data | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | Insufficient Data | $\begin{aligned} & \hline \text { Insuffi } \\ & \text { Da } \end{aligned}$ |  | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | 2.40 | 3.00 | None |
| $70-10^{2 \lambda}$ | 19 | 1.63 | 0.76 | 2.50 | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | Insufficient Data | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | Insufficient Data | $\begin{array}{r} \hline \text { Insuffi } \\ \text { Da } \end{array}$ |  | Insufficient Data | 8.63 | 2.51 | No UP |
| 70-112^ | 4 | 3.37 | 6.74 | 0.00 | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | Insufficient Data | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | Insufficient Data | Insuffi Da |  | Insufficient Data | 0.00 | 0.00 | No UP |
| $70-12^{2 \wedge}$ | 15 | 2.63 | 2.97 | 2.28 | Insufficient Data | 22\% | Insufficient Data | Insufficient | Insufficient Data | Insufficient Data | Insuffi Da |  | Insufficient Data | 17.39 | 0.00 | No UP |
| $\begin{gathered} 70 \mid 60- \\ 13^{1 *} \\ \hline \end{gathered}$ | 12 | 2.97 | 3.36 | 2.57 | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \\ & \hline \end{aligned}$ | 21\% | $\begin{gathered} \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \\ & \hline \end{aligned}$ | 1.58 | 1.67 | 1.49 | 1.58 | 22.75 | 26.52 | 15.84 |
| $\begin{aligned} & 60 \mathrm{E}- \\ & 14^{2^{\wedge}} \\ & \hline \end{aligned}$ | 16 | 1.78 | 1.50 | 2.07 | Insufficient Data | 81\% | Insufficient Data | Insufficient Data | Insufficient Data | 1.49 | 1.52 | 1.46 | 1.49 | 63.60 | 344.95 | 13.03 |
| $\begin{aligned} & 60 \mathrm{E}- \\ & 15^{2^{\wedge}} \\ & \hline \end{aligned}$ | 2 | Insufficient Data | Insufficient Data | Insufficient Data | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | Insufficient Data | Insufficient Data | Insufficient Data | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | 1.32 | 1.34 | 1.29 | 1.32 | 0.00 | 90.50 | 16.79 |
| $\begin{aligned} & 60 \mathrm{E}- \\ & 16^{2^{\wedge}} \end{aligned}$ | 2 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 1.28 | 1.14 | 1.42 | 1.28 | 52.20 | 12.25 | No UP |

b2,3 or 4 Lane Divided
4 or 5 Lane Undivided
Uninterrupted

* Interrupted

Note: "Insufficient Data" indicates there was not enough data available to generate reliable performance ratings
"No UP" indicates no underpasses are present in the segment

Table 11: Corridor Performance Summary by Segment and Performance Measure (continued)

| $\underset{\#}{\text { Segment }}$ | Segment Length (miles) | Safety Performance Area |  |  |  |  |  |  |  | Freight Performance Area |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Safety Index | Directional Safety Index |  | \% of Fatal + Suspected Serious Injury Crashes at Intersections | \% of Fatal + Suspected Serious Injury Crashes Involving Lane Departures | \% of Fatal + Suspected Serious Injury Crashes Involving Pedestrians | \% of Segment Fatal + Suspected Serious Injury Crashes Involving Trucks | \% of Segment <br> Fatal + Suspected Serious Injury Crashes Involving Bicycles | Freight TTTR | Directional Max TTTR |  | Combined Average Peak TTTR | Average Minutes Per Year Given Milepost Is Closed Per Segment Mile (NB/EB) |  | Bridge Vertical Clearance (feet) |
|  |  |  | NB/EB | SB/WB |  |  |  |  |  |  | NB/EB | SB/WB |  | NB/EB | SB/WB |  |
| $\begin{aligned} & \hline 60 \mathrm{E}- \\ & 17^{2^{\wedge}} \\ & \hline \end{aligned}$ | 11 | 1.23 | 1.82 | 0.65 | Insufficient Data | 78\% | Insufficient Data | Insufficient Data | Insufficient Data | 1.18 | 1.15 | 1.20 | 1.18 | 3.27 | 61.40 | No UP |
| $\begin{aligned} & 60 \mathrm{E}- \\ & 18^{2^{\wedge}} \\ & \hline \end{aligned}$ | 7 | 0.50 | 0.91 | 0.09 | Insufficient Data | 17\% | Insufficient Data | Insufficient Data | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | 1.22 | 1.32 | 1.13 | 1.22 | 0.00 | 22.29 | No UP |
| $\begin{gathered} 60 \mathrm{E}- \\ 19^{1 *} \\ \hline \end{gathered}$ | 6 | 0.95 | 1.62 | 0.27 | Insufficient Data | 60\% | Insufficient Data | Insufficient Data | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | 1.63 | 1.74 | 1.52 | 1.63 | 14.00 | 20.30 | No UP |
| $\begin{aligned} & 60 \mathrm{E}- \\ & 20^{1 \wedge} \\ & \hline \end{aligned}$ | 5 | 1.29 | 1.89 | 0.69 | Insufficient Data | 50\% | Insufficient Data | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | 1.20 | 1.25 | 1.14 | 1.20 | 74.94 | 7.11 | No UP |
| Wei <br> Ave |  | 1.11 | 1.19 | 1.03 | Insufficient Data | 45\% | Insufficient Data | Insufficient Data | Insufficient Data | 1.64 | 1.75 | 1.54 | 1.64 | 12.16 | 30.69 | 18.90 |
| SCALES |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  2 or 3 or 4 Lane Divided Highway <br> Performance 2 or 3 Undivided Highway <br> Level 4 or 5 Undivided Highway <br>  Urban 4 Lane Freeway |  |  |  |  |  |  |  |  |  | Uninterrupted |  |  |  | All |  |  |
| Good Ave Perfor | Above age mance |  | $\begin{aligned} & <0.81 \\ & <0.92 \\ & <0.78 \\ & <0.73 \end{aligned}$ |  | $\begin{aligned} & <23.4 \% \\ & <11.2 \% \\ & <43.8 \% \\ & <0.0 \% \end{aligned}$ | $\begin{aligned} & <56.4 \% \\ & <66.9 \% \\ & <21.1 \% \\ & <60.6 \% \end{aligned}$ | $\begin{aligned} & <16 \% \\ & <3.8 \% \\ & <8.8 \% \\ & <0.0 \% \end{aligned}$ | $\begin{aligned} & <3.7 \% \\ & <4.2 \% \\ & <0.8 \% \\ & <6.9 \% \end{aligned}$ | $\begin{aligned} & <0 \% \\ & <0 \% \\ & <0.5 \% \\ & <0 \% \end{aligned}$ | < 1.15 |  |  |  | < 44.18 |  | > 16.5 |
| Fair/A Perfor | verage mance |  | $\begin{gathered} 0.81-1.19 \\ 0.92-1.08 \\ 0.78-1.22 \\ 0.73-1.27 \end{gathered}$ |  | $\begin{gathered} 23.4 \%-29.3 \% \\ 11.2 \%-15.6 \% \\ 43.8 \%-49.5 \% \\ 0.0 \%-0.0 \% \end{gathered}$ | $\begin{gathered} 56.4 \%-65.0 \% \\ 66.9 \%-74.5 \% \\ 21.1 \%-32.1 \% \\ 60.6 \%-78.1 \% \end{gathered}$ | $\begin{gathered} 16 \%-26 \% \\ 3.8 \%-7.2 \% \\ 8.8 \%-13.5 \% \\ 0.0 \%-4.9 \% \\ \hline \end{gathered}$ | $\begin{gathered} 3.7 \%-9.9 \% \\ 4.2 \%-8.0 \% \\ 0.8 \%-5.5 \% \\ 6.9 \%-12.4 \% \end{gathered}$ | $\begin{gathered} 0 \%-2 \% \\ 0 \%-3.3 \% \\ 0.5 \%-3.8 \% \\ 0.0 \%-0.0 \% \end{gathered}$ | 1.15-1.35 |  |  |  | 44.18-124.86 |  | $\begin{gathered} 16.0- \\ 16.5 \end{gathered}$ |
| Poor Ave Perform | elow age mance |  | $\begin{array}{r} >1.19 \\ >1.08 \\ >1.22 \\ >1.27 \\ \hline \end{array}$ |  | $\begin{aligned} & >29.3 \% \\ & >15.6 \% \\ & >49.5 \% \\ & >0.0 \% \end{aligned}$ | > 65.0\% | $\begin{gathered} >26 \% \\ >7.2 \% \\ >13.5 \% \\ >4.9 \% \end{gathered}$ | $\begin{gathered} 9.90 \% \\ >8.0 \% \\ >5.5 \% \\ >12.4 \% \end{gathered}$ | $\begin{gathered} >2 \% \\ >3.3 \% \\ >3.8 \% \\ >0.0 \% \\ \hline \end{gathered}$ | > 1.35 |  |  |  | > 124.86 |  | < 16.0 |

${ }^{\text {a }} 2$ or 3 Lane Undivided
2,3 or 4 Lane Divided
${ }^{\wedge} \wedge$ Uninterrupted

* Interrupted

Note: "Insufficient Data" indicates there was not enough data available to generate reliable performance ratings "No UP" indicates no underpasses are present in the segment

### 3.0 NEEDS ASSESSMENT

### 3.1 Corridor Objectives

Statewide goals and performance measures were established by the ADOT Long-Range Transportation Plan (LRTP), 2016-2040. Statewide performance goals that are relevant to US 60|US 70|US 191 performance areas were identified, and corridor goals were then formulated for each of the five performance areas that aligned with the overall statewide goals established by the LRTP. Based on stakeholder input, corridor goals, corridor objectives, and performance results, three "emphasis areas" were identified for the US 60|US 70|US 191 corridor: Mobility Safety, and Freight.

Taking into account the corridor goals and identified emphasis areas, performance objective were developed for each quantifiable performance measure that identify the desired level of performance based on the performance scale levels for the overall corridor and for each segment of the corridor. For the performance emphasis areas, the corridor-wide weighted average performance objectives are identified with a higher standard than for the other performance areas. Table 12 shows the US 60|US 70|US 191 corridor goals, corridor objectives, and performance objectives, and how they align with the statewide goals
It is not reasonable within a financially constrained environment to expect that every performance measure will always be at the highest levels on every corridor segment. Therefore individual corridor segment objectives have been set as "fair/average" or better and should not fall below that standard

Achieving corridor and segment performance objectives will help ensure that investments are targeted toward improvements that support the safe and efficient movement of travelers on the corridor. Addressing current and future congestion, thereby improving mobility on congested segments, will also help the corridor fulfill its potential as a significant contributor to the region's economy
Corridor performance is measured against corridor and segment objectives to determine needs - the gap between observed performance and performance objectives

Goal achievement will improve or reduce current and future congestion, increase travel time reliability, and reduce fatalities and incapacitating injuries resulting from vehicle crashes. Where performance is currently rated "good", the goal is always to maintain that standard, regardless of whether or not the performance is in an emphasis area.

Table 12: Corridor Performance Goals and Objectives

| ADOT Statewide LRTP Goals | US 60\|US 70|US 191 Corridor Goals | US 60\|US 70|US 191 Corridor Objectives | Performance Area | Primary Index | Perform | Objective |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Secondary Measure Indicators | Corridor Average | Segment |
| Improve Mobility and Accessibility <br> Support Economic Growth | Provide a safe, reliable, and efficient connection for the communities along the corridor <br> Provide a safe and reliable route for recreational and tourist travel <br> Consider future land use when recommending infrastructure improvements since agricultural activities are transitioning to development activities | Reduce current and future congestion in the urbanized areas <br> Reduce delays from non-recurring events and incidents to improve reliability <br> Improve bicycle accommodation | Mobility <br> (Emphasis Area) | Mobility Index <br> Future Daily V/C <br> Existing Peak Hour V/C <br> Closure Extent <br> Directional Level of Travel Time Reliability <br> \% Bicycle Accommodation <br> \% Non-SOV Trips | Good | Fair or Better |
|  | Provide a safe, reliable and efficient freight route between Arizona and Mexico | Reduce delays and restrictions to freight movement to improve reliability <br> Improve travel time reliability (including impacts to motorists due to freight traffic) | Freight <br> (Emphasis Area) | Freight Index <br> Truck Travel Time Reliability Closure Duration Bridge Vertical Clearance | Good | Fair or Better |
| Preserve and Maintain the State Transportation System | Preserve and modernize highway infrastructure <br> Provide an all-weather transportation facility | Maintain structural integrity of bridges | Bridge | Bridge Index | Fair or Better | Fair or Better |
|  |  |  |  | Sufficiency Rating Lowest Bridge Rating |  |  |
|  |  |  |  |  |  |  |
|  |  | Improve pavement ride quality | Pavement | Pavement Index | Fair or Better | Fair or Better |
|  |  |  |  | Directional Pavement Serviceability Rating \% Area Failure |  |  |
| Enhance Safety and Security | Promote safety by implementing appropriate countermeasures, particularly in mountainous and rolling terrain | Reduce fatal and serious injury crashes | Safety <br> (Emphasis Area) | Safety Index | Above Average | Average or Better |
|  |  |  |  | Directional Safety Index <br> \% of Suspected Serious Injury Crashes at Intersections <br> \% of Suspected Serious Injury Crashes Involving Lane <br> Departures <br> \% of Suspected Serious Injury Crashes Involving Pedestrians <br> \% of Suspected Serious Injury Crashes Involving Trucks <br> \% of Suspected Serious Injury Crashes Involving Bicycles |  |  |

### 3.2 Needs Assessment Process

The following guiding principles were used as an initial step in developing a framework for the performance-based needs assessment process:

- Corridor needs are defined as the difference between the corridor performance and the performance objectives
- The needs assessment process should be systematic, progressive, and repeatable, but also allow for engineering judgment where needed
- The process should consider all primary and secondary performance measures developed for the study
- The process should develop multiple need levels including programmatic needs for the entire length of the corridor, performance area-specific needs, segment-specific needs, and location-specific needs (defined by MP limits)
- The process should produce actionable needs that can be addressed through strategic investments in corridor preservation, modernization, and expansion
The performance-based needs assessment process is illustrated in Figure 19 and described in the following sections.

Figure 19: Needs Assessment Process

| STEP 1 |  | STEP 2 | STEP 3 | STEP 4 | STEP 5 |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Initial Need Identification | Need Refinement | Contributing Factors | Segment Review | Corridor Needs |
| $\begin{aligned} & 2 \\ & \frac{2}{6} \\ & \frac{0}{4} \end{aligned}$ | Compare results of performance baseline to performance objectives to identify initial performance need | Refine intial performance need based on recently completed projects and hotspots | Perform "drill-down" investigation of refined need to confirm need and to identify contributing factors | Summarize need on each segment | Identify overlapping, common, and contrasting contributing factors |
| $\frac{8}{4}$ | Initial levels of need (none, low, medium, high) by performance area and segment | Refined needs by performance area and segment | Confirmed needs and contributing factors by performance area and segment | Numeric level of need for each segment | Actionable performance-based needs defined by location |

Step 1: Initial Needs Identification
The first step in the needs assessment process links baseline (existing) corridor performance with performance objectives. In this step, the baseline corridor performance is compared to the performance objectives to provide a starting point for the identification of performance needs This mathematical comparison results in an initial need rating of None, Low, Medium, or High for each primary and secondary performance measure. An illustrative example of this process is shown below in Figure 20.
Figure 20: Initial Need Ratings in Relation to Baseline Performance (Bridge Example)

| Performance <br> Thresholds | Performance Level | Initial Level of Need* | Description |
| :---: | :---: | :---: | :---: |
| 6.5 | Good | None | All levels of Good and top 1/3 of Fair (>6.0) |
|  | Good |  |  |
|  | Good |  | Low |
| 5.0 | Fair | Medium | Lower $1 / 3$ of Fair and top 1/3 of Poor (4.5-5.5) |
|  | Fair | Poor | High |
|  | Poor | Lower 2/3 of Poor (<4.5) |  |

*A segment need rating of 'None' does not indicate a lack of needed improvements; rather, it indicates that the segment performance score exceeds the established performance thresholds and strategic solutions for that segment will not be developed as part of this study

The initial level of need for each segment is refined to account for hot spots and recently completed or under construction projects, resulting in a final level of need for each segment. The final levels of need for each primary and secondary performance measure are combined to produce a weighted final need rating for each segment. Values of $0,1,2$, and 3 are assigned to the initial need levels of None, Low, Medium, and High, respectively. A weight of 1.0 is applied to the Performance Index need and equal weights of 0.20 are applied to each need for each secondary performance measure. For directional secondary performance measures, each direction of travel receives a weight of 0.10.
Step 2: Need Refinement
In Step 2, the initial level of need for each segment is refined using the following information and engineering judgment:

- For segments with an initial need of None that contain hot spots, the level of need should be increased from None to Low
- For segments with an initial level of need where recently completed projects or projects under construction are anticipated to partially or fully address the identified need, the level of need should be reduced or eliminated as appropriate
- Programmed projects that are expected to partially or fully address an identified need are not justification to lower the initial need because the programmed projects may not be implemented as planned; in addition, further investigations may suggest that changes in the scope of a programmed project may be warranted
The resulting final needs are carried forward for further evaluation in Step 3.


## Step 3: Contributing Factors

In Step 3, a more detailed review of the condition and performance data available from ADOT is conducted to identify contributing factors to the need. Typically, the same databases used to develop the baseline performance serve as the principal sources for the more detailed analysis. However, other supplemental databases may also be useful sources of information. The databases used for diagnostic analysis are listed below:
Pavement Performance Area

- Pavement Rating Database


## Bridge Performance Area

- ABISS

Mobility Performance Area

- Highway Performance Monitoring System (HPMS) Database
- AZTDM
- Real-time traffic conditions data produced by INRIX Database
- Highway Conditions Reporting System (HCRS) Database


## Safety Performance Area

- Crash Database

Freight Performance Area

- INRIX Database
- HCRS Database

In addition, other sources considered helpful in identifying contributing factors are:

- Maintenance history (from ADOT PeCoS database for pavement), the level of past investments, or trends in historical data that provide context for pavement and bridge history
- Field observations from ADOT district personnel can be used to provide additional information regarding a need that has been identified
- Previous studies can provide additional information regarding a need that has been identified
- Maintenance history, the level of past investments, or trends in historical data that provide context for pavement and bridge history. (ADOT PeCoS data results from the origina ADOT CPS studies were used for pavement; updated PeCoS information is not regularly at the time of completion of the updated assessments)
Step 3 results in the identification of performance-based needs and contributing factors by segment (and MP locations, if appropriate) that can be addressed through investments in preservation, modernization, and expansion projects to improve corridor performance. See Appendix D for more information.

Step 4: Segment Review
In this step, the needs identified in Step 2 and refined in Step 3 are quantified for each segment to numerically estimate the level of need for each segment. Values of 0 to 3 are assigned to the final need levels (from Step 3) of None, Low, Medium, and High, respectively. A weighting factor is applied to the performance areas identified as emphasis areas and a weighted average need is calculated for each segment. The resulting average need score can be used to compare levels of need between segments within a corridor and between segments in different corridors.

Step 5: Corridor Needs
In this step, the needs and contributing factors for each performance area are reviewed on a segment-by-segment basis to identify actionable needs and to facilitate the formation of solution sets that address multiple performance areas and contributing factors. The intent of this process is to identify overlapping, common, and contrasting needs to help develop strategic solutions. This step results in the identification of corridor needs by specific location.

### 3.3 Corridor Needs Assessment

This section documents the results of the needs assessment process described in the prior section. The needs in each performance area were classified as either None, Low, Medium, or High based on how well each segment performed in the existing performance analysis. The needs for each segment were numerically combined to estimate the average level of need for each segment of the corridor
The final needs assessments for each performance measure, along with the scales used in analysis, are shown in Table 13 through Table 17.

Pavement Needs Refinement and Contributing Factors
Of the 246 corridor miles, approximately 9 miles (4\%) exhibit no level of pavement need, 77 miles ( $31 \%$ ) exhibit "Low" level of pavement need, 57 miles ( $23 \%$ ) exhibit "Medium" level of pavement need, and 103 miles ( $42 \%$ ) exhibit "High" level of pavement need. Pavement hot spot failure needs were identified for 71 miles on US 191, 71 miles on US 70, and 28 miles on US 60.

Key contributing factors are summarized below:

- A high level of historical investment has occurred on Segments 191-3 and 70-9 and a medium level of historical investment has occurred through Segments 191-2, 191-4,191-5 $70-8,70-10$, and 70-12 through 60-17
- See other contributing factors in Appendix D

Table 13: Final Pavement Needs

| Segment \# | Performance Score and Level of Need |  |  |  | Initial Segment Need | Hot Spots | Recently Completed Projects | Final Segment Need |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Pavement Index | Directional PSR |  | \% Area Failure |  |  |  |  |
|  |  | NB | SB |  |  |  |  |  |
| 191-1 | 3.17 | 3.10 | 3.24 | 71\% | 1.80 | MP 0-5 Both; MP 5-6 NB; MP 6-7 Both; MP 7-8 NB, MP 9-10 Both; MP 12-15 NB; MP15-20 Both; MP 2022 NB; MP 22-23 SB; MP24-23 Both | None | Medium |
| 191-2 | 2.89 | 3.44 | 3.38 | 56\% | 2.60 | MP 24-27 Both; MP 27-28 SB; MP28-29 NB; MP 2932 Both; MP 32-33 SB; MP 33-35 Both; MP 35-36 SB; MP 42-43 SB MP 45-46 Both; MP 48-49 SB; MP 50-62 Both | None | High |
| 191-3 | 3.42 | 3.36 | 3.69 | 72\% | 0.60 | MP 88-89 NB; MP 89-092 Both; MP94-95 NB; MP95-101 Both; MP101-104 NB | None | Low |
| 191-4 | 3.44 | 3.29 | 3.32 | 42\% | 0.70 | MP 104-109 Both | None | Low |
| 191-5 | 3.10 | 3.16 | 3.07 | 80\% | 1.90 | MP117-121 Both | None | Medium |
| 70-6 | 3.23 | 3.15 | 3.25 | 60\% | 1.80 | MP 330-332 SB; MP332-333 Both; MP 333-335 SB; MP 335-336 NB; MP 336-337 Both; MP 338-339 SB; MP 339-340 Both | None | Medium |
| 70-7 | 2.83 | 2.87 | 3.08 | 87\% | 3.00 | MP 300-314 Both; MP314-315 EB; MP 327-329 EB; MP 329-330 Both | None | High |
| 70-8 | 2.59 | 3.35 | 3.67 | 100\% | 2.60 | MP 298-300 Both | None | High |
| 70-9 | 2.71 | 3.44 | 3.36 | 100\% | 2.60 | MP 293-298 Both | None | High |
| 70-10 | 2.69 | 3.10 | 3.35 | 79\% | 2.70 | MP 274-275 Both; MP279-293 Both | None | High |

Table 13: Final Pavement Needs (continued)

| Segment \# | Performance Score and Level of Need |  |  |  | Initial Segment Need | Hot Spots | Recently Completed Projects | Final Segment Need |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Pavement Index | Directional PSR |  | \% Area Failure |  |  |  |  |
|  |  | NB | SB |  |  |  |  |  |
| 70-11 | 2.40 | 3.27 | 3.28 | 88\% | 3.80 | MP 270-271 EB; MP 271-274 Both | None | High |
| 70-12 | 3.57 | 3.28 | 3.53 | 33\% | 0.70 | MP 255-256 EB; MP 256-257 Both; MP 257-258 WB; MP 258-260 Both; MP266-268 EB | None | Low |
| 70\|60-13 | 3.28 | 3.13 | 3.28 | 54\% | 1.80 | MP 243-244 EB; MP 244-245 Both; MP 245-246 <br> EB; MP 249-251 EB; MP 252-255 Both | None | Medium |
| 60E-14 | 3.68 | 3.66 | 3.82 | 44\% | 0.60 | MP236-243 Both | None | Low |
| 60E-15 | 4.03 | 3.70 | 3.65 | 0\% | 0.00 | None | None | None |
| 60E-16 | 4.50 | 4.22 | 4.15 | 0\% | 0.00 | None | None | None |
| 60E-17 | 3.51 | 3.93 | 3.99 | 76\% | 0.60 | MP 212-213 Both; MP 213-215 WB; MP215-219 Both; MP 219-220 EB; MP 220-221 Both; MP 221-222 EB | None | Low |
| 60E-18 | 3.30 | 3.62 | 3.83 | 93\% | 1.60 | MP 205-206 WB; MP 206-212 Both | None | Medium |
| 60E-19 | 3.57 | 3.57 | 3.65 | 33\% | 0.60 | MP 199-201 WB; MP 201-202 EB; MP 204-205 WB | None | Low |
| 60E-20 | 4.17 | 3.87 | 3.83 | 0\% | 0.00 | None | None | None |


| Need Scales for Interstates |  |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: |
| Level of <br> Need <br> (Score) | Performance Score Need Scale |  |  | Segment Level Need <br> Scale |  |
| None* (0) | $>3.5$ | $>3.63$ | $<10 \%$ | 0 |  |
| Low (1) | $3.25-3.5$ | $3.63-3.52$ | $10 \%-15 \%$ | $<1.5$ |  |
| Medium (2) | $2.75-3.25$ | $3.52-3.38$ | $15 \%-25 \%$ | $1.5-2.5$ |  |
| High (3) | $<2.75$ | $<3.38$ | $>25 \%$ | $>2.5$ |  |
|  |  |  |  |  |  |
| Level of Scales for Highways (Non-Interstates) <br> Need <br> (Score) | Performance Score Need Scale |  |  |  |  |
| None* (0) | $>3.33$ | $>3.30$ | $<10 \%$ | Segment Level Need <br> Scale |  |
| Low (1) | $3.07-3.33$ | $3.30-3.10$ | $10 \%-15 \%$ | 0 |  |
| Medium (2) | $2.53-3.07$ | $3.10-2.70$ | $15 \%-25 \%$ | $1.5-2.5$ |  |
| High (3) | $<2.53$ | $<2.70$ | $>25 \%$ | $>2.5$ |  |

*A segment need rating of 'None' does not indicate a lack of needed improvements; rather, it indicates that the segment performance score exceeds the established performance thresholds and strategic solutions for that segment will not be developed as part of this study.

Bridge Needs Refinement and Contributing Factors
Bridge needs were identified on four segments of the corridor, 73 miles ( $30 \%$ ) with a "Medium" level of bridge need. These included all bridges that were documented having a bridge rating of 5 or less in deck, substructure, superstructure, or overall structural evaluation. For the remainder of the corridor, 111 miles ( $45 \%$ ) have a "Low" level of bridge need, 52 miles ( $21 \%$ ) have no level of bridge need, and a final level of bridge need could not be determined for 10 miles ( $4 \%$ ) of the corridor due to insufficient data.
Key contributing factors are summarized as follows:

- None of the initial needs required adjustment since no recent bridge work has occurred within the corridor that would change the bridge conditions.
- Eleven bridges were defined as hot spots since they had multiple bridge ratings of 5 or one rating of 4. Three bridges have bridge ratings of 4: Pinal Creek Bridge (No. 266), Queen Creek Bridge (No. 406) and Waterfall Canyon Bridge (No. 328)
- Of the eleven hot spot bridges, seven also showed repetitive investment issues. These included the Cochise UPRR OP (No. 157), Holyoak Wash Bridge (\#514), Bloody Tanks Bridge (\#173), Pinal Creek Bridge (No. 36), Pinal Creek Bridge (No. 266), Pinal Creek Bridge (No. 549) and McMillen Wash Bridge (\#1028)
- See other contributing factors in Appendix D.

Table 14: Final Bridge Needs

| Segment \# | Performance Score and Level of Need |  |  | Initial Segment Need | Hot Spots | Recently Completed Projects | Final Segment Need |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Bridge Index | Sufficiency Rating | Lowest Bridge Rating |  |  |  |  |
| 191-1 | 6.00 | 87.80 | 6.00 | 0.0 | None | None | None |
| 191-2 | 5.36 | 69.23 | 5.00 | 2.4 | Cochise UPRR OP (\#157) (MP 62.88) | None | Medium |
| 191-3 | 5.51 | 93.81 | 5.00 | 1.2 | None | None | Low |
| 191-4 | 6.00 | 69.50 | 6.00 | 0.2 | None | None | Low |
| 191-5 | No Bridges | No Bridges | No Bridges | None | No Bridges within Segment | None | N/A |
| 70-6 | 6.00 | 68.10 | 6.00 | 0.2 | None | None | Low |
| 70-7 | 5.74 | 70.25 | 5.00 | 1.2 | Holyoak Wash Bridge (\#514) (MP 302.53) | None | Low |
| 70-8 | 6.00 | 73.00 | 6.00 | 0.0 | None | None | None |
| 70-9 | No Bridges | No Bridges | No Bridges | None | No Bridges within Segment | None | N/A |
| 70-10 | 7.00 | 80.00 | 7.00 | 0.0 | None | None | None |
| 70-11 | 6.69 | 82.02 | 5.00 | 0.2 | None | None | Low |
| 70-12 | 6.00 | 52.90 | 6.00 | 0.4 | None | None | Low |


| Level of <br> Need <br> (Score) | Performance Score Need Scale |  |  | Segment Level Need <br> Scale |
| :--- | :---: | :---: | :---: | :---: |
| None (0) | $>6.0$ | $>70$ | $>5$ | 0 |
| Low (1) | $5.5-6.0$ | $60-70$ | 5 | $<1.5$ |
| Medium (2) | $4.5-5.5$ | $40-60$ | 4 | $1.5-2.5$ |
| High (3) | $<4.5$ | $<40$ | $<4$ | $>2.5$ |

*A segment need rating of 'None' does not indicate a lack of needed improvements; rather, it indicates that the segment performance score exceeds the established performance thresholds and strategic solutions for that segment will not be developed as part of this study

Table 14: Final Bridge Needs (continued)

| Segment \# | Performance Score and Level of Need |  |  | Initial Segment Need | Hot Spots | Recently Completed Projects | Final Segment Need |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Bridge Index | Sufficiency Rating | Lowest Bridge Rating |  |  |  |  |
| 70\|60-13 | 5.16 | 78.01 | 4.00 | 2.4 | Bloody Tanks Bridge (\#173) (MP 243.71) <br> Pinal Creek Bridge (\#266) (MP249.64) <br> Pinal Creek Bridge (\#36) (MP249.80) <br> Pinal Creek Bridge (\#549) (MP 250.37) <br> McMillen Wash Bridge (\#1028) (MP251.75) | None | Medium |
| 60E-14 | 5.52 | 68.13 | 3.00 | 1.8 | Queen Creek Bridge (\#436) (MP 226.14) Waterfall Canyon Bridge (\# 328) (MP229.50) | None | Medium |
| 60E-15 | 6.32 | 84.08 | 6.00 | 0.0 | None | None | None |
| 60E-16 | 5.00 | 86.43 | 5.00 | 2.2 | None | None | Medium |
| 60E-17 | 6.64 | 95.57 | 5.00 | 0.2 | None | None | Low |
| 60E-18 | 5.89 | 90.24 | 5.00 | 1.2 | Sand Tanks Wash Bridge EB (\#435) (MP208.75) Bridge WB <br> (\# 857) (MP 207.98) | None | Low |
| 60E-19 | 5.93 | 91.43 | 5.00 | 1.2 | None | None | Low |
| 60E-20 | 6.00 | 93.95 | 6.00 | 0.0 | None | None | None |


| Level of <br> Need <br> (Score) | Performance Score Need Scale |  |  | Segment Level Need <br> Scale |
| :--- | :---: | :---: | :---: | :---: |
| None (0) | $>6.0$ | $>70$ | $>5$ | 0 |
| Low (1) | $5.5-6.0$ | $60-70$ | 5 | $<1.5$ |
| Medium <br> $(2)$ | $4.5-5.5$ | $40-60$ | 4 | $1.5-2.5$ |
| High (3) | $<4.5$ | $<40$ | $<4$ | $>2.5$ |

*A segment need rating of 'None' does not indicate a lack of needed improvements; rather, it indicates that the segment performance score exceeds the established performance thresholds and strategic solutions for that segment will not be developed as part of this study.

## Mobility Needs Refinement and Contributing Factors

Mobility was identified as a focus area for the US 60| US 70| US 191 corridor. No level of need was identified on 66 miles of the corridor ( $27 \%$ ), a low level of mobility need was identified on 153 miles ( $63 \%$ ) of the corridor and a high level of mobility need was identified on 27 miles (10\%) of the corridor.
Key contributing factors are summarized below:

- Multiple closures of the roadway due to incidents/crashes in Segment 60E-14 (MP 227-243)
- Limited bicycle accommodation on much of the corridor, on US 191 from MP 87-104 and MP 116 - 121, and US 60/70 from MP 330-339, MP 298 - 243 and MP 199-205,
- Segment 60E-19 has a high mobility level of need due to existing and future daily volume-tocapacity issues and limited bicycle accommodation
- Segment 60E-20 has a high level of need due to mobility, existing v/c and future v/c issues, closure extents in the NB/WB direction (due to crashes and accidents) and limited bicycle accommodation
See other contributing factors in Appendix D.


## Table 15: Final Mobility Needs

| Segment \# | Performance Score and Level of Need |  |  |  |  |  |  |  |  | Initial Segment Need | Recently Completed Projects |  | Final Segment Need |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Mobility Index | Future Daily V/C | Existing Peak Hour V/C |  | Closure Extent |  | Directional LOTTR |  | \% Bicycle Accommodation |  |  |  |  |
|  |  |  | NB/WB | SB/EB | NB/WB | SB/EB | NB/WB | SB/EB |  |  |  |  |  |
| 191-1 ${ }^{2}$ | 0.16 | 0.18 | 0.13 | 0.13 | 0.04 | 0.02 | 1.40 | 1.39 | 66\% | 0.8 | None |  | Low |
| 191-2 ${ }^{2}$ | 0.13 | 0.17 | 0.08 | 0.11 | 0.03 | 0.01 | No Data | No Data | 100\% | 0.0 | None |  | None |
| $191-3^{2}$ | 0.05 | 0.05 | 0.03 | 0.03 | 0.02 | 0.00 | No Data | No Data | 49\% | 0.6 | None |  | Low |
| 191-4 ${ }^{2}$ | 0.17 | 0.19 | 0.11 | 0.11 | 0.08 | 0.07 | No Data | No Data | 97\% | 0.0 | None |  | None |
| 191-51 | 0.27 | 0.30 | 0.15 | 0.16 | 0.20 | 0.20 | No Data | No Data | 27\% | 0.6 | None |  | Low |
| 70-6 ${ }^{1}$ | 0.41 | 0.45 | 0.31 | 0.29 | 0.02 | 0.04 | No Data | No Data | 46\% | 0.6 | None |  | Low |
| 70-7 ${ }^{2}$ | 0.18 | 0.20 | 0.11 | 0.10 | 0.04 | 0.01 | No Data | No Data | 73\% | 0.2 | None |  | Low |
| 70-8 ${ }^{2}$ | 0.11 | 0.12 | 0.08 | 0.05 | 0.10 | 0.00 | No Data | No Data | 0\% | 0.6 | None |  | Low |
| 70-9 ${ }^{2}$ | 0.24 | 0.26 | 0.16 | 0.12 | 0.04 | 0.04 | No Data | No Data | 26\% | 0.6 | None |  | Low |
| 70-10 ${ }^{2}$ | 0.15 | 0.17 | 0.11 | 0.08 | 0.07 | 0.05 | No Data | No Data | 4\% | 0.6 | None |  | Low |

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Table 15: Final Mobility Needs (continued)

| Segment \# | Performance Score and Level of Need |  |  |  |  |  |  |  |  | Initial Segment Need | Recently Completed Projects | Final Segment Need |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Mobility Index | Future Daily V/C | Existing Peak Hour V/C |  | Closure Extent |  | Directional LOTTR |  | \% Bicycle Accommodation |  |  |  |
|  |  |  | NB/WB | SB/EB | NB/WB | SB/EB | NB/WB | SB/EB |  |  |  |  |
| 70-11 ${ }^{2}$ | 0.18 | 0.20 | 0.13 | 0.10 | 0.00 | 0.00 | No Data | No Data | 4\% | 0.6 | None | Low |
| 70-12 ${ }^{2}$ | 0.24 | 0.27 | 0.16 | 0.17 | 0.17 | 0.00 | No Data | No Data | 23\% | 0.6 | None | Low |
| 70/60E-13 ${ }^{1}$ | 0.40 | 0.45 | 0.26 | 0.25 | 0.22 | 0.35 | 1.16 | 1.15 | 54\% | 0.4 | FY19 MP 247 New DMS Sign Eastbound (Arizona Statewide DMS Plan) and construct new sidewalks on northside from MP 243252 (Cobre Valley Comprehensive Transportation Study) | Low |
| 60E-14 ${ }^{2}$ | 1.42 | 1.71 | 0.79 | 1.14 | 0.67 | 1.84 | 1.12 | 1.17 | 49\% | 5.2 | None | High |
| $60 \mathrm{E}-15^{2}$ | 0.27 | 0.37 | 0.11 | 0.11 | 0.00 | 0.90 | 1.18 | 1.14 | 95\% | 0.3 | None | Low |
| $60 \mathrm{E}-16^{2}$ | 0.27 | 0.38 | 0.16 | 0.16 | 0.60 | 0.15 | 1.05 | 1.12 | 87\% | 0.2 | None | Low |
| $60 \mathrm{E}-17^{2}$ | 0.26 | 0.37 | 0.15 | 0.14 | 0.04 | 0.23 | 1.05 | 1.09 | 96\% | 0.0 | Picket Post- Construct new EB lanes parallel to existing, between Reymert Wash and Queen Creek and Gonzales Pass- Construct new EB lanes west of the summit, construct new WB lanes east of the summit | None |
| $60 \mathrm{E}-18^{2}$ | 0.53 | 0.66 | 0.30 | 0.32 | 0.00 | 0.23 | 1.12 | 1.05 | 100\% | 0.2 | None | Low |
| $60 \mathrm{E}-19^{1}$ | 1.01 | 0.86 | 0.86 | 0.91 | 0.10 | 0.30 | 1.20 | 1.14 | 42\% | 4.4 | None | High |
| $60 \mathrm{E}-20^{1}$ | 1.31 | 1.45 | 0.84 | 0.88 | 0.68 | 0.09 | 1.06 | 1.06 | 100\% | 4.2 | None | High |
| Level of Need (Score) | Performance Score Need Scale |  |  |  |  |  |  |  |  | Segment Level Need Scale |  |  |
| None* (0) | $\leq 0.77$ (Urban) |  |  |  | < 0.35 |  | $<1.27^{\text {a }}$ |  | > 80\% | 0 |  |  |
|  | $\leq 0.63$ (Rural) |  |  |  |  |  | < $1.27{ }^{\text {b }}$ |  |  |  |  |  |
| Low (1) | 0.77-0.83 (Urban) |  |  |  | 0.35-0.49 |  | 1.27 | $1.38{ }^{\text {a }}$ | 70\% - 80\% | <1.5 |  |  |
|  | 0.63-0.69 (Rural) |  |  |  |  |  | 1.27 | $1.38{ }^{\text {b }}$ |  |  |  |  |
| Medium (2) | 0.83-0.95 (Urban) |  |  |  | 0.49-0.75 |  | 1.38 | $1.62{ }^{\text {a }}$ | 50\% - 70\% | 1.5-2.5 |  |  |
|  | 0.69-0.83 (Rural) |  |  |  |  |  | 1.38 | $1.62{ }^{\text {b }}$ |  |  |  |  |
| High (3) | $\begin{aligned} & \geq 0.95 \text { (Urban) } \\ & \geq 0.83 \text { (Rural) } \end{aligned}$ |  |  |  | > 0.75 |  |  |  | < 50\% | > 2.5 |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |

${ }^{1}$ Urban or Fringe Urban Operating Environment
${ }^{2}$ Rural Operating Environment


Safety Needs Refinements and Contributing Factors
Safety was identified as a focus area for the US 60|US 70|US 191 corridor. A High level of safety need was identified for 77 miles ( $31 \%$ ) of the corridor, a Medium level of safety need was identified for 10.7 miles (4\%) of the corridor, a Low level of safety need was identified for 45 miles (18\%) of the corridor, and no level of safety need was identified for 102 miles (42\%) of the corridor. A safety need was not identified for the remaining 11 miles ( $4 \%$ ) of the corridor since there was insufficient data for each measure.
Key contributing factors to the safety needs are summarized below:

- Along US 70 from Bylas to Peridot, segment 70-10, 70 percent of crashes involved single vehicle and about half of the crashes involved collision with a fixed object
- Within Peridot, Segment 70-11, both fatal crashes involved pedestrian collision, occurred in darkunlighted conditions, and driving under the influence of drugs or alcohol
- On segment 70-12, from Peridot to the San Carlos Apache Reservation, 7 of the 9 crashes were fatal with $33 \%$ of crashes involving collision with a pedestrian.
- On Segment 60|70-13, 80\% of the crashes involved collision with another motor vehicle with $30 \%$ of collisions occurring at an angle
- On segment $60-14,50 \%$ of the crashes involved speeding too fast for conditions with over $50 \%$ of crashes involving collision with a fixed object.
- Along Segment 60-17, 4 out of 9 crashes occurred during dark unlit conditions and involved speeding too fast for conditions
- On Segment 60E-19, 60\% of the crashes involved exceeding lawful speed limits
- Out of the 6 fatal or incapacitating crashes in Segment 60E-20, half of the crashes were rear-end collisions and half were single-vehicle collisions
- See other Contributing Factors in Appendix D.

Table 16: Final Safety Needs

| $\underset{\#}{\text { Segment }}$ | Performance Score and Level of Need |  |  |  |  |  |  | \% of Fatal + Suspected Serious Injury Crashes Involving Bicycles | Initial Segment Need | Hot Spots | Recently Completed Projects |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Safety Index | Directional Safety Index |  | \% of Fatal + Suspected Serious Injury Crashes at Intersections | \% of Fatal + Suspected Serious Injury Crashes Involving Lane Departures | \% of Fatal + Suspected Serious Injury Crashes Involving Pedestrians | \% of Fatal + Suspected Serious Injury Crashes Involving Trucks |  |  |  |  | Final |
|  |  | NB/WB | SB/EB |  |  |  |  |  |  |  |  |  |
| 191-1 ${ }^{\text {a }}$ | 0.39 | 0.04 | 0.73 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 0.0 | None | None | None |
| 191-2 ${ }^{\text {a }}$ | 0.49 | 0.54 | 0.44 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 0.0 | None | None | None |
| $191-3^{\text {b }}$ | 0.59 | 0.00 | 1.18 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 0.2 | None | None | Low |
| $191-4{ }^{\text {a }}$ | 0.58 | 1.06 | 0.11 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 0.2 | None | None | Low |
| $191-5^{\text {c }}$ | 0.06 | 0.12 | 0.00 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 0.0 | None | Restripe to 5 lanes between 11th Street and US 70 (MP 120-121) | None |
| $70-6{ }^{\text {c }}$ | 0.38 | 0.67 | 0.08 | Insufficient Data | 25\% | Insufficient Data | Insufficient Data | Insufficient Data | 0.0 | $\begin{aligned} & \text { MP } 336.5 \text { - } \\ & \text { JCT US191 } \end{aligned}$ | Traffic Signal or Roundabout Construction (MP 335.5) Construct Pedestrian Bridge Extension (MP 299-300) | Low |
| $70-7^{\text {a }}$ | 0.68 | 0.89 | 0.48 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 0.0 | None | None | None |
| $70-8{ }^{\text {a }}$ | Insufficie nt Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 0.0 | None | None | N/A |

a 2 or 3 Lane Undivided
b2,3 or 4 Lane Divided
-4 or 5 Lane Undivided
d Urban 4 Lne Freeway
*A segment need rating of 'None' does not indicate a lack of needed improvements; rather, it indicates that the segment performance score exceeds the established performance thresholds and strategic solutions for that segment will not be developed as

## Table 16: Final Safety Needs (continued)

| $\underset{\#}{\text { Segment }}$ | Performance Score and Level of Need |  |  |  |  |  |  | \% of Fatal + <br> Suspected Serious Injury Crashes Involving Bicycles | Initial Segment Need | Hot Spots | Recently Completed Projects | Final Segment Need |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Safety Index | Directional Safety Index |  | \% of Fatal + Suspected Serious Injury Crashes at Intersections | \% of Fatal + Suspected Serious Injury Crashes Involving Lane Departures | \% of Fatal + <br> Suspected Serious Injury Crashes Involving Pedestrians | \% of Fatal + Suspected Serious Injury Crashes Involving Trucks |  |  |  |  |  |
|  |  | NB/WB | SB/EB |  |  |  |  |  |  |  |  |  |
| $70-9^{\text {a }}$ | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 0.0 | None | Reduced Speed from 50 MPH to 40 MPH (MP 294-298) <br> Eliminate passing zone through Bylas (MP 294.6-295.5) <br> Pedestrian Safety improvements Pedestrian crossings, warning signs/flashing lights, ADA compliant pedestrian gates (MP 294-298) Curb installation on north side of US 70 (MP 296.5) | N/A |
| $70-10^{\text {a }}$ | 1.63 | 0.76 | 2.50 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 3.3 | None | None | High |
| 70-11 ${ }^{\text {a }}$ | 3.37 | 6.74 | 0.00 | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 3.3 | None | None | High |
| $70-12^{\text {a }}$ | 2.63 | 2.97 | 2.28 | Insufficient Data | 22\% | Insufficient Data | Insufficient Data | Insufficient Data | 3.6 | None | Install Lighting and Center Turn Lane at US 70 \& 177 intersection (US 70 Cutter Safety Improvements previous round Prioritized solution) | High |
| $\underset{\mathrm{a}}{60 \mid 70-13}$ | 2.97 | 3.36 | 2.57 | Insufficient Data | 21\% | Insufficient Data | Insufficient Data | Insufficient Data | 3.6 | $\begin{gathered} \text { MP } 247 \text { - } \\ 253.4 \end{gathered}$ | Construct new sidewalks on north side (MP 243-252) <br> DMS Sign Eastbound Installed (MP 247) | High |
| 60-14 ${ }^{\text {a }}$ | 1.78 | 1.50 | 2.07 | Insufficient Data | 81\% | Insufficient Data | Insufficient Data | Insufficient Data | 4.2 | $\begin{gathered} \hline \text { MP 241- } \\ 242.6, \\ \text { MP } 227- \\ 232.3 \end{gathered}$ | None | High |
| $60-15^{\text {a }}$ | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 0.0 | None | None | N/A |
| 60-16 ${ }^{\text {a }}$ | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | Insufficient Data | 0.0 | None | None | N/A |

a 2 or 3 Lane Undivided
b2,3 or 4 Lane Divided
c 4 or 5 Lane Undivided
d Urban 4 Lane Freeway
*A segment need rating of 'None' does not indicate a lack of needed improvements; rather, it indicates that the segment performance score exceeds the established performance thresholds and strategic solutions for that segment will not be developed as

## Table 16: Final Safety Needs (continued)

| Segment \# | Performance Score and Level of Need |  |  |  |  |  |  |  | Initial Segment Need | Hot Spots | Recently Completed Projects | Final Segment Need |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Safety Index | Directional Safety Index |  | \% of Fatal + Suspected Serious Injury Crashes at Intersections | \% of Fatal + Suspected Serious Injury Crashes Involving Lane Departures | \% of Fatal + Suspected Serious Injury Crashes Involving Pedestrians | \% of Fatal + Suspected Serious Injury Crashes Involving Trucks | \% of Fatal + Suspected Serious Injury Crashes Involving Bicycles |  |  |  |  |
|  |  | NB/WB | SB/EB |  |  |  |  |  |  |  |  |  |
| $60-17^{\text {b }}$ | 1.23 | 1.82 | 0.65 | Insufficient Data | 78\% | Insufficient Data | Insufficient Data | Insufficient Data | 2.9 | $\begin{aligned} & \text { MP } \\ & 214.3 \\ & - \\ & 216.7 \end{aligned}$ | Picket Post- Construct new EB lanes parallel to existing, between Reymert Wash and Queen Creek (MP 219.9222.3) <br> Gonzales Pass- Construct new EB lanes west of the summit, construct new WB lanes east of the summit (MP 216.3219.9) | High |
| $60-18^{\text {b }}$ | 0.50 | 0.91 | 0.09 | Insufficient Data | 17\% | Insufficient Data | Insufficient Data | Insufficient Data | 0.0 | $\begin{array}{\|l\|} \hline \text { MP } \\ 206- \\ 208 \\ \hline \end{array}$ | None | Low |
| $60-19^{\text {b }}$ | 0.95 | 1.62 | 0.27 | Insufficient Data | 60\% | Insufficient Data | Insufficient Data | Insufficient Data | 1.5 | $\begin{aligned} & \hline \text { MP } \\ & 200.4 \\ & - \\ & 203.5 \\ & \hline \end{aligned}$ | None | Medium |
| $60-20^{\text {d }}$ | 1.29 | 1.89 | 0.69 | Insufficient Data | 50\% | Insufficient Data | Insufficient Data | Insufficient Data | 2.3 | $\begin{aligned} & \text { MP } \\ & 195- \\ & 197 \end{aligned}$ | None | Medium |


| $\begin{aligned} & \text { Leve } \\ & \text { Nee } \end{aligned}$ (Sco |  | Performance Score Needs Scale |  |  |  |  |  | Segment Level Need Scale |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| None <br> (0) | $\begin{aligned} & \mathrm{a} \\ & \mathrm{~b} \\ & \mathrm{c} \\ & \mathrm{~d} \end{aligned}$ | $\begin{aligned} & \leq 0.97 \\ & \leq 0.94 \\ & \leq 0.93 \\ & \leq 0.91 \end{aligned}$ | $\begin{aligned} & \leq 13 \% \\ & \leq 25 \% \\ & \leq 46 \% \\ & 0 \% \end{aligned}$ | $\begin{aligned} & \leq 69 \% \\ & \leq 59 \% \\ & \leq 25 \% \\ & \leq 66 \% \\ & \hline \end{aligned}$ | $\begin{aligned} & \leq 5 \% \\ & \leq 3 \% \\ & \leq 10 \% \\ & \leq 2 \% \end{aligned}$ | $\begin{aligned} & \leq 5 \% \\ & \leq 6 \% \\ & \leq 2 \% \\ & \leq 9 \% \end{aligned}$ | $\begin{gathered} \leq 1 \% \\ \leq 1 \% \\ \leq 2 \% \\ 0 \% \end{gathered}$ | 0 |
| $\begin{aligned} & \text { Low } \\ & \text { (1) } \end{aligned}$ | $\begin{aligned} & \hline a \\ & b \\ & c \\ & c \\ & d \end{aligned}$ | $\begin{aligned} & \hline 0.97-1.02 \\ & 0.94-1.07 \\ & 0.93-1.08 \\ & 0.91-1.09 \\ & \hline \end{aligned}$ | $\begin{gathered} \hline 13 \%-14 \% \\ 25 \%-27 \% \\ 46 \%-48 \% \\ 0 \% \end{gathered}$ | $\begin{gathered} \hline 69 \%-72 \% \\ 59 \%-62 \% \\ 25 \%-29 \% \\ 66 \%-72 \% \end{gathered}$ | $\begin{gathered} 5 \%-6 \% \\ 3 \%-3 \% \\ 10 \%-12 \% \\ 2 \%-4 \% \end{gathered}$ | $\begin{aligned} & 5 \%-6 \% \\ & 6 \%-8 \% \\ & 2 \%-4 \% \\ & 9 \%-11 \% \end{aligned}$ | $\begin{gathered} 1 \%-2 \% \\ 1 \%-2 \% \\ 2 \%-3 \% \\ 0 \% \end{gathered}$ | $\leq 1.5$ |
| Medi <br> um <br> (2) | $\begin{aligned} & \text { a } \\ & b \\ & c \\ & d \\ & d \end{aligned}$ | $\begin{gathered} \hline 1.02-1.13 \\ 1.07-1.32 \\ 1.08-1.4 \\ 1.09-1.45 \\ \hline \end{gathered}$ | $\begin{gathered} 14 \%-17 \% \\ 27 \%-31 \% \\ 48 \%-52 \% \\ 0 \% \end{gathered}$ | $72 \%$ - 77\% <br> 62\% - 68\% <br> 29\% - $36 \%$ <br> 72\% - 84\% | $\begin{gathered} \hline 6 \%-8 \% \\ 3 \%-4 \% \\ 12 \%-15 \% \\ 4 \%-7 \% \\ \hline \end{gathered}$ | $\begin{gathered} \hline 6 \%-9 \% \\ 8 \%-12 \% \\ 4 \%-7 \% \\ 11 \%-15 \% \end{gathered}$ | $\begin{gathered} 2 \%-4 \% \\ 2 \%-3 \% \\ 3 \%-5 \% \\ 0 \% \\ \hline \end{gathered}$ | 1.5-2.5 |
| $\begin{aligned} & \text { High } \\ & \text { (3) } \end{aligned}$ |  | $\begin{aligned} & \geq 1.13 \\ & \geq 1.32 \\ & \geq 1.4 \\ & \geq 1.45 \end{aligned}$ | $\begin{aligned} & \geq 17 \% \\ & \geq 31 \% \\ & \geq 52 \% \\ & 0 \% \end{aligned}$ | $\begin{aligned} & \geq 77 \% \\ & \geq 68 \% \\ & \geq 36 \% \\ & \geq 84 \% \end{aligned}$ | $\begin{aligned} & \geq 22 \% \\ & \geq 15 \% \\ & \geq 22 \% \\ & \geq 7 \% \end{aligned}$ | $\begin{aligned} & \geq 9 \% \\ & \geq 12 \% \\ & \geq 7 \% \\ & \geq 15 \% \end{aligned}$ | $\begin{aligned} & \geq 4 \% \\ & \geq 3 \% \\ & \geq 5 \% \\ & 0 \% \end{aligned}$ | $\geq 2.5$ |

${ }^{2} 2$ or 3 Lane Undivided
${ }^{\mathrm{b}} 2,3$ or 4 Lane Divided

- 4 or 5 Lane Undivided
dUrban 4 Lane Freeway
*A segment need rating of 'None' does not indicate a lack of needed improvements; rather, it indicates that the segment performance score exceeds the established performance thresholds and strategic solutions for that segment will not be developed as part of this study.

Freight Needs Refinements and Contributing Factors
A Low or medium level of freight needs was identified on 16 miles (7\%) each of the US 60 US $70 \mid$ US 191 corridor, and a High level of freight need was identified on 45 miles (18\%) of the corridor. The remainder of the corridor had no level of need, or there wasn't data to support a rating. Where there was data, a poor level of Truck Travel Time Reliability (TTTR) contributed to elevated freight needs on segments 191-1, 60-14, and 60-20. A segment need rating of 'None' does not indicate a lack of needed improvements; rather, it indicates that the segment performance score exceeds the established performance thresholds and strategic solutions for that segment will not be developed as part of this study.

Key contributing factors are summarized below

- Lengthy winter closures that shut down long segments on US 60 MP in 2016 and 2018 resulted in closure ratios that are above statewide average.
- Clearance restrictions exist at Pinal SPRR UP MP 253.63 (No. 562, height of 15.84 feet) and Queen Creek Tunnel MP 228.47 (height of 13.03 feet)
- See other Contributing Factors in Appendix D.


## Table 17: Final Freight Needs

| Segment \# | Performance Score and Level of Need |  |  |  |  |  | Initial Segment Need* | Hot Spots | Relevant Recently Completed or Under Construction Projects (which supersede performance data)* | Final Segment Need* |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Freight Index | Directional TTTR |  | Closure Duration |  | Bridge Clearance |  |  |  |  |
|  |  | NB/WB | SB/EB | $\begin{gathered} \text { NB/W } \\ \text { B } \end{gathered}$ | SB/EB |  |  |  |  |  |
| 191-1 ${ }^{\text {a }}$ | 2.26 | 1.40 | 1.39 | 3.02 | 1.00 | No UP | High | None | None | High |
| $191-2^{\text {a }}$ | Insufficient Data | Insufficient Data | Insufficient Data | 2.67 | 1.78 | 22.04 | None | None | None | N/A |
| $191-3^{\text {b }}$ | Insufficient Data | Insufficient Data | Insufficient Data | 2.47 | 0.00 | No UP | None | None | None | N/A |
| 191-4 ${ }^{\text {b }}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | Insufficient Data | Insufficient Data | 12.23 | 5.00 | No UP | None | None | None | N/A |
| 191-5 ${ }^{\text {a }}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | 26.08 | 16.96 | None | None | None | None | N/A |
| 70-6 ${ }^{\text {a }}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | 1.33 | 4.67 | No UP | None | None | None | N/A |
| 70-7 ${ }^{\text {b }}$ | Insufficient Data | Insufficient Data | Insufficient Data | 4.55 | 5.40 | 17.03 | None | None | None | N/A |
| $70-8{ }^{\text {b }}$ | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \\ \hline \end{gathered}$ | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | 14.30 | 0.00 | No UP | None | None | None | N/A |
| 70-9 ${ }^{\text {b }}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | $\begin{aligned} & \text { Insufficient } \\ & \text { Data } \end{aligned}$ | 2.40 | 3.00 | None | None | None | None | N/A |
| $70-10^{\text {b }}$ | Insufficient Data | Insufficient Data | Insufficient Data | 8.63 | 2.51 | No UP | None | None | None | N/A |
| $70-11^{\text {b }}$ | Insufficient Data | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | $\begin{gathered} \text { Insufficient } \\ \text { Data } \end{gathered}$ | 0.00 | 0.00 | No UP | None | None | None | N/A |

## Table 17: Final Freight Needs (continued)

| Segment \# | Performance Score and Level of Need |  |  |  |  |  | Initial Segment Need* | Hot Spots | Relevant Recently Completed or Under Construction Projects (which supersede performance data)* | Final Segment Need* |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Freight Index | Directional TTTR |  | Closure Duration |  | Bridge Clearance |  |  |  |  |
|  |  | NB/EB | SB/WB | NB/EB | SB/WB |  |  |  |  |  |
| $70-12^{\text {b }}$ | Insufficient Data | Insufficient Data | Insufficient Data | 17.39 | 0.00 | No UP | None | None | None | N/A |
| 70/60E-13 ${ }^{\text {a }}$ | 1.58 | 1.16 | 1.15 | 22.75 | 26.52 | 15.84 | Low | $\begin{gathered} 1 \text { (Pinal SPRR } \\ \text { UP - MP } \\ 253.63, \# 0562 \text { ) } \end{gathered}$ | FY19 MP 247 New DMS Sign EB (AZ State-wide DMS Plan); and MP 243-252 Construct new sidewalks on northside (Cobre Valley Comprehensive Transportation Study) | Low |
| $60 \mathrm{E}-14{ }^{\text {b }}$ | 1.49 | 1.12 | 1.17 | 63.60 | 344.95 | 13.03 | High | 1 (Queen Creek Tunnel) | Low | High |
| $60 \mathrm{E}-15^{\text {b }}$ | 1.32 | 1.18 | 1.14 | 0.00 | 90.50 | 16.79 | Medium | None | Low | Medium |
| $60 \mathrm{E}-16{ }^{\text {b }}$ | 1.28 | 1.05 | 1.12 | 52.20 | 12.25 | No UP | Low | None | Low | Low |
| $60 \mathrm{E}-17{ }^{\text {b }}$ | 1.18 | 1.05 | 1.09 | 3.27 | 61.40 | No UP | None | None | Picket Post- Construct new EB lanes parallel to existing, between Reymert Wash and Queen Creek and Gonzales Pass- Construct new EB lanes west of the summit, construct new WB lanes east of the summit | None |
| $60 \mathrm{E}-18{ }^{\text {b }}$ | 1.22 | 1.12 | 1.05 | 0.00 | 22.29 | No UP | Low | None | None | Low |
| $60 \mathrm{E}-19^{\text {a }}$ | 1.63 | 1.20 | 1.14 | 14.00 | 20.30 | No UP | Low | None | None | Low |
| $60 \mathrm{E}-20^{\text {b }}$ | 1.20 | 1.06 | 1.06 | 74.94 | 7.11 | No UP | Low | None | None | Low |


| Level of Need (Score) |  | Performance Score Need Scale |  |  | Segment Level Need Scale |
| :---: | :---: | :---: | :---: | :---: | :---: |
| None* <br> (0) | a | $\leq 1.58$ | $\leq 71.07$ | $\geq 16.33$ | 0 |
|  | b | $\leq 1.22$ |  |  |  |
| Low (1) | a | 1.58-1.72 | 71.07-97.97 | $\begin{gathered} 16.17- \\ 16.33 \end{gathered}$ | $\leq 1.5$ |
|  | b | 1.22-1.28 |  |  |  |
| Medium(2) | a | $1.72-1.98$ | 97.97-151.75 | $\begin{gathered} 15.83- \\ 16.17 \end{gathered}$ | 1.5-2.5 |
|  | b | 1.28-1.42 |  |  |  |
| High (3) | a | $\geq 1.98$ | $\geq 151.75$ | $\leq 15.83$ | $\geq 2.5$ |
|  | b | $\geq 1.42$ |  |  |  |

a: Interrupted Flow
b: Uninterrupted Flow
*A segment need rating of 'None' does not indicate a lack of needed improvements; rather, it indicates that the segment performance score exceeds the established performance thresholds and strategic solutions for that segment will not be developed as part of this study.

Segment Review
The needs for each segment were combined to numerically estimate the average level of need for each segment of the corridor. Table 18 provides a summary of needs for each segment across all performance areas, with the overall average need for each segment presented in the last row. All of the segments showed a Low level of average need.

Table 18: Summary of Needs by Segment

| Performance Area | Segment Number and Mileposts (MP) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 191-1 | 191-2 | 191-3 | 191-4 | 191-5 | 70-6 | 70-7 | 70-8 | 70-9 | 70-10 | 70-11 | 70-12 | 70\|60-13 | 60-14 | 60-15 | 60-16 | 60-17 | 60-18 | 60-19 | 60-20 |
|  | $\begin{aligned} & \text { MP } \\ & 0-24 \end{aligned}$ | $\begin{gathered} \text { MP } \\ 24-67 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 87-104 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 104-116 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 116-121 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 339-330 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 330-300 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 300-298 \end{gathered}$ | $\begin{gathered} \text { MP } \\ \text { 298-293 } \end{gathered}$ | $\begin{gathered} \text { MP } \\ 293-274 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 274-270 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 270-255 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 255-243 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 243-227 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 227-225 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 225-223 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 223-212 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 212-205 \end{gathered}$ | $\begin{gathered} \text { MP } \\ \text { 205-199 } \end{gathered}$ | $\begin{gathered} \hline \text { MP } \\ 199- \\ 194.3 \\ \hline \end{gathered}$ |
| Pavement | Medium | High | Low | Low | Medium | Medium | High | High | High | High | High | Low | Medium | Low | None* | None* | Low | Medium | Low | None* |
| Bridge | None* | Medium | Low | Low | N/A | Low | Low | None* | N/A | None* | Low | Low | Medium | Medium | None* | Medium | Low | Low | Low | None* |
| Mobility+ | Low | None* | Low | None* | Low | Low | Low | Low | Low | Low | Low | Low | Low | High | Low | Low | None* | Low | High | High |
| Safety+ | None* | None* | Low | Low | None* | Low | None* | N/A ${ }^{\#}$ | N/A\# | High | High | High | High | High | N/A\# | N/A ${ }^{\text {\# }}$ | High | Low | Medium | Medium |
| Freight+ | High | N/A ${ }^{\text {\# }}$ | N/A ${ }^{\text {\# }}$ | N/A ${ }^{\#}$ | N/A ${ }^{\text {\# }}$ | N/A ${ }^{\text {\# }}$ | N/A ${ }^{\#}$ | N/A ${ }^{\#}$ | N/A ${ }^{\text {\# }}$ | N/A ${ }^{\#}$ | N/A ${ }^{\#}$ | N/A ${ }^{\#}$ | Low | High | Medium | Low | None* | Low | Low | Low |
| Average Need | 1.23 | 0.77 | 0.77 | 0.54 | 0.54 | 0.92 | 0.85 | 0.69 | 0.69 | 1.38 | 1.54 | 1.23 | 1.77 | 2.54 | 0.69 | 0.77 | 1.00 | 1.15 | 1.69 | 1.38 |

*A segment need rating of 'None' does not indicate a lack of needed improvements; rather, it indicates that the segment performance score exceeds the established performance thresholds and strat solutions for that segment will not be
developed as part of this study.

+ Identified as an emphasis area for the US 60|US 70|US 191 corridor
\# N/A indicates insufficient or no data available to determine level of need

| Average Need Scale |  |
| :---: | :---: |
| None $^{*}$ | $<0$ |
| Low | $0.1-1.0$ |
| Medium | $1.0-2.0$ |
| High | $>2.0$ |

Summary of Corridor
The needs in each performance area are shown in Figure 21 and summarized below:

## Pavement Needs

- Seventeen segments (191-1 through 70|60-14 and 60-17 through 60-19) contain Pavement hot spots
- Segments 60-15, 60-16 and 60-20 have a final need of None, Segments 191-3, 191-4, 7012, 60-14, 60-17 and 60-19 have a final need of Low, Segments 191-1, 191-5, 70-6, 70|6013 and 60-18 have a final need of Medium, and Segments 191-2, and 70-7 through 70-11 show a final level need of High
- A high level of historical investment has occurred on Segments 191-3 and 70-9 and a medium level of historical investment has occurred through Segments 191-2, 191-4, 191-5, 70-8, 7010 , and 70-12 through 60-17


## Bridge Needs

- Bridge needs were identified on four segments of the corridor, 73 miles (30\%) with a "Medium" level of bridge need.
- Seven bridges showed potential repetitive investment issues and may be candidates for lifecycle cost analysis to evaluate alternative solutions.
- Three bridges have bridge ratings of 4: Pinal Creek Bridge (No. 266), Waterfall Canyon Bridge (No. 328), and Queen Creek Bridge (No. 406).
- Eleven bridges were defined as hot spots since they had multiple bridge ratings of 5 or less or one bridge rating of 4 .
- Of the eleven hot spot bridges, six also showed repetitive investment issues. These included the Holyoak Wash Bridge (No. 514), Pinal Creek Bridge (No. 549), Pinal Creek Bridge (No. 36), Pinal Creek Bridge (No. 266), Waterfall Canyon Bridge (No. 328) and Queen Creek Bridge (No. 406)


## Mobility Needs

- Mobility Performance is an Emphasis Area for the US 60| US 70| US 191 corridor, giving it a heavier weight in the analysis
- Segments 60-14, 60-19 and 60-20 have a final segment need of High; all other segments on the corridor have a final segment need of Low or None
- There is lack of bicycle accommodation along $60 \%$ of the corridor
- Mobility needs are primarily due to mobility, future $\mathrm{v} / \mathrm{c}$, and existing $\mathrm{v} / \mathrm{c}$ issues


## Safety Needs

- Safety Performance is an Emphasis Area for the US 60| US 70| US 191 corridor, giving it a heavier weight in the analysis.
- Segments 70-10 through 60-14, 60-17, 60-19 and 60-20 have a final segment need of Medium or High; all other segments on the corridor have a final segment of Low or None
- Safety hot spots exist in Segments 70-6, 60|70-13, 60-14, and 60-17 through 60-20
- There is insufficient data to generate reliable ratings for the secondary measures including crashes at intersections, involving pedestrians, involving trucks, or involving bicycles Freight Needs
- Freight Performance is an Emphasis Area for the US 60| US 70| US 191 corridor, giving it a heavier weight in the analysis
- Segments 191-1, 60-14 and 60-15 have a final segment need of Medium or High; all other segments on the corridor have a final segment need of Low or None
- Freight needs are primarily due to Freight TTTR, Closure Duration and Bridge Clearance
- There are two freight hot spots along the corridor: Pinal SR UP and Queen Creek Tunnel Overlapping Needs
This section identifies overlapping performance needs on the US 60|US 70|US 191 corridor, which provides guidance to develop strategic solutions that address more than one performance area with elevated levels of need. Completing projects that address multiple needs presents the opportunity to improve overall performance more effectively. A summary of the overlapping needs that relate to locations with elevated levels of need is provided below:
- All segments on the corridor have overlapping needs. Traffic counters do not exist in Segments 191-2 through 70-12, approximately 161 miles or $66 \%$ of the corridor, resulting in insufficient data to calculate needs in the freight performance area for those locations.
- US 60|70 MP 243 to MP 255 (Segment 70|60-13) and US 60 MP 227 to MP 243 (Segment 60-14) have overlapping needs in all five performance areas. These segments comprised 28 of the 246 corridor miles
- Segment 70|60-13 has an overall Medium need score on the corridor. Some needs are site specific while others are characteristics of the segment. Medium bridge needs are related to the Bloody Tanks Bridge (No. 173), Pinal Creek Bridge (No. 36), Pinal Creek Bridge (No. 266), Pinal Creek Bridge (No. 549) and McMillen Wash Bridge (No. 1028) which are hot spots due to poor structural ratings and exhibit high repetitive investment. High safety needs are due to the number of fatal or suspected serious injury collisions exceeding the statewide average which are due to failure to yield right-of-way and involve vehicles running off the road (left). Low freight needs are due to the bridge vertical clearance for the Pinal SPRR UP (No. 0562)
- Segment 60-14 has an overall High need and the highest need score in the corridor. This segment has significant grades and subsequently suffers from freight, safety and mobility needs related to delay and incidents/accidents associated with the grade along with speeding too fast for conditions. The segment includes 2 hot spot bridges, both which do not have repetitive investment histories. The Queen Creek Tunnel, also located in the segment affects bridge and freight needs with low vertical clearance.
- Segments $60-19$ registers an overall Medium need score on the corridor with overlapping mobility and safety needs. Medium safety needs are due to the number of fatal or suspected serious injury collisions exceeding the statewide average which are due to dark-lighted conditions and involve vehicles running of the road (left). High mobility needs are due to poor bicycle accommodation and poor existing peak hour volume-to-capacity ratio.


## ADOT

## Figure 21: Corridor Needs Summary



### 4.0 STRATEGIC SOLUTIONS

The principal objective of the CPS is to identify strategic solutions (investments) that are performance-based to ensure that available funding resources are used to maximize the performance of the State's key transportation corridors. One of the first steps in the development of strategic solutions is to identify areas of elevated levels of need (i.e., Medium or High). Addressing areas of Medium or High need will have the greatest effect on corridor performance and are the focus of the strategic solutions. Segments with Medium or High needs and specific locations of hot spots are considered strategic investment areas for which strategic solutions should be developed. Segments with lower levels of need or without identified hot spots are not considered candidates for strategic investment and are expected to be addressed through other ADOT programming processes. The US 60|US 70|US 191 strategic investment areas (resulting from the elevated needs) are shown in Figure 22

### 4.1 Screening Process

This section examines qualifying strategic needs and determines if the needs in those locations require action. In some cases, needs that are identified do not advance to solutions development and are screened out from further consideration because they have been or will be addressed through other measures including

- A project is programmed to address this need
- The need is a result of a Pavement or Bridge hot spot that does not show historical investment or rating issues; these hot spots will likely be addressed through other ADOT programming means.
- A bridge is not a hot spot but is located within a segment with a Medium or High level of need; this bridge will likely be addressed through current ADOT bridge maintenance and preservation programming processes.
- The need is determined to be non-actionable (i.e., cannot be addressed through an ADOT project)
- The conditions/characteristics of the location have changed since the performance data was collected that was used to identify the need
Table 19 notes if each potential strategic need advanced to solution development, and if not, the reason for screening the potential strategic need out of the process. Locations advancing to solutions development are marked with Yes (Y); locations not advancing are marked with No ( N ) and highlighted. This screening table provides specific information about the needs in each segment that will be considered for strategic investment. The table identifies the level of need either Medium or High segment needs, or segments without Medium or High level of need that have a hot spot. Each area of need is assigned a location number in the screening table to help document and track locations considered for strategic investment.


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## Figure 22: Strategic Investment Areas



Table 19: Strategic Investment Area Screening

| Segment \# and MP | Level of Strategic Need |  |  |  |  | Location <br> \# | Type | Need Description | Advance (YIN) | Screening Description |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Pavement | Bridge | Mobility | Safety | Freight |  |  |  |  |  |
| $\begin{gathered} 191-1 \\ \text { (MP 0-24) } \end{gathered}$ | Medium | - | - |  | High | L1 | Pavement | $71 \%$ area failure and numerous Hot Spots throughout the segment in both directions (MP 0-5 Both; MP 5-6 NB; MP 6-7 Both; MP 7-8 NB, MP 9-10 Both; MP $12-15$ NB; MP15-20 Both; MP 20-22 NB; MP 22-23 SB; MP24-23 Both) | N | No high historical investment so not considered a strategic investment; will likely be addressed by current ADOT processes |
|  |  |  |  |  |  | L2 | Freight | Extremely poor Truck Travel Time reliability in the segment in both directions, with indexes approaching double the threshold, influenced by weigh station lines and wait times. | N | No programmed project to address freight need because freight need was due to weigh station |
| $\begin{gathered} 191-2 \\ \text { (MP 24-67) } \end{gathered}$ | High | Medium | - |  | - | L3 | Pavement | Failure in a high percentage of surface area. Hot Spots throughout the segment in both directions (MP 24-27 Both; MP 27-28 SB; MP28-29 NB; MP 2932 Both; MP 32-33 SB; MP 33-35 Both; MP 35-36 SB; MP 42-43 SB MP 45-46 Both; MP 48-49 SB; MP 50-62 Both) | N | A medium level of historical investment has occurred on Segment 191-2 according to PeCOS data and recent pavement preservation projects. No pavement preservation projects are currently programmed for this portion of the segment. Anticipated to be addressed through current ADOT pavement maintenance and preservation programming processes. |
|  |  |  |  |  |  | L4 | Bridge | Medium level of need related to deck rating $=5$. The bridge was not identified as a Hot Spot. <br> Cochise UPRR OP (MP 62.88, \#157) | N | No high historical investment so not considered a strategic investment; will likely be addressed by current ADOT processes |
| $\begin{aligned} & \text { 191-3 } \\ & \text { (MP 87-1 } \\ & \text { 104) } \end{aligned}$ | Hot Spots | - | - | - | - | L5 | Pavement | All hot spots are either in the northbound direction or in both directions, and span nearly the entire segment (MP 88-89 NB; MP 89-92 Both; MP94-95 NB; MP95-101 Both; MP101-104 NB) | Y | High historical investment; meets criteria for strategic investment |
| $\begin{gathered} 191-4 \\ \text { (MP 104- } \\ 116 \text { ) } \end{gathered}$ | Hot Spot | - | - | - | - | L6 | Pavement | Hot spot (MP 104-109) in both directions | N | No high historical investment so not considered a strategic investment; will likely be addressed by current ADOT processes |

Legend: $\qquad$ Strategic investment area screened out from further consideration.

Table 19: Strategic Investment Area Screening (continued)

| Segment \# and MP | Level of Strategic Need |  |  |  |  | Location <br> \# | Type | Need Description | Advance <br> (YIN) | Screening Description |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Pavement | Bridge | Mobility | Safety | Freight |  |  |  |  |  |
| $\begin{gathered} 191-5 \\ (\text { MP 116- } \\ 121) \end{gathered}$ | Medium | - | - |  | - | L7 | Pavement | 80\% area failure, with Hot Spot in NB lanes MP 117-121 | N | No high historical investment so not considered a strategic investment; will likely be addressed by current ADOT processes |
| $\begin{gathered} 70-6 \\ (\text { MP 339- } \\ 330) \end{gathered}$ | Medium |  | - | $\begin{aligned} & \text { Hot } \\ & \text { Spot } \end{aligned}$ |  | L8 | Pavement | $60 \%$ area failure, with Hot Spots in both directions throughout the segment (MP 330-332 <br> SB; MP332-333 Both; MP $333-335$ SB; MP 335-336 NB; MP 336-337 Both; MP 338-339 SB; <br> MP 339-340 Both) | N | A low level of historical investment has occurred on Segment 70-6. No pavement preservation projects are currently programmed for this portion of the segment. Anticipated to be addressed through current ADOT pavement maintenance and preservation programming processes. |
|  |  |  |  |  |  | L9 | Safety | Cluster of crashes in both directions from MP 336.5 to the junction with US 191. Eleven fatal crashes and one suspected serious injury crash; $25 \%$ involve failure to yield right-of-way, $17 \%$ involve failure to keep in proper lane, $33 \%$ occurred in dark-lighted conditions, $25 \%$ involve a first unit event of ran off road right, $17 \%$ involve a first unit event of collision with pedestrian, $17 \%$ involve illness | Y | No programmed project to address Safety need |
| $\begin{gathered} 70-7 \\ (\text { MP } 330- \\ 300) \end{gathered}$ | High | $\begin{aligned} & \text { Hot } \\ & \text { Spot } \end{aligned}$ | - | - | - | L10 | Pavement | Failure in a high percentage of surface area. Hot Spots in both directions throughout the segment (MP 300-314 Both; MP314-315 EB; MP 327-329 EB; MP 329-330 Both) | N | No high historical investment so not considered a strategic investment; will likely be addressed by current ADOT processes |
|  |  |  |  |  |  | L11 | Bridge | Hot Spot at Holyoak Wash Bridge (MP 302.53, \#514) | N | No high historical investment so not considered a strategic investment; will likely be addressed by current ADOT processes |
| $\begin{gathered} 70-8 \\ (M P 300 \\ 298) \end{gathered}$ | High | - | - | - | - | L12 | Pavement | Poor pavement index and $100 \%$ area failure. The entire segment is a hot spot in both directions. | N | No high historical investment so not considered a strategic investment; will likely be addressed by current ADOT processes |
| $\begin{gathered} 70-9 \\ (\text { MP 298 - } \\ \text { 293) } \end{gathered}$ | High |  | - |  |  | L13 | Pavement | Poor pavement index and $100 \%$ area failure. The entire segment is a hot spot in both directions. | Y | High historical investment; meets criteria for strategic investment |
| Legend |  | Str | gic inves | ment | scree | d out fro | m further | nsideration. |  |  |

## Table 19: Strategic Investment Area Screening (continued)

| Segment \# and MP | Level of Strategic Need |  |  |  |  | Location <br> \# | Type | Need Description | Advance <br> (YIN) | Screening Description |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Pavement | Bridge | Mobility | Safety | Freight |  |  |  |  |  |
| $\begin{gathered} 70-10 \\ \text { (MP 293- } \\ 274 \end{gathered}$ | High | - | - | High | - | L14 | Pavement | MP 274-293 has a High level of need due to poor performance scores for Pavement Index and fair performance scores for directional PSR; segment also has poor \% Area Failure ratings <br> Hot Spots in both directions MP 274-275 and MP 279-293 | N | No high historical investment so not considered a strategic investment; will likely be addressed by current ADOT processes |
|  |  |  |  |  |  | L15 | Safety | Four fatal crashes in segment; $14 \%$ involved pedal cyclist, $14 \%$ involved failure to yield right-of-way, $14 \%$ were head on crashes, $29 \%$ occurred in dark-unlit conditions, $29 \%$ involve a first unit event of ran off the road (right), $29 \%$ involve overturn, $29 \%$ under the influence of drugs or alcohol | Y | No programmed project to address Safety need |
| $\begin{aligned} & 70-11 \\ & \text { (MP 274 } \\ & -270) \end{aligned}$ | High | - | - | High | - | L16 | Pavement | MP 270-274 has a High level of need due to poor performance scores for Pavement Index and fair performance scores for directional PSR; segment also has poor \% Area Failure ratings <br> Hot Spots in both directions MP 270-271 and MP 271-274 | N | No high historical investment so not considered a strategic investment; will likely be addressed by current ADOT processes |
|  |  |  |  |  |  | L17 | Safety | Two fatal crashes in this segment; Both crashes involve a pedestrian, both crashes involve driver under the influence of drugs or alcohol, both crashes occurred during dark-unlit conditions, both crashes involve driving in opposing lane | N | Need considered non-actionable because fatal crashes involved drugs or alcohol |
| $\begin{gathered} 70-12 \\ (\text { MP270- } \\ 255) \end{gathered}$ | Hot Spot | - | - | High | - | L18 | Pavement | Hot Spots: MP $255-256$ EB, MP 256 - 257 both directions, MP 257-258 WB, MP 258260 both directions, MP 266-268 EB | N | No high historical investment so not considered a strategic investment; will likely be addressed by current ADOT processes |
|  |  |  |  |  |  | L19 | Safety | Seven fatal crashes and two suspected serious injury crashes in segment; $33 \%$ involved a pedestrian, $33 \%$ involve speed too fast for conditions, $33 \%$ occurred in dark-unlit conditions, $22 \%$ involved overturning, $11 \%$ involved driver falling asleep/fatigued, $22 \%$ involve rear end | Y | No programmed project to address Safety need |

Legend: $\qquad$ Strategic investment area screened out from further consideration.

Table 19: Strategic Investment Area Screening (continued)


Table 19: Strategic Investment Area Screening (continued)

| Segment \# and MP | Level of Strategic Need |  |  |  |  | Location <br> \# | Type | Need Description | Advance <br> (YIN) | Screening Description |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Pavement | Bridge | Mobility | Safety | Freight |  |  |  |  |  |
| $\begin{gathered} 60-14 \\ (\mathrm{MP243-} \\ 227) \end{gathered}$ | Hot Spot | Medium | High | High | High | L28 | Pavement | Hot Spot: MP 236-243 both directions | N | No high historical investment so not considered a strategic investment; will likely be addressed by current ADOT processes |
|  |  |  |  |  |  | L29 | Bridge | Queen Creek Bridge MP 227.71 (\#406) has deck and superstructure ratings of 4 and substructure rating of 3 | N | No high historical investment so not considered a strategic investment; will likely be addressed by current ADOT processes |
|  |  |  |  |  |  | L30 | Bridge | Waterfall Canyon Bridge MP 229.50 (\#328) has substructure rating of 5 and superstructure rating of 4 | N | No high historical investment so not considered a strategic investment; will likely be addressed by current ADOT processes |
|  |  |  |  |  |  | L31 | Mobility | Mobility Index, Future Daily V/C, Existing Peak Hour V/C in both directions, Closure Extent in both directions and Bicycle Accommodation performance are below average. There were 99 closures along this segment | N | Construct alternative alignment/widen to 4 lanes (MP 227-243) programmed in FY 2030 |
|  |  |  |  |  |  | L32 | Safety | Five fatal crashes and twenty-one crashes in segment; Hot Spots at MP 241-242.6 and MP 227-232.3; 54\% collision with fixed object, $19 \%$ head on, $50 \%$ speed too fast for conditions, $15 \%$ involve drove in opposing lane, $12 \%$ involve wet conditions, $27 \%$ involve overturning, $19 \%$ involve ran off the road, $31 \%$ involve under the influence of drugs or alcohol | Y | No programmed project to address Safety need |
|  |  |  |  |  |  | L33 | Freight | MP 227-243 has a High level of need due to poor performance scores for Freight Index and for SB/WB closure duration | N | Lane alignment/widening project (MP 227-243) programmed in FY 2030 is expected to address freight needs. |
|  |  |  |  |  |  | L34 | Freight | Vertical clearance hot spot at Queen Creek Tunnel (\#538, MP 339.20 ) has low vertical clearance of 13.03 feet and cannot be ramped around | Y | No programmed project to address Freight need |
| $\begin{gathered} 60-15 \\ (\text { MP227- } \\ 225) \end{gathered}$ | - | - | - | - | Medium | L35 | Freight | MP 225-227 has a Medium level of need due to fair performance scores for Freight Index, NB/EB directional TTTR and for SB/WB closure duration | N | Lane alignment/widening project (MP 225-227) programmed in FY 2030 is expected to address freight needs. |
| Legend: | Strategic investment area screened out from further consideration. |  |  |  |  |  |  |  |  |  |

Table 19: Strategic Investment Area Screening (continued)


Legend: $\qquad$ Strategic investment area screened out from further consideration.

Table 19: Strategic Investment Area Screening (continued)

| Segment \# and MP | Level of Strategic Need |  |  |  |  | Location <br> \# | Type | Need Description | Advance (YIN) | Screening Description |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Pavement | Bridge | Mobility | Safety | Freight |  |  |  |  |  |
| $\begin{gathered} 60-19 \\ (\text { MP 199- } \\ \text { 205) } \end{gathered}$ | Hot Spot | - | High | Medium |  | L44 | Pavement | Hot Spots: MP 199-201 WB, MP 201-202 EB, MP 204-205 WB | N | No high historical investment so not considered a strategic investment; will likely be addressed by current ADOT processes |
|  |  |  |  |  |  | L45 | Mobility | Mobility Index, Existing Peak Hour V/C in the SB/EB direction and Bicycle Accommodation performance are below average. | Y | No programmed project to address mobility need |
|  |  |  |  |  |  | L46 | Safety | Two fatal crashes and eight suspected serious injury crashes; Hot spot at MP 200.4-203.5; $20 \%$ involve collision with a fixed object, $60 \%$ involve exceeded lawful speed, $33 \%$ occurred in dark-lighted conditions, $40 \%$ involve overturn, $20 \%$ involve ran off the road (left), $30 \%$ under the influence of drugs or alcohol | Y | No programmed project to address safety need |
| $\begin{gathered} 60-20 \\ \text { (MP } \\ 194.3 \\ 199) \end{gathered}$ | - | - | High | Medium |  | L47 | Mobility | Mobility Index, Future Daily V/C, Closure Extent in the NB/WB and Bicycle Accommodation performance are below average. Majority of the closures due to crashes and accidents | Y | No programmed project to address mobility need |
|  |  |  |  |  | - | L48 | Safety | Four fatal crashes in the segment; Hot spot at MP 195-197; 17\% involve collision with a fixed object, $67 \%$ involve exceeded lawful speed, $17 \%$ involve other unsafe passing, $33 \%$ occur in dark-lighted conditions, $33 \%$ involve ran off the road (left), $20 \%$ fatiguedffell asleep, $30 \%$ under the influence of drugs/alcohol | Y | No programmed project to address safety need |

Legend: $\qquad$ Strategic investment area screened out from further consideration.

### 4.2 Candidate Solutions

For each elevated need within a strategic investment area that is not screened out, a candidate solution is developed to address the identified need. Each candidate solution is assigned to one of the following three P2P investment categories based on the scope of the solution:

- Preservation
- Modernization
- Expansion

Documented performance needs serve as the foundation for developing candidate solutions for corridor preservation, modernization, and expansion. Candidate solutions are not intended to be a substitute or replacement for traditional ADOT project development processes where various ADOT technical groups and districts develop candidate projects for consideration in the performance-based programming in the P2P process. Rather, these candidate solutions are intended to complement ADOT's traditional project development processes through a performance-based process to address needs in one or more of the five performance areas of Pavement, Bridge, Mobility, Safety, and Freight. Candidate solutions developed for the US 60|US 70|US 191 corridor will be considered along with other candidate projects in the ADOT statewide programming process.

Characteristics of Strategic Solutions
Candidate solutions should include some or all of the following characteristics:

- Do not recreate or replace results from normal programming processes
- May include programs or initiatives, areas for further study, and infrastructure projects
- Address elevated levels of need (High or Medium) and hot spots
- Focus on investments in modernization projects (to optimize current infrastructure)
- Address overlapping needs
- Reduce costly repetitive maintenance
- Extend operational life of system and delay expansion
- Leverage programmed projects that can be expanded to address other strategic elements
- Provide measurable benefit


## Candidate Solutions

UA set of 14 candidate solutions are proposed to address the identified needs on the US 60|US 70|US 191 corridor.
Table 20 identifies each strategic location that has been assigned a candidate solution with a number (e.g., CS8.1, CS8.2, etc.). Each candidate solution is comprised of one or more components to address the identified needs. The assigned candidate solution numbers are linked to the location number and provide tracking capability through the rest of the process. The locations of proposed solutions are shown on the map in Figure 23.

Candidate solutions developed to address an elevated need in the Pavement or Bridge performance area will include two options; rehabilitation or full replacement. These solutions are initially evaluated through a Life-Cycle Cost Analysis (LCCA) to provide insights into the costeffectiveness of these options so a recommended approach can be identified. Candidate solutions developed to address an elevated need in the Mobility, Safety, or Freight performance areas are advanced directly to the Performance Effectiveness Evaluation. In some cases, there may be multiple solutions identified to address the same area of need.

Candidate solutions that are recommended to expand or modify the scope of an already programmed project are noted and are not advanced to solution evaluation and prioritization. These solutions are directly recommended for programming.

## Table 20: Candidate Solutions

| Candidate Solution \# | Segmen t \# | Locatio n \# | Beginning Milepost | Ending Milepost | Candidate Solution Name | Option | Scope | Investment Category (Preservation [P], Modernization [M], Expansion [E]) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CS191.1 | 191-3 | L5 | 88 | 104 | US191 Pavement Preservation South of Safford | A | Rehabilitate/repair pavement | P |
|  |  |  |  |  |  | B | Replace pavement | M |
| CS70.2 | 70-6 | L9 | 339 | 336.5 | East Safford Safety Improvements | - | Provide flashing traffic signal warning signs at Milepost 337.82 and Milepost 338.03. Consider installing feedback signs in both directions at $20^{\text {th }}$ Avenue | M |
| CS70.3 | 70-9 | L13 | 298 | 293 | Bylas Area Pavement Preservation | A | Rehabilitate/repair pavement | P |
|  |  |  |  |  |  | B | Replace pavement | M |
| CS70.4 | 70-10 | L15 | 293 | 274 | Bylas to Peridot Safety Improvements | - | Widen shoulders Milepost 274-278 | M |
|  |  |  |  |  |  |  | Install centerline rumble strips MP 275.5-276.5,MP 279.5-287.5 |  |
|  |  |  |  |  |  |  | Install shoulder rumble strips MP 275.5-276.5,MP 279.5-287.5 |  |
|  |  |  |  |  |  |  | Install high visibility striping and delineators from milepost 274-278 |  |
|  |  |  |  |  |  |  | Improve existing pedestrian / speed warning signs to also include flashing beacons and speed feedback signs (MP 292,MP 280, MP 278.5) |  |
|  |  |  |  |  |  |  | Construct passing lanes (WB MP 288.2-289.6) |  |
|  |  |  |  |  |  |  | Formalize pullouts (signage, ROW for pullouts) (WB MP 274.5, EB MP 279, EB MP 289, WB 292) |  |
| CS70.5 | 70-12 | L19 | 270 | 255 | East of Globe Safety Improvements | - | Widen shoulders MP 255-270 | M |
|  |  |  |  |  |  |  | Install centerline and shoulder rumble strips MP 255-270 <br> Install improved lighting from milepost 269-270 <br> Construct passing lane in each direction (MP 255-256) |  |
|  |  |  |  |  |  |  | Improve existing pedestrian and speed warning signs to include flashing beacons and speed feedback signs (MP 269.25) |  |
| CS70\|60.6 | 70\|60-13 | L26 | 255 | 243 | Globe Area Safety Improvements | - | Consider installing speed feedback signs (2 EB and 2 WB between MP 246-250) High visibility striping <br> Install signal ahead warning signs with beacons in advance of SR 188 intersection Construct passing lane in each direction from MP 243-243.25 and MP 253.6-255 | M |

[^1]
## Table 20: Candidate Solutions (continued)

| Candidate Solution \# | Segmen t \# | Locatio n \# | Beginning Milepost | Ending Milepost | Candidate Solution Name | Option | Scope | Investment Category (Preservation [P], Modernization [M], Expansion [E]) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CS70\|60.7 | 70\|60-13 | L27 | 255 | 243 | Globe Area <br> Freight <br> Improvements | A | Reconstruct Pinal SPRR UP to increase vertical clearance | E |
|  |  |  |  |  |  | B | Reprofile mainline to increase vertical clearance | M |
| CS60.8 | 60-14 | L32 | 243 | 227 | Superior East Area Safety Improvements | - | Consider installing speed feedback signs at MP 229.9, MP 236, MP 241 <br> Install centerline rumble strips at MP 229-231 <br> Install high visibility striping and delineators MP 228-228.3 and MP 241-242 <br> Install edge line rumble strips EB MP 228.17-228.3, MP 229.2-229.26, and MP 247-247.26 | M |
| CS60.9 | 60-14 | L34 | 243 | 227 | Superior East Area Freight Improvements | A | Reconstruct Queen Creek Tunnel to increase vertical clearance | E |
|  |  |  |  |  |  | B | Reprofile mainline to increase vertical clearance | M |
| CS60.10 | 60-17 | L39 | 223 | 212 | Superior Area Safety Improvements | - | Install lighting at N Queen Valley Road and US 60 intersection Consider installing speed feedback sign MP 212.5 Install chevrons or curve warning sign at MP 219.33 | M |
| CS60.11 | 60-18 | L43 | 208 | 206 | US-60 SW of Gold Canyon Safety Improvements | - | Install lighting MP 205-207 <br> Consider installing speed feedback sign Widen inside shoulder 208.3-212 | M |
| CS60.12 | 60-19 | $\begin{gathered} \mathrm{L} 45 \text { / } \\ \mathrm{L} 46 \end{gathered}$ | 205 | 199 | Gold Canyon Area <br> Mobility and <br> Safety <br> Improvements | - | Add SB/EB through lane MP 199.12 to 206 <br> Widen shoulders MP 199.12 to 205 <br> Consider installing speed feedback sign at MP 201 Install lighting MP 201-202 | E |
| CS60.13 | 60-20 | $\begin{gathered} \mathrm{L} 47 \text { / } \\ \mathrm{L} 49 \end{gathered}$ | 199 | 194.3 | Apache Junction Area Mobility and Freight Improvements | - | Add through lane in NB/WB direction | M |
| CS60.14 | 60-20 | L48 | 199 | 194.3 | Apache Junction Area Safety Improvements | - | Install inside and edge line rumble strips through entire segment Consider installing speed feedback sign MP 195 | M |

[^2]
## Figure 23: Candidate Solutions



### 5.0 SOLUTION EVALUATION AND PRIORITIZATION

Candidate solutions are evaluated using the following steps: LCCA (where applicable), Performance Effectiveness Evaluation, Solution Risk Analysis, and Candidate Solution Prioritization. The methodology and approach to this evaluation are shown in Figure 24 and described more fully below.

## Life-Cycle Cost Analysis

All Pavement and Bridge candidate solutions have two options: rehabilitation/repair or reconstruction. These options are evaluated through an LCCA to determine the best approach for each location where a Pavement or Bridge solution is recommended. The LCCA can eliminate options from further consideration and identify which options should be carried forward for further evaluation.

When multiple independent candidate solutions are developed for Mobility, Safety, or Freight strategic investment areas, these candidate solution options advance directly to the Performance Effectiveness Evaluation without an LCCA
Performance Effectiveness Evaluation
After completing the LCCA process, all remaining candidate solutions are evaluated based on their performance effectiveness. This process includes determining a Performance Effectiveness Score (PES) based on how much each solution impacts the existing performance and needs scores for each segment. This evaluation also includes a Performance Area Risk Analysis to help differentiate between similar solutions based on factors that are not directly addressed in the performance system.
Solution Risk Analysis
All candidate solutions advanced through the Performance Effectiveness Evaluation are also evaluated through a Solution Risk Analysis process. A solution risk probability and consequence analysis is conducted to develop a solution-level risk weighting factor. This risk analysis is a numeric scoring system to help address the risk of not implementing a solution based on the likelihood and severity of performance failure.

## Candidate Solution Prioritization

The PES, weighted risk factor, and segment average need score are combined to create a prioritization score. The candidate solutions are ranked by prioritization score from highest to lowest. The highest prioritization score indicates the candidate solution that is recommended as the highest priority. Solutions that address multiple performance areas tend to score higher in this process.

Figure 24: Candidate Solution Evaluation Process


### 5.1 Life-Cycle Cost Analysis

LCCA is conducted for any candidate solution that is developed as a result of a need in the Pavement or Bridge Performance Area. The intent of the LCCA is to determine which options warrant further investigation and eliminate options that would not be considered strategic.

LCCA is an economic analysis that compares cost streams over time and presents the results in a common measure, the present value of all future costs. The cost stream occurs over an analysis period that is long enough to provide a reasonably fair comparison among alternatives that may differ significantly in scale of improvement actions over shorter time periods. For both bridge and pavement LCCA, the costs are focused on agency (ADOT) costs for corrective actions to meet the objective of keeping the bridge or pavement serviceable over a long period of time.

LCCA is performed to provide a more complete holistic perspective on asset performance and agency costs over the life of an investment stream. This approach helps ADOT look beyond initial and short-term costs, which often dominate the considerations in transportation investment decision making and programming.

## Bridge LCCA

For the bridge LCCA, three basic strategies are analyzed that differ in timing and scale of improvement actions to maintain the selected bridges, as described below:

- Bridge replacement (large upfront cost but small ongoing costs afterwards)
- Bridge rehabilitation until replacement (moderate upfront costs then small to moderate ongoing costs until replacement)
- On-going repairs until replacement (low upfront and ongoing costs until replacement)

The bridge LCCA model developed for the CPS reviews the characteristics of the candidate bridges including bridge ratings and deterioration rates to develop the three improvement strategies (full replacement, rehabilitation until replacement, and repair until replacement). Each strategy consists of a set of corrective actions that contribute to keeping the bridge serviceable over the analysis period. Cost and effect of these improvement actions on the bridge condition are essential parts of the model. Other considerations in the model include bridge age, elevation, pier height, length to span ratio, skew angle, and substandard characteristics such as shoulders and vehicle clearance. The following assumptions are included in the bridge LCCA model:

- The bridge LCCA only addresses the structural condition of the bridge and does not address other issues or costs
- The bridge will require replacement at the end of its 75 -year service life regardless of current condition
- The bridge elevation, pier height, skew angle, and length to span ratio can affect the replacement and rehabilitation costs
- The current and historical ratings are used to estimate a rate of deterioration for each candidate bridge
- Following bridge replacement, repairs will be needed every 20 year
- Different bridge repair and rehabilitation strategies have different costs, expected service life, and benefit to the bridge rating
- The Net Present Value (NPV) of future costs are discounted at $3 \%$ and all dollar amounts are in 2023 dollars
- If the LCCA evaluation recommends rehabilitation or repair, the solution is not considered strategic and the rehabilitation or repair will be addressed by normal programming processes
- Since this LCCA is conducted at a planning level, and due to the variabilities in costs and improvement strategies, the LCCA NPV results that are within $15 \%$ should be considered equally; in such a case, the solution should be carried forward as a strategic replacement project - more detailed scoping will confirm if replacement or rehabilitation is needed

Based on the candidate solutions presented in Table 20, LCCA was not conducted for any bridges on the US 60|US 70|US 191 corridor. A summary of this analysis is shown in Table 21 Additional information regarding the LCCA is included in Appendix E.

## Pavement LCCA

The LCCA approach to pavement is very similar to the process used for bridges. For the pavement LCCA, three basic strategies are analyzed that differ in timing and scale of improvement actions to maintain the selected pavement, as described below:

- Pavement replacement (large upfront cost but small ongoing costs afterwards - could be replacement with asphalt or concrete pavement)
- Pavement major rehabilitation until replacement (moderate upfront costs then small to moderate ongoing costs until replacement)
- Pavement minor rehabilitation until replacement (low upfront and ongoing costs unti replacement)

The pavement LCCA model developed for the CPS reviews the characteristics of the candidate paving locations including the historical rehabilitation frequency to develop potential improvement strategies (full replacement, major rehabilitation until replacement, and minor rehabilitation until replacement, for either concrete or asphalt, as applicable). Each strategy consists of a set of corrective actions that contribute to keeping the pavement serviceable over the analysis period. The following assumptions are included in the pavement LCCA model:

- The pavement LCCA only addresses the condition of the pavement and does not address other issues or costs
- The historical pavement rehabilitation frequencies at each location are used to estimate future rehabilitation frequencies
- Different pavement replacement and rehabilitation strategies have different costs and expected service life
- The NPV of future costs are discounted at 3\% and all dollar amounts are in 2023 dollars
- If the LCCA evaluation recommends rehabilitation or repair, the solution will not be considered strategic and the rehabilitation will be addressed by normal programming processes
- Because this LCCA is conducted at a planning level, and due to the variabilities in costs and improvement strategies, the LCCA NPV results that are within $15 \%$ should be considered equally; in such a case, the solution should be carried forward as a strategic replacement project - more detailed scoping will confirm if replacement or rehabilitation is needed

Based on the candidate solutions presented in Table 20, LCCA was conducted for two pavement projects on the US 60|US 70|US 191 corridor. A summary of this analysis is shown in Table 22. Additional information regarding the LCCA is contained in Appendix E.

As shown in Table 21 and Table 22, the following conclusions were determined based on the LCCA:

- Rehabilitation or repair was determined to be the most effective approach for Candidate Solution 191.1 (US191 Pavement Preservation South of Safford). However, the net present value for the replacement option is within $15 \%$ of the lowest cost option, therefore this Candidate Solution was considered strategic and moved forward in the process to Performance Effectiveness Evaluation.
- Rehabilitation or repair was determined to be the most effective approach for the candidate solution 70.3, and this location does not have other Needs. Therefore, it is assumed that these will be addressed by normal programming processes and these candidate solutions will be dropped from further consideration

Table 21: Bridge Life Cycle Cost Analysis Results

| Candidate Solution | Present Value at 3\% Discount Rate (\$) |  |  | Ratio of Present Value Compared to Lowest Present Value |  |  | Other Needs | Results |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Replace | Rehab | Repair | Replace | Rehab | Repair |  |  |

No LCCA conducted for any bridge candidate solutions on the US 6070191 Corridor.

Table 22: Pavement Life Cycle Cost Analysis Results

| Candidate Solution | Present Value at 3\% Discount Rate (\$) |  |  |  | Ratio of Present Value Compared to Lowest Present Value |  |  |  | Other <br> Needs | Results |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Concrete Reconstruction | Asphalt Reconstruction | Asphalt Medium Rehabilitation | Asphalt Light Rehabilitation | Concrete Reconstruction | Asphalt Reconstruction | Asphalt Medium Rehabilitation | Asphalt Light Rehabilitation |  |  |
| US191 Pavement Preservation South of Safford (CS 191.1, MP 88-104) | \$262,287,676 | \$240,087,944 | \$221,616,023 | \$209,767,952 | 1.25 | 1.14 | 1.06 | 1.00 | N | Asphalt reconstruction are within $15 \%$ of the lowest cost replacement is recommended |
| Bylas Area Pavement Preservation (CS 70.3, MP 293-298) | \$35,953,272 | \$32,910,227 | \$28,425,286 | \$32,545,584 | 1.26 | 1.16 | 1.00 | 1.14 | N | Reconstruction is not within $15 \%$ of lowest cost - Rehabilitation is recommended |

### 5.2 Performance Effectiveness Evaluation

The results of the Performance Effectiveness Evaluation are combined with the results of a Performance Area Risk Analysis to determine the Performance Effectiveness Score (PES). The objectives of the Performance Effectiveness Evaluation include:

- Measure the benefit to the performance system versus the cost of the solution
- Include risk factors to help differentiate between similar solutions
- Apply to each performance area that is affected by the candidate solution
- Account for emphasis areas identified for the corridor

The Performance Effectiveness Evaluation includes the following steps:

- Estimate the post-solution performance for each of the five performance areas (Pavement, Bridge, Mobility, Safety, and Freight)
- Use the post-solution performance scores to calculate a post-solution level of need for each of the five performance areas
- Compare the pre-solution level of need to the post-solution level of need to determine the reduction in level of need (potential solution benefit) for each of the five performance areas
- Calculate performance area risk weighting factors for each of the five performance areas
- Use the reduction in level of need (benefit) and risk weighting factors to calculate the PES


## Post-Solution Performance Estimation

For each performance area, a slightly different approach is used to estimate the post-solution performance. This process is based on the following assumptions:

- Pavement:
- The IRI rating would decrease (to 30 for replacement or 45 for rehabilitation)
- The Cracking rating would decrease (to 0 for replacement or rehabilitation)
- Bridge:
- The structural ratings would increase ( +1 for repair, +2 for rehabilitation, or increase to 8 for replacement)
- The Sufficiency Rating would increase (+10 for repair, +20 for rehabilitation, or increase to 98 for replacement)
- Mobility:
- Additional lanes would increase the capacity and therefore affect the Mobility Index and associated secondary measures
- Other improvements (ramp metering, parallel ramps, variable speed limits) would also increase the capacity (to a lesser extent than additional lanes) and therefore would affect the Mobility Index and associated secondary measures
- Changes in the Mobility Index (due to increased capacity) and Safety Index (due to crash reductions) would have a direct effect on the LOTTR secondary measure
- Changes in the Safety Index (due to crash reductions) would have a direct effect on the Closure Extent secondary measure
- Safety:
- Crash modification factors were developed that would be applied to estimate the reduction in crashes (for additional information see Appendix F)
- Freight:
- Changes in the Mobility Index (due to increased capacity) and Safety Index (due to crash reductions) would have a direct effect on the Freight Index and the TTTR secondary measure
- Changes in the Safety Index (due to crash reductions) would have a direct effect on the Closure Duration secondary measure
Performance Area Risk Analysis
The Performance Area Risk Analysis is intended to develop a numeric risk weighting factor for each of the five performance areas (Pavement, Bridge, Mobility, Safety, and Freight). This risk analysis addresses other considerations for each performance area that are not directly included in the performance system. A risk weighting factor is calculated for each candidate solution based on the specific characteristics at the solution location. For example, the Pavement Risk Factor is based on factors such as the elevation, daily traffic volumes, and amount of truck traffic. Additional information regarding the Performance Area Risk Factors is included in Appendix G.

Following the calculation of the reduction in level of need (benefit) and the Performance Area Risk Factors, these values are used to calculate the PES. In addition, the reduction in level of Need in each emphasis area is also included in the PES.

Net Present Value Factor
The benefit (reduction in need) is measured as a one-time benefit. However, different types of solutions will have varying service lives during which the benefits will be obtained. For example, a preservation solution would likely have a shorter stream of benefits over time when compared to a modernization or expansion solution. To address the varying lengths of benefit streams, each solution is classified as a 10 -year, 20-year, 30 -year, or 75 -year benefit stream, or the NPV factor ( $F_{\text {NPV }}$ ). A $3 \%$ discount rate is used to calculate $F_{\text {NPV }}$ for each classification of solution. The service lives and respective factors are described below:

- A 10-year service life is generally reflective of a preservation solution; this would include pavement and bridge preservation solutions which would likely have a 10 -year stream of benefits; for these solutions, a $F_{N P V}$ of 8.8 is used in the PES calculation
- A 20 -year service life is reflective of modernization solutions that generally do not include new infrastructure; these solutions would likely have a 20 -year stream of benefits; for these solutions, a $\mathrm{F}_{\mathrm{NPV}}$ of 15.3 is used in the PES calculation
- A 30-year service life is generally reflective of an expansion solution or a modernization solution that includes new infrastructure; these solutions would likely have a 30-year stream of benefits; for these solutions, a F ${ }_{N P V}$ of 20.2 is used in the PES calculation
- A 75-year service life is used for bridge replacement solutions; for these solutions, a FNPV of 30.6 is used in the PES calculation


## Vehicle-Miles Travelled Factor

Another factor in assessing benefits is the number of travelers who would benefit from the implementation of the candidate solution. This factor varies between candidate solutions depending on the length of the solution and the magnitude of daily traffic volumes. Multiplying the solution length by the daily traffic volume results in vehicle-miles travelled (VMT), which provides a measure of the amount of traffic exposure that would receive the benefit of the proposed solution. The VMT is converted to a VMT factor (known as $\mathrm{FVMT}_{\mathrm{V}}$ ), which is on a scale between 0 and 5 , using the equation below:

$$
F_{V M T}=5-\left(5 x e^{\text {VMT } x-0.0000139}\right)
$$

## Performance Effectiveness Score

## The PES is calculated using the following equation:

PES $=($ Sum of all Risk Factored Benefit Scores + Sum of all Risk Factored Emphasis Area Scores) / Cost) x $F_{V M T} \times F_{N P V}$

Where:
Risk Factored Benefit Score $=$ Reduction in Segment-Level Need (benefit) x Performance Area Risk Weighting Factor (calculated for each performance area)
Risk Factored Emphasis Area Score = Reduction in Corridor-Level Need x Performance Area Risk Factors x Emphasis Area Factor (calculated for each emphasis area)
Cost = estimate cost of candidate solution in millions of dollars (see Appendix H)
$F_{V M T}=$ Factor between 0 and 5 to account for VMT at location of candidate solution based on existing (2023) daily volume and length of solution
$F_{N P V}=$ Factor (ranging from 8.8 to 30.6 as previously described) to address anticipated longevity of service life (and duration of benefits) for each candidate solution
The resulting PES values are shown in Table 23. Additional information regarding the calculation of the PES is contained in Appendix I.

For candidate solutions with multiple options to address Mobility, Safety, or Freight needs, the PES should be compared to help identify the best performing option. If one option clearly performs better than the other options (e.g., more than twice the PES value and a difference in magnitude of at least 20 points), the other options should be eliminated from further consideration. If multiple options have similar PES values, or there are other factors not accounted for in the performance system that could significantly influence the ultimate selection of an option (e.g., potential environmental concerns, potential adverse economic impacts), those options should all be advanced to the prioritization process. On the US 60|US 70|US 191 corridor, the following candidate solutions have options:

- CS70|60.7 (A, B) - Globe Area Freight Improvements MP 243-255
- CS60.9 (A, B) - Miami Area Freight Improvements MP 227-243


## Table 23: Performance Effectiveness Scores

| Candidate Solution \# | Segment \# | Option | Candidate Solution Name | Milepost Location | Estimated Cost* (in millions) | Risk Factored Benefit Score |  |  |  |  | Risk Factored Emphasis Area Scores |  |  | Total Factored Benefit Score | Fvmt | Fnpv | Performance Effectiveness Score |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  | Pavement | Bridge | Mobility | Safety | Freight | Mobility | Safety | Freight |  |  |  |  |
| CS191.1 | 191-3 | B | US191 Pavement Preservation South of Safford - Option B (replace pavement) | 88-104 | \$200.3 | 1.85 | 0.00 | 0.00 | 0.03 | 0.00 | 0.00 | 0.00 | 0.00 | 1.88 | 2.21 | 20.2 | 0.4 |
| CS70.2 | 70-6 | - | East Safford Safety Improvements | 336.5-339 | \$0.1 | 0.00 | 0.00 | 0.00 | 0.05 | 0.00 | 0.00 | 0.04 | 0.00 | 0.08 | 0.17 | 15.3 | 2.1 |
| CS70.4 | 70-10 | - | Bylas to Peridot Safety Improvements | 274-293 | \$15.1 | 0.00 | 0.00 | 0.02 | 3.74 | 0.00 | 0.00 | 0.75 | 0.00 | 4.50 | 0.13 | 20.2 | 0.8 |
| CS70.5 | 70-12 | - | East of Globe Safety Improvements | 255-270 | \$31.1 | 0.00 | 0.00 | 0.31 | 3.73 | 0.00 | 0.03 | 0.55 | 0.00 | 4.61 | 3.50 | 20.20 | 10.5 |
| CS70\|60.6 | 70\|60-13 | - | Globe Area Safety Improvements | 243-255 | \$22.6 | 0.00 | 0.00 | 0.23 | 10.03 | 2.26 | 0.01 | 1.19 | 1.13 | 14.85 | 1.13 | 20.20 | 15.0 |
| CS70\|60.7 | 70\|60-13 | A | Globe Area Freight Improvements Option A (reconstruct Pinal SPRR UP) | 243-255 | \$8.2 | 0.00 | 0.00 | 0.00 | 0.00 | 1.90 | 0.00 | 0.00 | 0.01 | 1.91 | 0.37 | 20.20 | 1.7 |
|  |  | B | Globe Area Freight Improvements Option B (reprofile mainline) | 243-255 | \$2.1 | 0.00 | 0.00 | 0.05 | 1.98 | 1.52 | 0.00 | 0.00 | 0.35 | 3.90 | 0.37 | 20.20 | 13.9 |
| CS60.8 | 60-14 | - | Superior East Area Safety Improvements | 227-243 | \$17.0 | 0.00 | 0.00 | 0.51 | 13.77 | 6.49 | 0.00 | 1.82 | 1.04 | 23.63 | 4.47 | 8.80 | 54.6 |
| CS60.9A | 60-14 | A | Superior East Area Freight Improvements - Option A (reconstruct Pinal SPRR UP) | 228.47 | \$33.3 | 0.00 | 0.00 | 0.00 | 0.00 | 3.79 | 0.00 | 0.00 | 0.02 | 3.82 | 0.34 | 20.20 | 0.8 |
| CS60.9B | 60-14 | B | Superior East Area Freight Improvements - Option A (reprofile mainline) | 228.47 | \$1.9 | 0.00 | 0.00 | 0.28 | 1.68 | 4.91 | 0.00 | 0.00 | 0.79 | 7.66 | 0.34 | 20.20 | 27.1 |
| CS60.10 | 60-17 | - | Superior Area Safety Improvements | 212-223 | \$0.4 | 0.00 | 0.00 | 0.03 | 0.87 | 0.74 | 0.00 | 0.08 | 0.16 | 1.88 | 3.29 | 8.80 | 121.4 |
| CS60.11 | 60-18 | - | US-60 SW of Gold Canyon Safety Improvements | 206-208 | \$3.9 | 0.00 | 0.00 | 0.02 | 0.06 | 1.13 | 0.00 | 0.03 | 0.08 | 1.32 | 1.61 | 15.30 | 8.3 |
| CS60.12 | 60-19 | - | Gold Canyon Area Mobility and Safety Improvements | 199-205 | \$44.0 | 0.00 | 0.00 | 19.52 | 0.64 | 2.16 | 0.08 | 0.13 | 0.15 | 22.67 | 4.16 | 20.20 | 43.3 |

Table 23: Performance Effectiveness Scores (continued)

| Candidate Solution \# | $\underset{\#}{\text { Segment }}$ | Option | Candidate Solution Name | Milepost Location | Estimated Cost* (in millions) | Risk Factored Benefit Score |  |  |  |  | Risk Factored Emphasis Area Scores |  |  | Total <br> Factored Benefit Score | Fvmt | Fnpv | Performance Effectiveness Score |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  | Pavement | Bridge | Mobility | Safety | Freight | Mobility | Safety | Freight |  |  |  |  |
| CS60.13 | 60-20 | - | Apache Junction Area Mobility Improvements | 194.3-199 | \$24.7 | 0.00 | 0.00 | 15.22 | 0.13 | 1.13 | 0.04 | 0.00 | 0.22 | 16.74 | 3.97 | 20.2 | 54.5 |
| CS60.14 | 60-20 | - | Apache Junction Area Safety Improvements | 194.3-199 | \$0.3 | 0.00 | 0.00 | 0.03 | 0.81 | 0.31 | 0.00 | 0.05 | 0.02 | 1.21 | 4.19 | 8.8 | 158.2 |

### 5.3 Solution Risk Analysis

Following the calculation of the PES, an additional step is taken to develop the prioritized list of solutions. A solution risk probability and consequence analysis is conducted to develop a solution-level risk weighting factor. This risk analysis is a numeric scoring system to help address the risk of not implementing a solution based on the likelihood and severity of performance failure. Figure $\mathbf{2 5}$ shows the risk matrix used to develop the risk weighting factors.

Figure 25: Risk Matrix

|  |  | Severity/Consequence |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Insignificant | Minor | Significant | Major | Catastrophic |
|  | Very Rare | Low | Low | Low | Moderate | Major |
|  | Rare | Low | Low | Moderate | Major | Major |
|  | Seldom | Low | Moderate | Moderate | Major | Severe |
|  | Common | Moderate | Moderate | Major | Severe | Severe |
|  | Frequent | Moderate | Major | Severe | Severe | Severe |

Using the risk matrix in Figure 25, numeric values were assigned to each category of frequency and severity. The higher the risk, the higher the numeric factor that was assigned. The risk weight for each area of the matrix was calculated by multiplying the severity factor times the frequency factor. These numeric factors are shown in Figure 26.

Figure 26: Numeric Risk Matrix

|  |  |  | Severity/Consequence |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Insignificant | Minor | Significant | Major | Catastrophic |
|  |  | Weight | 1.00 | 1.10 | 1.20 | 1.30 | 1.40 |
|  | Very Rare | 1.00 | 1.00 | 1.10 | 1.20 | 1.30 | 1.40 |
|  | Rare | 1.10 | 1.10 | 1.21 | 1.32 | 1.43 | 1.54 |
|  | Seldom | 1.20 | 1.20 | 1.32 | 1.44 | 1.56 | 1.68 |
|  | Common | 1.30 | 1.30 | 1.43 | 1.56 | 1.69 | 1.82 |
|  | Frequent | 1.40 | 1.40 | 1.54 | 1.68 | 1.82 | 1.96 |

Using the values in Figure 26, risk weighting factors were calculated for each of the four risk categories (low, moderate, major, and severe). These values are simply the average of the values in Figure 26 that fall within each category. The resulting average risk weighting factors are:

| Low | $\frac{\text { Moderate }}{1.36}$ | $\frac{\text { Major }}{1.51}$ | Severe |
| :---: | :---: | :---: | :---: |
| 1.14 | 1.78 |  |  |

The risk weighting factors listed above are assigned to the five performance areas as follows:

- Safety $=1.78$
- The Safety performance area quantifies the likelihood of fatal or incapacitating injury crashes; therefore, it is assigned the Severe (1.78) risk weighting factor
- Bridge = 1.51
- The Bridge performance area focuses on the structural adequacy of bridges; a bridge failure may result in crashes or traffic being detoured for long periods of time resulting in significant travel time increases; therefore, it is assigned the Major (1.51) risk weighting factor
- Mobility and Freight $=1.36$
- The Mobility and Freight performance areas focus on capacity and congestion; failure in either of these performance areas would result in increased travel times but would not have significant effect on safety (crashes) that would not already be addressed in the Safety performance area; therefore, they are assigned the Moderate (1.36) risk weighing factor
- Pavement $=1.14$
- The Pavement performance area focuses on the ride quality of the pavement failure in this performance area would likely be a spot location that would not dramatically affect drivers beyond what is already captured in the Safety performance area; therefore, it is assigned the Low (1.14) risk weighting factor
The benefit in each performance area is calculated for each candidate solution as part of the Performance Effectiveness Evaluation. Using this information on benefits and the risk factors listed above, a weighted (based on benefit) solution-level numeric risk factor is calculated for each candidate solution. For example, a solution that has $50 \%$ of its benefit in Safety and 50\% of its benefit in Mobility has a weighted risk factor of $1.57(0.50 \times 1.36+0.50 \times 1.78=1.57)$.


### 5.4 Candidate Solution Prioritization

The PES, weighted risk factor, and segment average need score are combined to create a prioritization score as follows:

Prioritization Score $=$ PES $\times$ Weighted Risk Factor $\times$ Segment Average Need Score

## Where:

PES = Performance Effectiveness Score as shown in Table 23
Weighted Risk Factor = Weighted factor to address risk of not implementing a solution based on the likelihood and severity of the performance failure
Segment Average Need Score = Segment average need score as shown in Table 17
Table 24 shows the prioritization scores for the candidate solutions subjected to the solution evaluation and prioritization process. Solutions that address multiple performance areas tend to score higher in this process. A prioritized list of candidate solutions is provided in the subsequent section. See Appendix J for additional information on the prioritization process.

Table 24: Prioritization Scores

| Candidate Solution \# | Segment \# | Option | Candidate Solution Name | Milepost Location | Estimated Cost (in millions) | Performance Effectiveness Score | Weighted Risk Factor | Segment Average Need Score | Prioritization Score | Percentage by which Solution Reduces Performance Area Segment Needs |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  | Pavement | Bridge | Mobility | Safety | Freight |
| CS191.1 | 191-3 | B | US191 Pavement Preservation South of Safford - Option B (replace pavement) | 88-104 | \$200.3 | 0.4 | 1.15 | 0.77 | 0 | 67\% | - | 0\% | 30\% | 0\% |
| CS70.2 | 70-6 | - | East Safford Safety Improvements | 336.5-339 | \$0.1 | 2.1 | 1.78 | 0.92 | 3 | - | - | 0\% | 4\% | 0\% |
| CS70.4 | 70-10 | - | Bylas to Peridot Safety Improvements | 274-293 | \$15.1 | 0.8 | 1.78 | 1.38 | 2 | - | - | 0\% | 34\% | 0\% |
| CS70.5 | 70-12 | - | East of Globe Safety Improvements | 255-270 | \$31.1 | 10.5 | 1.75 | 1.23 | 23 | - | - | 6\% | 97\% | 0\% |
| CS70\|60.6 | 70\|60-13 | - | Globe Area Safety Improvements | 243-255 | \$22.6 | 15.0 | 1.68 | 1.77 | 44 | - | - | 4\% | 30\% | 46\% |
| CS70\|60.7 | 70\|60-13 | A | Globe Area Freight Improvements - Option A (reconstruct Pinal SPRR UP) | 243-255 | \$8.2 | 1.7 | 1.36 | 1.77 | 4 | - | - | 0\% | 0\% | 38\% |
|  |  | B | Globe Area Freight Improvements - Option B (reprofile mainline) | 243-255 | \$2.1 | 13.9 | 1.57 | 1.77 | 39 | - | - | 1\% | 6\% | 31\% |
| CS60.8 | 60-14 | - | Superior East Area Safety Improvements | 227-243 | \$17.0 | 54.6 | 1.64 | 2.54 | 227 | - | - | 0\% | 56\% | 18\% |
| CS60.9A | 60-14 | A | Superior East Area Freight Improvements - Option A (reconstruct Pinal SPRR UP) | 227-243 | \$33.3 | 0.8 | 1.36 | 2.54 | 3 | - | - | 0\% | 0\% | 10\% |
| CS60.9B | 60-14 | B | Superior East Area Freight Improvements - Option A (reprofile mainline) | 227-243 | \$1.9 | 27.1 | 1.45 | 2.54 | 100 | - | - | 0\% | 7\% | 13\% |
| CS60.10 | 60-17 | - | Superior Area Safety Improvements | 212-223 | \$0.4 | 121.4 | 1.57 | 1.00 | 191 | - | - | 1\% | 40\% | 36\% |
| CS60.11 | 60-18 | - | US-60 SW of Gold Canyon Safety Improvements | 206-208 | \$3.9 | 8.3 | 1.39 | 1.15 | 13 | - | - | 1\% | 12\% | 23\% |
| CS60.12 | 60-19 | - | Gold Canyon Area Mobility and Safety Improvements | 199-205 | \$44.0 | 43.3 | 1.37 | 1.69 | 101 | - | - | 72\% | 21\% | 42\% |
| CS60.13 | 60-20 | - | Apache Junction Area Mobility Improvements | 194.3-199 | \$24.7 | 54.5 | 1.36 | 1.38 | 102 | - | - | 64\% | 3\% | 80\% |
| CS60.14 | 60-20 | - | Apache Junction Area Safety Improvements | 194.3-199 | \$0.3 | 158.2 | 1.66 | 1.38 | 362 | - | - | 0\% | 19\% | 22\% |

### 6.0 SUMMARY OF CORRIDOR RECOMMENDATIONS

### 6.1 Prioritized Candidate Solution Recommendations

Table 25 and Figure 27 show the prioritized candidate solutions recommended for the US 60|US 70|US 191 corridor. The highest prioritization score indicates the candidate solution that is recommended as the highest priority. Implementation of these solutions is anticipated to improve performance of the corridor. The following observations were noted about the prioritized solutions:

- Most of the anticipated improvements in performance are in the Mobility, Safety, and Freight performance areas
- The highest ranking solutions tended to have overlapping benefits in the Mobility, Safety, and Freight performance areas
- The highest priority solutions address needs in the US 60 Apache Junction area (MP 194.3199 to MP 243), Superior Area (MP 212-223) and Miami Area (MP 228-247)


### 6.2 Other Corridor Recommendations

As part of the investigation of strategic investment areas and candidate solutions, other corridor recommendations can also be identified. These recommendations could include modifications to the existing Statewide Construction Program, areas for further study, or other corridor-specific recommendations that are not related to construction or policy. The list below identifies other corridor recommendations for the US 60|US 70 US 191 corridor:

- Road Safety Assessments are recommended in Peridot, Cutter and Globe to identify safety improvements, specifically pedestrian circulation and access needs in Peridot.
- Access Control Studies in Peridot (MP 270 - 274) and Globe-Miami (MP 243 - 255) are recommended to identify potential for access consolidation, signage, etc to reduce friction and improve safety
- Recommend Superior to Globe DCR/Feasibility Study
- Recommend San Carlos Area (MP 268 - 292) Superelevation Study


### 6.3 Policy and Initiative Recommendations

In addition to location-specific needs, general corridor and system-wide needs have also been identified through the CPS process. While these needs are more overarching and cannot be individually evaluated through this process, it is important to document them. A list of recommended policies and initiatives was developed for consideration when programming future projects not only on US 60|US 70|US 191, but across the entire state highway system where the conditions are applicable. The following list, which is in no particular order of priority, was derived from the Round 1 , Round 2, and Round 3 CPS

- Install Intelligent Transportation System (ITS) conduit with all new infrastructure projects
- Prepare strategic plans for Closed Circuit Television (CCTV) camera and Road Weather Information System (RWIS) locations statewide
- Leverage power and communication at existing weigh-in-motion (WIM), dynamic message signs (DMS), and call box locations to expand ITS applications across the state
- Consider solar power for lighting and ITS where applicable
- Investigate ice formation prediction technology where applicable
- Conduct highway safety manual evaluation for all future programmed projects
- Develop infrastructure maintenance and preservation plans (including schedule and funding) for all pavement and bridge infrastructure replacement or expansion projects
- Develop standardized bridge maintenance procedures so districts can do routine maintenance work
- Review historical ratings and level of previous investment during scoping of pavement and bridge projects. In pavement locations that warrant further investigation, conduct subsurface investigations during project scoping to determine if full replacement is warranted
- For pavement rehabilitation projects, enhance the amount/level of geotechnical investigations to address issues specific to the varying conditions along the project
- Expand programmed and future pavement projects as necessary to include shoulders
- Expand median cable barrier guidelines to account for safety performance
- Install CCTV cameras with all DMS
- In locations with limited communications, use CCTV cameras to provide still images rather than streaming video
- Develop statewide program for pavement replacement
- Install additional continuous permanent count stations along strategic corridors to enhance traffic count data
- When reconstruction or rehabilitation activities will affect existing bridge vertical clearance, the dimension of the new bridge vertical clearance should be a minimum of 16.25 feet where feasible
- All new or reconstructed roadway/shoulder edges adjacent to an unpaved surface should be constructed with a Safety Edge
- Collision data on tribal lands may be incomplete or inconsistent; additional coordination for data on tribal lands is required to ensure adequate reflection of safety issues
- Expand data collection devices statewide to measure freight delay
- Evaluate and accommodate potential changes in freight and goods movement trends that may result from improvements and expansions to the state roadway network

Table 25: Prioritized Recommended Solutions

| Rank | Candidate Solution \# | Option | Solution Name and Location | Description / Scope | Estimated Cost (in millions) | Investment Category (Preservation [P], Modernization [M], Expansion [E]) | Prioritization Score |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 60.14 | - | Apache Junction Area Safety Improvements | Install inside and edge line rumble strips through entire segment Consider installing speed feedback sign MP 195 | \$0.3 | M | 362 |
| 2 | 60.8 | - | Superior Area East Safety Improvements | Consider installing speed feedback signs at MP 229.9, MP 236, MP 241 Install centerline rumble strips at MP 229-231 <br> Install high visibility striping and delineators MP 228-228.3 and MP 241-242 Install edge line rumble strips EB MP 228.17-228.3, MP 229.2-229.26, and MP 247247.26 | \$17.0 | M | 227 |
| 3 | 60.10 | - | Superior Area Safety Improvements | Install lighting at N Queen Valley Road and US 60 intersection Consider installing speed feedback sign MP 212.5 Install chevrons or curve warning sign at MP 219.33 | \$0.4 | M | 191 |
| 4 | 60.13 | - | Apache Junction Area Mobility Improvements | Add through lane in NB/WB direction | \$24.7 | M | 102 |
| 5 | 60.12 | - | Gold Canyon Area Mobility and Safety Improvements | Add SB/EB through lane MP 199.12 to 206 <br> Consider installing speed feedback sign at MP 201 Widen shoulders MP 199.12 to 205 Install lighting MP 201-202 | \$44.0 | E | 101 |
| 6 | 60.9 | B | Superior Area East Freight Improvements | Reprofile mainline to increase vertical clearance | \$1.9 | E | 100 |
| 7 | 70\|60.6 | - | Globe Area Safety Improvements | Consider installing speed feedback signs (2 EB and 2 WB between MP 246-250) High visibility striping <br> Install signal ahead warning signs with beacons in advance of SR 188 intersection Construct passing lane in each direction from MP 243-243.25 and MP 253.6-255 | \$22.6 | M | 44 |
| 8 | 70\|60.7 | B | Globe Area Freight Improvements | Reprofile mainline to increase vertical clearance | \$2.1 | E | 39 |
| 9 | 70.5 | - | East of Globe Safety Improvements | Widen shoulders MP 255-270 <br> Install centerline rumble strips MP 255-270 <br> Install improved lighting from milepost 269-270 <br> Construct passing lane in each direction (MP 255-256) <br> Improve existing pedestrian and speed warning signs to include flashing beacons and speed feedback signs (MP 269.25) | \$31.1 | M | 23 |
| 10 | 60.11 | - | US-60 SW of Gold Canyon Safety Improvements | Install lighting MP 205-207 <br> Widen inside shoulder 208.3-212 <br> Consider installing speed feedback sign | \$3.9 | M | 13 |

Table 25: Prioritized Recommended Solutions (Continued)

| Rank | Candidate Solution \# | Option | Solution Name and Location | Description / Scope | Estimated Cost (in millions) | Investment Category (Preservation [P], Modernization [M], Expansion [E]) | Prioritization Score |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 11 | 70\|60.7 | A | Globe Area Freight Improvements | Reconstruct Pinal SPRR UP to increase vertical clearance | \$8.2 | E | 4 |
| 12 | 70.2 | - | East Safford Safety Improvements | Provide flashing traffic signal warning signs at Milepost 337.82 and Milepost 338.03. Consider installing feedback signs in both directions at 20th Avenue | \$0.1 | M | 3 |
| 13 | 60.9 | A | Superior Area East Freight Improvements | Reconstruct Queen Creek Tunnel to increase vertical clearance | \$33.3 | E | 3 |
| 14 | 70.4 | - | Bylas to Peridot Safety Improvements | Widen shoulders Milepost 274-278 <br> Install centerline rumble strips MP 275.5-276.5,MP 279.5-287.5 <br> Install shoulder rumble strips MP 275.5-276.5,MP 279.5-287.5 <br> Install high visibility striping and delineators from milepost 274-278 <br> Improve existing pedestrian / speed warning signs to also include flashing beacons and speed feedback signs (MP 292,MP 280, MP 278.5) <br> Construct passing lanes (WB MP 288.2-289.6) <br> Formalize pullouts (signage, ROW for pullouts) (WB MP 274.5, EB MP 279, EB MP 289, WB 292) | \$15.1 | M | 2 |
| 15 | 191.1 | B | US191 Pavement Preservation South of Safford | Replace pavement | \$200.3 | M | 0 |



### 6.4 Next Steps

The candidate solutions recommended in this study are not intended to be a substitute or replacement for traditional ADOT project development processes where various ADOT technical groups and districts develop candidate projects for consideration in the performance-based programming in the P2P process. Rather, these candidate solutions are intended to complement ADOT's traditional project development processes through a performance-based process to address needs in one or more of the five performance areas of Pavement, Bridge, Mobility, Safety, and Freight. Candidate solutions developed for the US 60|US 70|US 191 corridor will be considered along with other candidate projects in the ADOT statewide programming process.

It is important to note that the candidate solutions are intended to represent strategic solutions to address existing performance needs related to the Pavement, Bridge, Mobility, Safety, and Freight performance areas. Therefore, the strategic solutions are not intended to preclude recommendations related to the ultimate vision for the corridor that may have been defined in the context of prior planning studies and/or design concept reports. Recommendations from such studies are still relevant to addressing the ultimate corridor objectives.

These results will be incorporated into a summary document comparing all corridors that is expected to provide a performance-based review of statewide needs and candidate solutions.


## Aロロт

## Appendix A: Corridor Performance Maps

This appendix contains maps of each primary and secondary measure associated with the five performance areas for the SR 260/US 60 corridor. The following are the areas and maps included: Pavement Performance Area:

- Pavement Index and Hot Spots
- Pavement Serviceability (directional)
- Percentage of Pavement Area Failure


## Bridge Performance Area:

- Bridge Index and Hot Spots
- Bridge Sufficiency
- Lowest Bridge Rating

Mobility Performance Area:

- Mobility Index
- Future Daily V/C Ratio
- Existing Peak Hour V/C Ratio (directional)
- Closure Frequency (directional)
- Level of Travel Time Reliability (directional)
- Multimodal Opportunities
- Percentage of Bicycle Accommodation


## Safety Performance Area:

- Safety Index and Hot Spots
- Safety Index and Hot Spots (directional)
- Relative Frequency of Fatal + Suspected Serious Injury Crashes Involving Intersection Crashes Compared to the Statewide Average for Similar Segments (insufficient data - not included)
- Relative Frequency of Fatal + Suspected Serious Injury Crashes Involving Lane Departures Compared to the Statewide Average for Similar Segments
- Relative Frequency of Fatal + Suspected Serious Injury Crashes Involving Pedestrians Compared to the Statewide Average for Similar Segments (insufficient data - not included)
- Relative Frequency of Fatal + Suspected Serious Injury Crashes Involving Trucks Compared to the Statewide Average for Similar Segments (insufficient data - not included)
- Relative Frequency of Fatal + Suspected Serious Injury Crashes Involving Bicycles Compared to the Statewide Average for Similar Segments (insufficient data - not included)

Freight Performance Area:

- Freight Index and Hot Spots
- Truck Travel Time Reliability (directional)
- Closure Duration (directional)
- Bridge Vertical Clearance


























## Aロロт

## Appendix B: Performance Area Detailed Calculation Methodologies

## Pavement Performance Area Calculation Methodologies

This section summarizes the approach for developing the primary and secondary performance measures in the Pavement performance area as shown in the following graphic:


This performance area is used to evaluate mainline pavement condition. Pavement condition data for ramps, frontage roads, crossroads, etc. was not included in the evaluation.

## Primary Pavement Index

The Pavement Index is calculated based on the use of three pavement condition ratings from the ADOT Pavement Database. The three ratings are the International Roughness Index (IRI), the Cracking rating, and the Rutting rating. The calculation of the Pavement Index uses a combination of these two ratings.
The IRI is a measurement of the pavement roughness based on field-measured longitudinal roadway profiles. To facilitate the calculation of the index, the IRI rating was converted to a Pavement Serviceability Rating (PSR) using the following equation:

$$
P S R=5 * e^{-0.0038 * I R I}
$$

The Cracking Rating is a measurement of the amount of surface cracking based on a field-measured area of 1,000 square feet that serves as a sample for each mile. The Rutting rating is a measurement of the depth of pavement rutting based on field measurements. To facilitate the calculation of the
index, the Cracking Rating and Rutting Rating were combined and converted to a Pavement Distress Index (PDI) using the following equation:

$$
P D I=5-\left[\left(0.345 * C^{0.66}\right)+\left(0.01428 *\left(\frac{R}{2} * 100\right)^{1.32}\right)-\left(0.0823 * C^{0.18} *\left(\frac{R}{2} * 100\right)^{0.50}\right)\right]
$$

Both the PSR and PDI use a 0 to 5 scale with 0 representing the lowest performance and 5 representing the highest performance. The performance thresholds for interstates and noninterstates shown in the tables below were used for the PSR and PDI.

| Performance Level for Interstates | IRI (PSR) | Cracking \& Rutting (PDI) |
| :---: | :---: | :---: |
| Good | $<75(>3.75)$ | Cracking $<5.75$ Rutting $<0.35$ |
| Fair | $75-102(3.40-3.75)$ | Cracking $5.75-12$ <br> Rutting $0.35-0.55$ |
| Poor | $>102(<3.40)$ | Cracking $>12$ <br> Rutting $>0.55$ |


| Performance Level for Non-Interstates | IRI (PSR) | Cracking \& Rutting (PDI) |
| :---: | :---: | :---: |
| Good | $<94(>3.5)$ | Cracking $<5.75$ <br> Rutting $<0.35$ |
| Fair | $94-142(2.90-3.5)$ | Cracking $5.75-12$ <br> Rutting $0.35-0.55$ |
| Poor | $>142(<2.90)$ | Cracking $>12$ <br> Rutting $>0.55$ |

The PSR and PDI are calculated for each 1-mile section of roadway. If PSR or PDI falls into a poor rating ( $<3.4$ for PSR for interstates, for example) for a 1-mile section, then the score for that 1-mile section is entirely ( $100 \%$ ) based on the lower score (either PSR or PDI). If neither PSR or PDI fall into a poor rating for a 1-mile section, then the score for that 1-mile section is based on a combination of the lower rating ( $70 \%$ weight) and the higher rating ( $30 \%$ weight). The result is a score between 0 and 5 for each direction of travel of each mile of roadway based on a combination of both the PSR and the PDI.
The project corridor has been divided into segments. The Pavement Index for each segment is a weighted average of the directional ratings based on the number of travel lanes. Therefore, the condition of a section with more travel lanes will have a greater influence on the resulting segment Pavement Index than a section with fewer travel lanes.

## Secondary Pavement Measures

Three secondary measures are evaluated:

- Directional Pavement Serviceability
- Pavement Failure
- Pavement Hot Spots

Directional Pavement Serviceability: Similar to the Pavement Index, the Directional Pavement Serviceability is calculated as a weighted average (based on number of lanes) for each segment. However, this rating only utilizes the PSR and is calculated separately for each direction of travel. The PSR uses a 0 to 5 scale with 0 representing the lowest performance and 5 representing the highest performance.

Pavement Failure: The percentage of pavement area rated above the failure thresholds for IRI, Cracking, or Rutting is calculated for each segment. In addition, the Standard score (z-score) is calculated for each segment.
The Standard score (z-score) is the number of standard deviations above or below the mean. Therefore, a Standard score between -0.5 and +0.5 is "average", less than -0.5 is lower (better) than average, and higher than +0.5 is above (worse) than average.
Pavement Hot Spots: The Pavement Index map identifies locations that have an IRI rating, Cracking rating, or Rutting rating that fall above the failure threshold as identified by ADOT Pavement Group. For interstates, an IRI rating above 105, a Cracking rating above 10, or a Rutting rating above 0.4 will be used as the thresholds which are slightly different than the ratings shown previously. For non-interstates, an IRI rating above 142, a Cracking rating above 10, or a Rutting rating above 0.4 will be used as the thresholds.

Scoring

| Performance <br> Level | Pavement Index |  |
| :---: | :---: | :---: |
|  | Interstates | Non-Interstates |
| Good | $>3.75$ | $>3.6$ |
| Fair | $3.0-3.75$ | $2.8-3.6$ |
| Poor | $<3.0$ | $<2.8$ |
| Performance <br> Level | Directional Pavement Serviceability |  |
|  | Interstates | Non-Interstates |
|  | $>3.75$ | $>3.5$ |
| Fair | $3.4-3.75$ | $2.9-3.5$ |
| Poor | $<3.4$ | $<2.9$ |


| Performance <br> Level | \% Pavement Failure |
| :---: | :---: |
| Good | $<5 \%$ |
| Fair | $5 \%-20 \%$ |
| Poor | $>20 \%$ |

## Bridge Performance Area Calculation Methodologies

This section summarizes the approach for developing the primary and secondary performance measures in the Bridge performance area as shown in the following graphic:


This performance area is used to evaluate mainline bridges. Bridges on ramps (that do not cross the mainline), frontage roads, etc. should not be included in the evaluation. Basically, any bridge that carries mainline traffic or carries traffic over the mainline should be included and bridges that do not carry mainline traffic, run parallel to the mainline (frontage roads), or do not cross the mainline should not be included.

## Primary Bridge Index

The Bridge Index is calculated based on the use of four bridge condition ratings from the ADOT Bridge Database, also known as the Arizona Bridge Information and Storage System (ABISS). The four ratings are the Deck Rating, Substructure Rating, Superstructure Rating, and Structural Evaluation Rating. The calculation of the Bridge Index uses the lowest of these four ratings.

Each of the four condition ratings use a 0 to 9 scale with 0 representing the lowest performance and 9 representing the highest performance.
The project corridor has been divided into segments and the bridges are grouped together according to the segment definitions. In order to report the Bridge Index for each corridor segment, the Bridge Index for each segment is a weighted average based on the deck area for each bridge. Therefore,

## Scoring:

| Performance Level | Bridge Index |
| :---: | :---: |
| Good | $>6.5$ |
| Fair | $5.0-6.5$ |
| Poor | $<5.0$ |


| Performance Level | Sufficiency Rating |
| :---: | :---: |
| Good | $>80$ |
| Fair | $50-80$ |
| Poor | $<50$ |


| Performance Level | Bridge Rating |
| :---: | :---: |
| Good | $>6$ |
| Fair | $5-6$ |
| Poor | $<5$ |

## Mobility Performance Area Calculation Methodologies

This section summarizes the approach for developing the primary and secondary performance measures in the Mobility performance area as shown in the following graphic:


## Primary Mobility Index

The primary Mobility Index is an average of the existing daily volume-to-capacity (V/C) ratio and the future daily V/C ratio for each segment of the corridor.
Existing Daily V/C: The existing daily V/C ratio for each segment is calculated by dividing the existing Annual Average Daily Traffic (AADT) volume for each segment by the total Level of Service (LOS) E capacity volume for that segment
The capacity is calculated using the HERS Procedures for Estimating Highway Capacity ${ }^{1}$. The HERS procedure incorporates HCM 2010 methodologies. The methodology includes capacity estimation procedures for multiple facility types including freeways, rural two-lane highways, multilane highways, and signalized and non-signalized urban sections.

The segment capacity is defined as a function of the number of mainline lanes, shoulder width, interrupted or uninterrupted flow facilities, terrain type, percent of truck traffic, and the designated urban or rural environment.

The AADT for each segment is calculated by applying a weighted average across the length of the segment based on the individual 24 -hour volumes and distances associated with each HPMS count station within each segment.
The following example equation is used to determine the weighted average of a segment with two HPMS count locations within the corridor
((HPMS 1 Distance $x$ HPMS 1 Volume) $+($ HPMS 2 Distance $\times$ HPMS 2 Volume))/Total Segment Length
For specific details regarding the HERS methodology used, refer to the Procedures for Estimating Highway Capacity, draft Technical Memorandum.
Future Daily V/C: The future daily V/C ratio for each segment is calculated by dividing the future AADT volume for each segment by the existing LOS E capacity. The capacity volume used in this calculation is the same as is utilized in the existing daily $\mathrm{V} / \mathrm{C}$ equation.
The future AADT daily volumes are generated by applying an average annual compound growth rate (ACGR) to each existing AADT segment volume. The following equation is used to apply the average annual compound growth rate:

Future $A A D T=$ Existing $A A D T x\left((1+A C G R)^{\wedge}(F u t u r e ~ Y e a r-E x i s t i n g ~ Y e a r)\right) ~$
The ACGR for each segment is defined by comparing the total volumes in the existing Arizona Travel Demand Model (AZTDM2) to the future AZTDM2 traffic volumes at each existing HPMS count station location throughout the corridor. Each existing and future segment volume is defined using the same weighted average equation described in the Existing Daily V/C section above and then summing the directional volumes for each location. The following equation is used to determine the ACGR for each segment:

$$
\text { ACGR }=\left(\left((\text { Future Volume/Existing Volume })^{\wedge}(1 /(\text { Future Year-Existing Year) }))\right)-1\right.
$$

## Secondary Mobility Measures

Four secondary measures are evaluated

- Future Congestion
- Peak Congestion
- Travel Time Reliability

[^3]- Closure Extent
- Directional Level of Travel Time Reliability
- Multimodal Opportunities
- \% Bicycle Accommodation
- \% Non-Single Occupancy Vehicle (SOV) Trips

Future Congestion: The future daily V/C ratios for each segment in the corridor that are calculated and used in the Mobility Index as part of the overall average between Existing Daily V/C and Future Daily V/C are applied independently as a secondary measure. The methods to calculate the Future Daily V/C can be referenced in the Mobility Index section.
Peak Congestion: Peak Congestion has been defined as the peak hour V/C ratio in both directions of the corridor. The peak hour V/C ratio is calculated using the HERS method as described previously. The peak hour volume utilizes the directional AADT for each segment, which is calculated by applying a weighted average across the length of the segment based on the individual directional 24 -hour volumes and distances associated with each HPMS count station within each segment. The segment capacity is defined based on the characteristics of each segment including number of lanes, terrain type, and environment, similar to the 24 -hour volumes using the HERS method.

Travel Time Reliability: Travel time reliability is a secondary measure that includes two indicators. The two indicators are the number of times a piece of a corridor is closed for any specific reason, and the directional Level of Travel Time Reliability (LOTTR) metric.
Closure Extent: The number of times a roadway is closed is documented through the HCRS dataset. Closure Extent is defined as the average number of times a particular milepost of the corridor is closed per year per mile in a specific direction of travel. The weighted average of each occurrence takes into account the distance over which a specific occurrence spans

Thresholds that determine levels of good, fair, and poor are based on the average number of closures per mile per year within each of the identified statewide significant corridors by ADOT. The thresholds shown at the end of this section represent statewide averages across those corridors.

Directional Level of Travel Time Reliability: In terms of overall mobility, the LOTTR is the relationship of the $80^{\text {th }}$ percentile travel time to average ( $50^{\text {th }}$ percentile) travel time for a given corridor segment in a specific direction
Using INRIX data provided by ADOT, four time periods for each data point were collected throughout the day (AM peak, mid-day, PM peak, and off-peak). The highest value of the four time periods calculation is defined as the LOTTR for that data point. The weighted average LOTTR is calculated within each segment based on the number of data points collected and the length associated with the TMC location. The value of the weighted average LOTTR across each entry is used as the LOTTR for each respective segment within the corridor.

Multimodal Opportunities: Three multimodal opportunity indicators reflect the characteristics of the corridor that promote alternate modes to a single occupancy vehicle (SOV) for trips along the corridor. The three indicators include the percent bicycle accommodation, non-SOV trips, and transit dependency along the corridor.
Percent Bicycle Accommodation: For this secondary performance evaluation, outside shoulder widths are evaluated considering the roadway's context and conditions. This requires use of the roadway data that includes right shoulder widths, shoulder surface types, and speed limits, all of which are available in the following ADOT geographic information system (GIS) data sets:

- Right Shoulder Widths
- Left Shoulder Widths (for undivided roadways)
- Shoulder Surface Type (Both Left/Right)
- Speed Limit

Additionally, each segment's average AADT, estimated earlier in the Mobility performance area methodology, is used for the criteria to determine if the existing shoulder width meets the effective width.

The criteria for screening if a shoulder segment meets the recommended width criteria are as followed:
(1) If AADT <= 1500 OR Speed Limit <= 25 miles per hour (mph):

The segment's general purpose lane can be shared with bicyclists (no effective shoulder width required)
(2) If AADT > 1500 AND Speed Limit between (25-50 mph) AND Pavement Surface is Paved: Effective shoulder width required is 4 feet or greater
(3) If AADT > 1500 AND Speed Limit >= 50 mph and Pavement Surface is Paved: Effective shoulder width required is 6 feet or greater
The summation of the length of the shoulder sections that meet the defined effective width criteria, based on criteria above, is divided by the segment's total length to estimate the percent of the segment that accommodates bicycles as illustrated at the end of this section. If shoulder data is not available or appears erroneous, field measurements can substitute for the shoulder data.

Percent Non-SOV Trips: The percentage of non-SOV trips over distances less than 50 miles gives an indication of travel patterns along a section of the corridor that could benefit from additiona multimodal options in the future.
Thresholds that determine levels of good, fair, and poor are based on the percent non-SOV trips within each of the identified statewide significant corridors by ADOT. The thresholds shown at the end of this section represent statewide averages across those corridors.

Percent Transit Dependency: U.S. Census American Community Survey tract and state level geographic data and attributes from the tables B08201 (Number of Vehicles Available by Household Size) and B17001 (Population in Poverty within the Last 12 Months) were downloaded with margins of error included from the Census data retrieval application Data Ferret. Population ranges for each tract were determined by adding and subtracting the margin of error to each estimate in excel. The tract level attribute data was then joined to geographic tract data in GIS. Only tracts within a one mile buffer of each corridor are considered for this evaluation.
Tracts that have a statistically significantly larger number of either people in poverty or households with only one or no vehicles available than the state average are considered potentially transit dependent.
Example: The state average for zero or one vehicles households (HHs) is between $44.1 \%$ and $45.0 \%$. Tracts which have the lower bound of their range above the upper bound of the state range have a greater percentage of zero/one vehicle HHs than the state average. Tracts that have their upper bound beneath the lower bound of the state range have a lesser percentage of zero/one vehicles HHs than the state average. All other tracts that have one of their bounds overlapping with the state average cannot be considered statistically significantly different because there is a chance the value is actually the same.

In addition to transit dependency, the following attributes are added to the Multimodal Opportunities map based on available data.

- Shoulder width throughout the corridor based on ‘Shoulder Width’ GIS dataset provided by ADOT
- Intercity bus routes
- Multiuse paths within the corridor right-of-way, if applicable

Scoring:

| Volume-to-Capacity Ratios |  |  |
| :---: | :---: | :--- |
| Urban and Fringe Urban |  |  |
| Good - LOS A-C | $\mathrm{V} / \mathrm{C} \leq 0.71$ | *Note - ADOT Roadway Design Standards indicate |
| Fair - LOS D | $\mathrm{V} / \mathrm{C}>0.71 \& \leq 0.89$ | Urban and Fringe Urban roadways should be |
| designed to level of service C or better |  |  |


| Performance Level | Closure Extent |
| :---: | :---: |
| Good | $\leq 0.22$ |
| Fair | $>0.22 \& \leq 0.62$ |
| Poor | $\mathrm{V} / \mathrm{C}>0.62$ |


| $\begin{array}{\|c\|c\|}\hline \text { Performance Level } & \begin{array}{c}\text { LOTTR on Uninterrupted Flow } \\ \text { Facilities }\end{array} \\ \hline \text { Good } & <1.15\end{array}$ |  |  |
| :--- | :---: | :---: |
| Fair |  |  |
| Poor |  | $\geq 1.15 \&<1.50$ |
| Performance Level |  |  | \(\left.\begin{array}{c}LOTTR on Interrupted Flow <br>

Facilities\end{array}\right]\) <1.50

| Performance Level | Percent Transit Dependency |  |  |
| :---: | :---: | :---: | :---: |
| Good | Tracts with both zero and one vehicle <br> household population in poverty <br> percentages below the statewide average |  |  |
| Fair | Tracts with either zero and one vehicle <br> household or population in poverty <br> percentages below the statewide average |  |  |
| Poor | Tracts with both zero and one vehicle <br> household and population in poverty <br> percentages above the statewide average |  |  |
| Performance Level |  |  | Percent Bicycle Accommodation |
| Good | $\geq 90 \%$ |  |  |
| Fair |  |  |  |

## Safety Performance Area Calculation Methodologies

This section summarizes the approach for developing the primary and secondary performance measures in the Safety performance area as shown in the following graphic:


Primary Safety Index
The Safety Index is a safety performance measure based on the bi-directional (i.e., both directions combined) frequency and rate of fatal and suspected serious injury crashes, the relative cost of those types of crashes, and crash occurrences on similar roadways in Arizona. According to ADOT's 2018 Highway Safety Improvement Program Application, fatal crashes have an estimated cost that is 17.3 times the estimated cost of suspected serious injury crashes ( $\$ 9.5$ million compared to $\$ 555,000$ ).
The Combined Safety Score (CSS) is an interim measure that combines fatal and suspected serious injury crashes into a single value. The CSS is calculated using the following generalized formula: CSS $=17.3^{*}$ (Normalized Fatal Crash Rate + Frequency $)+($ Normalized Suspected Serious Injury Crash Rate + Frequency)
Because crashes vary depending on the operating environment of a particular roadway, statewide CSS values were developed for similar operating environments defined by functional classification, urban vs. rural setting, number of travel lanes, and traffic volumes. To determine the Safety Index of a particular segment, the segment CSS is compared to the average statewide CSS for the similar statewide operating environment.

The Safety Index is calculated using the following formula
Safety Index = Segment CSS / Statewide Similar Operating Environment CSS
The average annual Safety Index for a segment is compared to the statewide similar operating environment annual average, with one standard deviation from the statewide average forming the scale break points.
The more a particular segment's Safety Index value is below the statewide similar operating environment average, the better the safety performance is for that particular segment as a lower value represents fewer crashes.

## Scoring:

The scale for rating the Safety Index depends on the operating environments selected, as shown in the table below

| Similar Operating Environment | Safety Index (Overall \& Directional) |  |
| :--- | :---: | :---: |
|  | Lower Limit of <br> Average $^{*}$ | Upper Limit of <br> Average* $^{*}$ |
| 2 or 3 Lane Undivided Highway | 0.92 | 1.08 |
| 2 or 3 or 4 Lane Divided Highway | 0.81 | 1.19 |
| 4 or 5 Lane Undivided Highway | 0.78 | 1.22 |
| 6 Lane Highway | 0.76 | 1.24 |
| Rural 4 Lane Freeway with Daily Volume < 25,000 | 0.84 | 1.16 |
| Rural 4 Lane Freeway with Daily Volume > 25,000 | 0.78 | 1.22 |
| Urban 4 Lane Freeway | 0.73 | 1.27 |
| Urban or Rural 6 Lane Freeway | 0.65 | 1.35 |
| Urban > 6 Lane Freeway | 0.89 | 1.11 |

*Lower/upper limit of Average calculated as one standard deviation below/above the Mean
Some corridor segments may have a very low number of total fatal and suspected serious injury crashes. Low crash frequencies (i.e., a small sample size) can translate into performance ratings that can be unstable. In some cases, a change in crash frequency of one crash (one additional crash or one less crash) could result in a change in segment performance of two levels. To avoid reliance on performance ratings where small changes in crash frequency result in large changes in performance, the following two criteria were developed to identify segments with "insufficient data" for assessing performance for the Safety Index. Both of these criteria must be met for a segment to have "insufficient data" to reliably rate the Safety Index performance:

- If the crash sample size (total fatal plus suspected serious injury crashes) for a given segment is less than five crashes over the five-year analysis period; AND
- If a change in one crash results in a change in segment performance by two levels (i.e., a change from below average to above average performance or a change from above average to below average frequency), the segment has "insufficient data" and Safety Index performance ratings are unreliable.


## Secondary Safety Measures

The Safety performance area has four secondary measures related to fatal and suspected serious injury crashes:

- Directional Safety Index
- Strategic Traffic Safety Plan (STSP) Emphasis Areas
- Other Crash Unit Types
- Safety Hot Spots

Directional Safety Index: The Directional Safety Index shares the same calculation procedure and thresholds as the Safety Index. However, the measure is based on the directional frequency and rate of fatal and suspected serious injury crashes
Similar to the Safety Index, the segment CSS is compared to the average statewide CSS for the similar statewide operating environment. The Directional Safety Index follows the lead of the Safety Index in terms of "insufficient data" status. If the Safety Index meets both criteria for "insufficient data", the Directional Safety Index should also be changed to "insufficient data". If the Safety Index does not meet both criteria for "insufficient data", the Directional Safety Index would also not change to say "insufficient data"
STSP Emphasis Areas: ADOT's 2019 STSP identifies several emphasis areas for reducing fatal and suspected serious injury crashes. The three relevant STSP emphasis areas relate to crashes involving:

- Intersections
- Lane departures
- Pedestrians

To develop a performance measure that reflects these emphasis areas, the percentage of total fatal and suspected serious injury crashes that involves a given emphasis area on a particular segment is compared to the statewide average percentage of crashes involving that same emphasis area on roads with similar operating environments in a process similar to how the Safety Index is developed. The STSP emphasis areas performance is calculated using the following formula:
\% Crashes Involving STSP Emphasis Area = Segment Crashes Involving STSP Emphasis Area / Total Segment Crashes

Emphasis Area / Total Segment Crashes

The percentage of total crashes involving STSP emphasis areas for a segment is compared to the statewide percentages on roads with similar operating environments. One standard deviation from the statewide average percentage forms the scale break points.
When assessing the performance of the STSP emphasis areas, the more the frequency of crashes involving STSP emphasis areas is below the statewide average implies better levels of segment performance. Thus, lower values are better, similar to the Safety Index.
Scoring:
The scale for rating the STSP emphasis areas performance depends on the crash history on similar statewide operating environments, as shown in the tables below:

| Similar Operating Environment | Crashes at Intersections |  |
| :--- | :---: | :---: |
|  | Lower Limit of <br> Average* | Upper Limit of <br> Average* |
| 2 or 3 Lane Undivided Highway | $11.2 \%$ | $15.6 \%$ |
| 2 or 3 or 4 Lane Divided Highway | $23.4 \%$ | $29.3 \%$ |
| 4 or 5 Lane Undivided Highway | $43.8 \%$ | $49.5 \%$ |
| 6 Lane Highway | $57.8 \%$ | $73.2 \%$ |
| Rural 4 Lane Freeway with Daily Volume < 25,000 | $0.00 \%$ | $0.00 \%$ |
| Rural 4 Lane Freeway with Daily Volume >25,000 | $0.00 \%$ | $0.00 \%$ |
| Urban 4 Lane Freeway | $0.00 \%$ | $0.00 \%$ |
| Urban or Rural 6 Lane Freeway | $0.00 \%$ | $0.00 \%$ |
| Urban > 6 Lane Freeway | $0.00 \%$ | $0.00 \%$ |
| *Lowr |  |  |

*Lower/upper limit of Average calculated as one standard deviation below/above the Mean

| Similar Operating Environment | Crashes Involving Lane Departures |  |
| :--- | :---: | :---: |
|  | Lower Limit of <br> Average* | Upper Limit of <br> Average* |
| 2 or 3 Lane Undivided Highway | $66.9 \%$ | $74.5 \%$ |
| 2 or 3 or 4 Lane Divided Highway | $56.4 \%$ | $65.0 \%$ |
| 4 or 5 Lane Undivided Highway | $21.1 \%$ | $32.1 \%$ |
| 6 Lane Highway | $11.7 \%$ | $38.1 \%$ |
| Rural 4 Lane Freeway with Daily Volume < 25,000 | $72.8 \%$ | $76.4 \%$ |
| Rural 4 Lane Freeway with Daily Volume > 25,000 | $69.0 \%$ | $77.5 \%$ |
| Urban 4 Lane Freeway | $60.6 \%$ | $78.1 \%$ |
| Urban or Rural 6 Lane Freeway | $55.7 \%$ | $62.9 \%$ |
| Urban > 6 Lane Freeway | $40.4 \%$ | $43.2 \%$ |


| Similar Operating Environment | Crashes Involving Pedestrians |  |
| :--- | :---: | :---: |
|  | Lower Limit of <br> Average* $^{*}$ | Upper Limit of <br> Average* |
| 2 or 3 Lane Undivided Highway | $3.8 \%$ | $7.2 \%$ |
| 2 or 3 or 4 Lane Divided Highway | $2.4 \%$ | $3.6 \%$ |
| 4 or 5 Lane Undivided Highway | $8.8 \%$ | $13.5 \%$ |
| 6 Lane Highway | $0.4 \%$ | $11.9 \%$ |
| Rural 4 Lane Freeway with Daily Volume <25,000 | $1.0 \%$ | $3.3 \%$ |
| Rural 4 Lane Freeway with Daily Volume >25,000 | $0.7 \%$ | $4.7 \%$ |
| Urban 4 Lane Freeway | $0.0 \%$ | $4.9 \%$ |
| Urban or Rural 6 Lane Freeway | $4.0 \%$ | $7.9 \%$ |
| Urban > 6 Lane Freeway | $1.6 \%$ | $4.7 \%$ |

* Lower/upper limit of Average calculated as one standard deviation below/above the Mean

The STSP emphasis area secondary safety performance measures for the Safety performance area include proportions of specific types of crashes within the total fatal and suspected serious injury crash frequencies. This more detailed categorization of fatal and suspected serious injury crashes can result in low crash frequencies (i.e., a small sample size) that translate into performance ratings that can be unstable. In some cases, a change in crash frequency of one crash (one additional crash or one less crash) could result in a change in segment performance of two levels. To avoid reliance on performance ratings where small changes in crash frequency result in large changes in performance, the following criteria were developed to identify segments with "insufficient data" for assessing performance for the STSP emphasis area secondary safety performance measures. If
any of these criteria are met for a segment, that segment has "insufficient data" to reliably rate that STSP emphasis area performance:

- If the crash sample size (total fatal plus suspected serious injury crashes) for a given segment is less than five crashes over the five-year analysis period, the segment has "insufficient data" and performance ratings are unreliable. OR
- If a change in one crash results in a change in segment performance by two levels (i.e., a change from below average to above average performance or a change from above average to below average frequency), the segment has "insufficient data" and performance ratings are unreliable. OR
- If the corridor average segment crash frequency for any of the STSP emphasis area performance measures is less than two crashes over the five-year analysis period, that entire STSP emphasis area performance measure has "insufficient data" and performance ratings are unreliable.
Other Crash Unit Types: Other crash unit types of interest are:
- Truck-involved crashes
- Bicycle-involved crashes

To develop a performance measure that reflects the aforementioned crash unit types, the percentage of total fatal and suspected serious injury crashes that involves a given crash unit type on a particular segment is compared to the statewide average percentage of crashes involving that same crash unit type on roads with similar operating environments in a process similar to how the Safety Index is developed.
The crash unit type performance is calculated using the following formula:
\% Crashes Involving Crash Unit Type = Segment Crashes Involving Crash Unit Type / Total Segment Crashes
The percentage of total crashes involving each crash unit type for a segment is compared to the statewide percentages on roads with similar operating environments. One standard deviation from the statewide average percentage forms the scale break points.
When assessing the performance of the crash unit types, the more the frequency of crashes involving crash unit types is below the statewide average implies better levels of segment performance. Thus, lower values are better, similar to the Safety Index.

## Scoring:

The scale for rating the unit-involved crash performance depends on the crash history on similar statewide operating environments, as shown in the following tables.

| Similar Operating Environment | Crashes Involving Bicycles |  |
| :---: | :---: | :---: |
|  | Lower Limit of Average* | Upper Limit of Average* |
| 2 or 3 Lane Undivided Highway | 0.0\% | 3.3\% |
| 2 or 3 or 4 Lane Divided Highway | 0.0\% | 2.2\% |
| 4 or 5 Lane Undivided Highway | 0.5\% | 3.8\% |
| 6 Lane Highway | 0.0\% | 7.2\% |
| Rural 4 Lane Freeway with Daily Volume < 25,000 | 0.0\% | 0.9\% |
| Rural 4 Lane Freeway with Daily Volume > 25,000 | 0.0\% | 0.0\% |
| Urban 4 Lane Freeway | 0.0\% | 0.0\% |
| Urban or Rural 6 Lane Freeway | 0.0\% | 1.3\% |
| Urban > 6 Lane Freeway | 0.0\% | 0.0\% |

* Lower/upper limit of Average calculated as one standard deviation below/above the Mean

| Similar Operating Environment | Crashes Involving Trucks |  |
| :---: | :---: | :---: |
|  | Lower Limit of Average* | Upper Limit of Average* |
| 2 or 3 Lane Undivided Highway | 4.2\% | 8.0\% |
| 2 or 3 or 4 Lane Divided Highway | 3.7\% | 9.9\% |
| 4 or 5 Lane Undivided Highway | 0.8\% | 5.5\% |
| 6 Lane Highway | 4.3\% | 7.5\% |
| Rural 4 Lane Freeway with Daily Volume < 25,000 | 19.0\% | 22.5\% |
| Rural 4 Lane Freeway with Daily Volume > 25,000 | 8.5\% | 18.0\% |
| Urban 4 Lane Freeway | 6.9\% | 12.4\% |
| Urban or Rural 6 Lane Freeway | 5.0\% | 12.9\% |
| Urban > 6 Lane Freeway | 1.9\% | 5.1\% |

* Lower/upper limit of Average calculated as one standard deviation below/above the Mean

The crash unit types have the same "insufficient data" criteria as the STSP emphasis areas.
Safety Hot Spots: A hot spot analysis was conducted that identified abnormally high
concentrations of fatal and suspected serious injury crashes along the study corridor by direction of travel. The identification of crash concentrations involves a GIS-based function known as
"kernel density analysis". This measure is mapped for graphical display purposes with the
Directional Safety Index but is not included in the Safety performance area rating calculations.

## Freight Performance Area Calculation Methodologies

This section summarizes the approach for developing the primary and secondary performance measures in the Freight performance area as shown in the following graphic:


## Primary Freight Index

The Freight Index is a reliability performance measure based on the bi-directional truck travel time reliability (TTTR) for truck travel. The industry standard definition for the Truck Travel Time Reliability (TTTR) is the ratio of the $95^{\text {th }}$ percentile travel time to average ( $50^{\text {th }}$ percentile) travel time for trucks.
Using INRIX data provided by ADOT, four time periods for each data point were collected throughout the day (AM peak, mid-day, PM peak, and off-peak).
The highest calculated value of the four time periods is defined as the TTTR for that data point. The weighted average TTTR is calculated within each segment based on the number of data points collected and the length associated with the TMC location. The value of the weighted average TTTR across each entry is used as the TTTR for each respective segment within the corridor.
For each corridor segment, the TTTR is calculated for each direction of travel and then averaged to create a bi-directional TTTR. The Freight Index is equal to the average bi-directional TTTR for the segment.

The scale for rating the Freight Index differs between uninterrupted and interrupted flow facilities.

## Secondary Freight Measures

The Freight performance area includes three secondary measures that provide an in-depth evaluation of the different characteristics of freight performance:

- Travel Time Reliability
- Directional Truck Travel Time Reliability
- Closure Duration
- Bridge Vertical Clearance
- Bridge Vertical Clearance Hot Spots

Travel Time Reliability: Travel time reliability is a secondary measure that includes two indicators The two indicators are the directional Truck Travel Time Reliability (TTTR) and the duration a piece of a corridor is closed for any specific reason.
Truck Travel Time Reliability: The performance measure for truck travel time reliability is directional TTTR. The industry standard definition for TTTR is the ratio of $95^{\text {th }}$ percentile travel time to average ( $50^{\text {th }}$ percentile) travel time for trucks for a given corridor segment in a specific direction.

Using INRIX data provided by ADOT, four time periods for each data point were collected throughout the day (AM peak, mid-day, PM peak, and off-peak). The highest value of the four time periods calculation is defined as the TTTR for that data point. The weighted average TTTR is calculated within each segment based on the number of data points collected and the length associated with the TMC location. The value of the weighted average TTTR across each entry is used as the TTTR for each respective segment within the corridor.

Closure Duration: This performance measure related to road closures is average roadway closure (i.e., full lane closure) duration time in minutes. There are three main components to full closures that affect reliability - frequency, duration, and extent. In the freight industry, closure duration is the most important component because trucks want to minimize travel time and delay.
Data on the frequency, duration, and extent of full roadway closures on the ADOT State Highway System is available in the HCRS database that is managed and updated by ADOT.
The average closure duration in a segment - in terms of the average time a milepost is closed per mile per year on a given segment - is calculated using the following formula:
Closure Duration = Sum of Segment (Closure Clearance Time * Closure Extent) / Segment Length The segment closure duration time in minutes can then be compared to statewide averages for closure duration in minutes, with one-half standard deviation from the average forming the scale break points. The scale for rating closure duration in minutes is found at the end of this section.
Bridge Vertical Clearance: This performance measure uses the vertical clearance information from the ADOT Bridge Database to identify locations with low vertical clearance. The minimum vertical
clearance for all underpass structures (i.e., structures under which mainline traffic passes) is determined for each segment.

Bridge Vertical Clearance Hot Spots: This performance measure related to truck restrictions is the locations, or hot spots, where bridge vertical clearance issues restrict truck travel. Sixteen feet three inches (16.25') is the minimum standard vertical clearance value for state highway bridges over travel lanes.
Locations with lower vertical clearance values than the minimum standard are categorized by the ADOT Intermodal Transportation Department Engineering Permits Section as either locations where ramps exist that allow the restriction to be avoided or locations where ramps do not exist and the restriction cannot be avoided. The locations with vertical clearances below the minimum standard that cannot be ramped around are considered hot spots. This measure is mapped for graphical display purposes with the bridge vertical clearance map but is not included in the Freight performance area rating calculations.
Scoring:

| Performance Level | Freight Index |  |
| :---: | :---: | :---: |
|  | Uninterrupted Flow Facilities | Interrupted Flow Facilities |
| Good | $<1.15$ | $<1.45$ |
| Fair | $1.15-1.35$ | $1.45-1.85$ |
| Poor | $>1.35$ | $>1.85$ |


| Performance Level | TTTR |  |
| :---: | :---: | :---: |
|  | Uninterrupted Flow Facilities | Interrupted Flow Facilities |
| Good | $<1.15$ | $<1.45$ |
| Fair | $1.15-1.35$ | $1.45-1.85$ |
| Poor | $>1.35$ | $>1.85$ |


| Performance Level | Closure Duration (minutes) |
| :---: | :---: |
| Good | $<44.18$ |
| Fair | $44.18-124.86$ |
| Poor | $>124.86$ |

## Aロロт

## Appendix C: Performance Area Data

## Pavement Performance Area Data



|  |  |  |  | Direction 1 (Northound) |  |  |  | Direction 2 (Southbound) |  |  |  | Direction 1 (Northbound) |  | Direction 2 <br> (Southbound) |  | Composite |  | Pavement Index | \% Pavement Failure |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | \# of Lanes | IRI | Cracking | Rutting | \# of Lanes | IRI | Cracking | Rutting | PSR | PDI | PSR | PDI | $\begin{aligned} & \hline \text { Dir 1 } \\ & \text { (NB) } \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline \text { Dir } 2 \\ & (\mathrm{SB}) \\ & \hline \end{aligned}$ |  | Dir 1 <br> (NB) | $\begin{aligned} & \hline \text { Dir } 2 \\ & (\mathrm{SB}) \\ & \hline \end{aligned}$ |
| Segment 2 |  |  | tate? | No |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Milepost | 24 | to | 25 | 1 | 206.10 | 22.80 | 0.31 | 1 | 222.29 | 28.70 | 0.35 | 2.28 | 2.32 | 2.15 | 1.85 | 2.28 | 2.15 |  | 1 | 1 |
| Milepost | 25 | to | 26 | 1 | 191.55 | 15.82 | 0.39 | 1 | 174.09 | 17.73 | 0.28 | 2.41 | 2.74 | 2.58 | 2.75 | 2.41 | 2.58 |  | 1 | 1 |
| Milepost | 26 | to | 27 | 1 | 193.91 | 10.10 | 0.35 | 1 | 172.02 | 13.80 | 0.36 | 2.39 | 3.30 | 2.60 | 2.97 | 2.39 | 2.60 |  | 1 | 1 |
| Milepost | 27 | to | 28 | 1 | 113.90 | 8.60 | 0.09 | 1 | 118.86 | 11.80 | 0.15 | 3.24 | 3.73 | 3.18 | 3.39 | 3.39 | 3.24 |  | 0 | 1 |
| Milepost | 28 | to | 29 | 1 | 95.02 | 10.30 | 0.08 | 1 | 110.01 | 9.90 | 0.15 | 3.48 | 3.55 | 3.29 | 3.57 | 3.53 | 3.37 |  | 1 | 0 |
| Milepost | 29 | to | 30 | 1 | 144.57 | 8.22 | 0.16 | 1 | 144.27 | 13.44 | 0.21 | 2.89 | 3.73 | 2.89 | 3.19 | 2.89 | 2.89 |  | 1 | 1 |
| Milepost | 30 | to | 31 | 1 | 112.12 | 10.18 | 0.18 | 1 | 129.04 | 14.18 | 0.20 | 3.27 | 3.52 | 3.06 | 3.13 | 3.34 | 3.08 |  | 1 | 1 |
| Milepost | 31 | to | 32 | 1 | 92.74 | 10.10 | 0.16 | 1 | 108.27 | 16.40 | 0.20 | 3.51 | 3.54 | 3.31 | 2.95 | 3.53 | 3.06 |  | 1 | 1 |
| Milepost | 32 | to | 33 | 1 | 98.39 | 7.50 | 0.13 | 1 | 108.38 | 16.60 | 0.22 | 3.44 | 3.83 | 3.31 | 2.91 | 3.56 | 3.03 |  | 0 | 1 |
| Milepost | 33 | to | 34 | 1 | 95.12 | 11.50 | 0.15 | 1 | 127.14 | 16.50 | 0.22 | 3.48 | 3.41 | 3.08 | 2.92 | 3.43 | 2.97 |  | 1 | 1 |
| Milepost | 34 | to | 35 | 1 | 123.43 | 15.18 | 0.15 | 1 | 130.12 | 18.27 | 0.17 | 3.13 | 3.09 | 3.05 | 2.82 | 3.10 | 2.89 |  | 1 | 1 |
| Milepost | 35 | to | 36 | 1 | 94.10 | 4.11 | 0.17 | 1 | 121.37 | 11.56 | 0.18 | 3.50 | 4.19 | 3.15 | 3.39 | 3.71 | 3.22 |  | 0 | 1 |
| Milepost | 36 | to | 37 | 1 | 105.60 | 6.50 | 0.10 | 1 | 92.14 | 6.10 | 0.15 | 3.35 | 3.95 | 3.52 | 3.97 | 3.53 | 3.66 |  | 0 | 0 |
| Milepost | 37 | to | 38 | 1 | 103.39 | 8.00 | 0.16 | 1 | 85.21 | 2.60 | 0.15 | 3.38 | 3.76 | 3.62 | 4.42 | 3.49 | 3.86 |  | 0 | 0 |
| Milepost | 38 | to | 39 | 1 | 75.49 | 6.60 | 0.08 | 1 | 55.61 | 2.70 | 0.09 | 3.75 | 3.94 | 4.05 | 4.44 | 3.89 | 4.32 |  | 0 | 0 |
| Milepost | 39 | to | 40 | 1 | 56.61 | 0.80 | 0.08 | 1 | 52.21 | 4.60 | 0.07 | 4.03 | 4.77 | 4.10 | 4.18 | 4.25 | 4.16 |  | 0 | 0 |
| Milepost | 40 | to | 41 | 1 | 56.21 | 2.90 | 0.08 | 1 | 58.20 | 7.60 | 0.08 | 4.04 | 4.41 | 4.01 | 3.83 | 4.30 | 3.89 |  | 0 | 0 |
| Milepost | 41 | to | 42 | 1 | 55.65 | 9.90 | 0.07 | 1 | 61.34 | 5.10 | 0.07 | 4.05 | 3.59 | 3.96 | 4.12 | 3.73 | 4.07 |  | 0 | 0 |
| Milepost | 42 | to | 43 | 1 | 61.69 | 8.10 | 0.07 | 1 | 55.97 | 10.30 | 0.09 | 3.96 | 3.78 | 4.04 | 3.55 | 3.83 | 3.70 |  | 0 | 1 |
| Milepost | 43 | to | 44 | 1 | 68.81 | 8.20 | 0.05 | 1 | 60.32 | 9.80 | 0.07 | 3.85 | 3.76 | 3.98 | 3.60 | 3.79 | 3.71 |  | 0 | 0 |
| Milepost | 44 | to | 45 | 1 | 66.60 | 5.60 | 0.07 | 1 | 59.41 | 5.40 | 0.06 | 3.88 | 4.06 | 3.99 | 4.08 | 4.01 | 4.06 |  | 0 | 0 |
| Milepost | 45 | to | 46 | 1 | 74.83 | 20.50 | 0.10 | 1 | 79.70 | 17.20 | 0.09 | 3.76 | 2.66 | 3.69 | 2.93 | 2.66 | 3.16 |  | 1 | 1 |
| Milepost | 46 | to | 47 | 1 | 80.83 | 9.40 | 0.09 | 1 | 81.59 | 8.60 | 0.09 | 3.68 | 3.64 | 3.67 | 3.73 | 3.65 | 3.71 |  | 0 | 0 |
| Milepost | 47 | to | 48 | 1 | 91.71 | 0.00 | 0.05 | 1 | 94.27 | 0.00 | 0.09 | 3.53 | 4.95 | 3.49 | 4.89 | 3.96 | 3.91 |  | 0 | 0 |
| Milepost | 48 | to | 49 | 1 | 114.60 | 7.00 | 0.13 | 1 | 115.27 | 12.50 | 0.13 | 3.23 | 3.88 | 3.23 | 3.33 | 3.43 | 3.26 |  | 0 | 1 |
| Milepost | 49 | to | 50 | 1 | 95.89 | 0.00 | 0.05 | 1 | 101.41 | 0.00 | 0.07 | 3.47 | 4.96 | 3.40 | 4.92 | 3.92 | 3.86 |  | 0 | 0 |
| Milepost | 50 | to | 51 | 1 | 94.98 | 26.50 | 0.07 | 1 | 102.53 | 27.40 | 0.09 | 3.49 | 2.20 | 3.39 | 2.14 | 2.20 | 2.14 |  | 1 | 1 |
| Milepost | 51 | to | 52 | 1 | 92.15 | 54.40 | 0.12 | 1 | 118.36 | 55.10 | 0.10 | 3.52 | 0.44 | 3.19 | 0.39 | 0.44 | 0.39 |  | 1 | 1 |
| Milepost | 52 | to | 53 | 1 | 83.08 | 51.70 | 0.11 | 1 | 106.83 | 57.30 | 0.12 | 3.65 | 0.59 | 3.33 | 0.27 | 0.59 | 0.27 |  | 1 | 1 |
| Milepost | 53 | to | 54 | 1 | 131.28 | 48.70 | 0.14 | 1 | 134.33 | 50.00 | 0.16 | 3.04 | 0.77 | 3.00 | 0.69 | 0.77 | 0.69 |  | 1 | 1 |
| Milepost | 54 | to | 55 | 1 | 166.51 | 49.40 | 0.15 | 1 | 175.37 | 52.00 | 0.20 | 2.66 | 0.73 | 2.57 | 0.55 | 0.73 | 0.55 |  | 1 | 1 |
| Milepost | 55 | to | 56 | 1 | 94.47 | 35.90 | 0.17 | 1 | 112.43 | 47.90 | 0.16 | 3.49 | 1.55 | 3.26 | 0.81 | 1.55 | 0.81 |  | 1 | 1 |
| Milepost | 56 | to | 57 | 1 | 116.07 | 35.70 | 0.13 | 1 | 130.49 | 49.60 | 0.12 | 3.22 | 1.58 | 3.05 | 0.72 | 1.58 | 0.72 |  | 1 | 1 |
| Milepost | 57 | to | 58 | 1 | 118.52 | 21.40 | 0.13 | 1 | 114.01 | 51.20 | 0.13 | 3.19 | 2.59 | 3.24 | 0.62 | 2.59 | 0.62 |  | 1 | 1 |
| Milepost | 58 | to | 59 | 1 | 115.01 | 19.50 | 0.11 | 1 | 132.91 | 52.10 | 0.14 | 3.23 | 2.74 | 3.02 | 0.57 | 2.74 | 0.57 |  | 1 | 1 |
| Milepost | 59 | to | 60 | 1 | 108.25 | 41.40 | 0.10 | 1 | 111.11 | 48.80 | 0.14 | 3.31 | 1.21 | 3.28 | 0.76 | 1.21 | 0.76 |  | 1 | 1 |

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|  |  |  |  | Direction 1 (Northound) |  |  |  | Direction 2 (Southbound) |  |  |  | Direction 1 (Northbound) |  | Direction 2 (Southbound) |  | Composite |  | Pavement Index | \% Pavement Failure |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | \# of Lanes | IRI | Cracking | Rutting | \# of Lanes | IRI | Cracking | Rutting | PSR | PDI | PSR | PDI | Dir 1 <br> (NB) | $\begin{aligned} & \hline \text { Dir } 2 \\ & \text { (SB) } \\ & \hline \end{aligned}$ |  | Dir 1 <br> (NB) | $\begin{aligned} & \hline \text { Dir } 2 \\ & \text { (SB) } \end{aligned}$ |
| Milepost | 60 | to | 61 | 1 | 94.06 | 42.60 | 0.09 | 1 | 100.00 | 48.10 | 0.13 | 3.50 | 1.14 | 3.42 | 0.81 | 1.14 | 0.81 |  | 1 | 1 |
| Milepost | 61 | to | 62 | 1 | 97.32 | 18.00 | 0.10 | 1 | 87.61 | 21.10 | 0.12 | 3.45 | 2.87 | 3.58 | 2.61 | 3.04 | 2.61 |  | 1 | 1 |
| Milepost | 62 | to | 63 | 1 | 97.27 | 1.30 | 0.15 | 1 | 115.50 | 2.60 | 0.12 | 3.46 | 4.62 | 3.22 | 4.44 | 3.80 | 3.59 |  | 0 | 0 |
| Milepost | 63 | to | 64 | 1 | 71.43 | 0.10 | 0.10 | 1 | 76.99 | 0.80 | 0.14 | 3.81 | 4.92 | 3.73 | 4.73 | 4.14 | 4.03 |  | 0 | 0 |
| Milepost | 64 | to | 65 | 1 | 55.82 | 0.00 | 0.09 | 1 | 49.25 | 0.00 | 0.14 | 4.04 | 4.89 | 4.15 | 4.82 | 4.30 | 4.62 |  | 0 | 0 |
| Milepost | 65 | to | 66 | 1 | 59.34 | 0.00 | 0.08 | 1 | 57.27 | 0.00 | 0.10 | 3.99 | 4.91 | 4.02 | 4.88 | 4.27 | 4.28 |  | 0 | 0 |
| Milepost | 66 | to | 67 | 1 | 75.47 | 1.17 | 0.08 | 1 | 85.58 | 4.10 | 0.10 | 3.75 | 4.70 | 3.61 | 4.24 | 4.04 | 3.80 |  | 0 | 0 |
|  |  |  | tal | 43 |  |  |  | 43 |  |  |  |  |  |  |  |  |  |  |  | 48 |
| Weighted Average |  |  |  |  |  |  |  |  |  |  |  | 3.44 | 3.24 | 3.38 | 2.93 | 3.00 | 2.78 |  |  |  |
| Factor |  |  |  |  |  |  |  |  |  |  |  | 1.00 |  | 1.00 |  |  |  |  |  |  |
| Indicator Score |  |  |  |  |  |  |  |  |  |  |  | 3.44 |  | 3.38 |  |  |  |  |  | 55.8\% |
| Pavement Index |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 2.89 |  |  |
| Segment 3 |  | Interstate? |  | No |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Milepost | 87 | to | 88 | 2 | 79.96 | 0.00 | 0.13 | 2 | 89.10 | 0.00 | 0.12 | 3.69 | 4.83 | 3.56 | 4.85 | 4.03 | 3.95 |  | 0 | 0 |
| Milepost | 88 | to | 89 | 2 | 81.49 | 10.88 | 0.15 | 1 | 90.91 | 9.24 | 0.12 | 3.67 | 3.47 | 3.54 | 3.65 | 3.53 | 3.62 |  | 2 | 0 |
| Milepost | 89 | to | 90 | 2 | 86.38 | 10.25 | 0.14 | 1 | 90.99 | 10.20 | 0.11 | 3.60 | 3.54 | 3.54 | 3.56 | 3.56 | 3.55 |  | 2 | 1 |
| Milepost | 90 | to | 91 | 1 | 77.84 | 19.33 | 0.13 | 2 | 89.30 | 14.42 | 0.16 | 3.72 | 2.75 | 3.56 | 3.15 | 3.04 | 3.27 |  | 1 | 2 |
| Milepost | 91 | to | 92 | 1 | 63.79 | 11.90 | 0.19 | 2 | 83.45 | 14.50 | 0.14 | 3.92 | 3.35 | 3.64 | 3.15 | 3.52 | 3.30 |  | 1 | 2 |
| Milepost | 92 | to | 93 | 1 | 48.79 | 3.20 | 0.23 | 1 | 52.20 | 0.20 | 0.11 | 4.15 | 4.24 | 4.10 | 4.89 | 4.21 | 4.34 |  | 0 | 0 |
| Milepost | 93 | to | 94 | 1 | 84.94 | 0.91 | 0.18 | 1 | 55.38 | 0.64 | 0.13 | 3.62 | 4.66 | 4.05 | 4.77 | 3.93 | 4.27 |  | 0 | 0 |
| Milepost | 94 | to | 95 | 2 | 82.05 | 13.78 | 0.16 | 2 | 65.46 | 7.00 | 0.13 | 3.66 | 3.20 | 3.90 | 3.88 | 3.34 | 3.89 |  | 2 | 0 |
| Milepost | 95 | to | 96 | 2 | 94.90 | 16.50 | 0.19 | 2 | 72.50 | 13.30 | 0.15 | 3.49 | 2.94 | 3.80 | 3.25 | 3.11 | 3.41 |  | 2 | 2 |
| Milepost | 96 | to | 97 | 2 | 83.30 | 17.30 | 0.25 | 2 | 76.39 | 11.60 | 0.16 | 3.64 | 2.83 | 3.74 | 3.40 | 3.07 | 3.50 |  | 2 | 2 |
| Milepost | 97 | to | 98 | 2 | 102.00 | 22.00 | 0.30 | 2 | 75.42 | 14.70 | 0.17 | 3.39 | 2.40 | 3.75 | 3.11 | 2.40 | 3.30 |  | 2 | 2 |
| Milepost | 98 | to | 99 | 1 | 101.38 | 23.30 | 0.24 | 2 | 104.52 | 17.40 | 0.12 | 3.40 | 2.37 | 3.36 | 2.91 | 2.37 | 3.05 |  | 1 | 2 |
| Milepost | 99 | to | 100 | 1 | 91.89 | 24.09 | 0.23 | 2 | 93.79 | 17.91 | 0.12 | 3.53 | 2.32 | 3.50 | 2.87 | 2.32 | 3.06 |  | 1 | 2 |
| Milepost | 100 | to | 101 | 1 | 95.90 | 16.60 | 0.22 | 1 | 90.60 | 12.67 | 0.10 | 3.47 | 2.92 | 3.54 | 3.33 | 3.08 | 3.39 |  | 1 | 1 |
| Milepost | 101 | to | 102 | 1 | 80.68 | 16.90 | 0.18 | 1 | 61.99 | 1.09 | 0.06 | 3.68 | 2.92 | 3.95 | 4.72 | 3.15 | 4.18 |  | 1 | 0 |
| Milepost | 102 | to | 103 | 1 | 79.57 | 14.44 | 0.14 | 1 | 63.04 | 1.78 | 0.06 | 3.70 | 3.16 | 3.93 | 4.59 | 3.32 | 4.13 |  | 1 | 0 |
| Milepost | 103 | to | 104 | 1 | 75.80 | 13.60 | 0.12 | 1 | 79.25 | 7.40 | 0.08 | 3.75 | 3.24 | 3.70 | 3.85 | 3.39 | 3.81 |  | 1 | 0 |
|  |  |  |  |  |  |  |  | 26 |  |  |  |  |  |  |  |  |  |  |  | 36 |
| Weighted Average |  |  |  |  |  |  |  |  |  |  |  | 3.63 | 3.26 | 3.69 | 3.64 | 3.27 | 3.57 |  |  |  |
| Factor |  |  |  |  |  |  |  |  |  |  |  | 1.00 |  | 1.00 |  |  |  |  |  |  |
| Indicator Score |  |  |  |  |  |  |  |  |  |  |  | 3.63 |  | 3.69 |  |  |  |  |  | 72.0\% |
| Pavement Index |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 3.42 |  |  |



|  |  |  |  | Direction 1 (Northound) |  |  |  | Direction 2 (Southbound) |  |  |  | Direction 1 (Northbound) |  | Direction 2 (Southbound) |  | Composite |  | Pavement Index | \% Pavement Failure |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | \# of Lanes | IRI | Cracking | Rutting | \# of Lanes | IRI | Cracking | Rutting | PSR | PDI | PSR | PDI | $\begin{aligned} & \hline \text { Dir } 1 \\ & \text { (NB) } \end{aligned}$ | $\begin{aligned} & \hline \text { Dir } 2 \\ & (\mathrm{SB}) \\ & \hline \end{aligned}$ |  | Dir 1 <br> (NB) | $\begin{gathered} \text { Dir } 2 \\ (S B) \end{gathered}$ |
| Segment 6 |  | Interstate? |  | No |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Milepost | 330 | to | 331 | 2 | 127.11 | 9.50 | 0.14 | 2 | 154.98 | 14.30 | 0.19 | 3.08 | 3.61 | 2.77 | 3.13 | 3.24 | 2.77 |  | 0 | 2 |
| Milepost | 331 | to | 332 | 2 | 113.47 | 8.67 | 0.25 | 2 | 126.49 | 11.78 | 0.28 | 3.25 | 3.59 | 3.09 | 3.25 | 3.35 | 3.14 |  | 0 | 2 |
| Milepost | 332 | to | 333 | 2 | 96.14 | 10.09 | 0.21 | 2 | 79.35 | 13.64 | 0.25 | 3.47 | 3.50 | 3.70 | 3.13 | 3.49 | 3.30 |  | 2 | 2 |
| Milepost | 333 | to | 334 | 2 | 97.53 | 5.60 | 0.27 | 2 | 93.96 | 14.20 | 0.29 | 3.45 | 3.89 | 3.50 | 3.04 | 3.58 | 3.18 |  | 0 | 2 |
| Milepost | 334 | to | 335 | 2 | 110.73 | 9.60 | 0.29 | 2 | 82.52 | 12.20 | 0.27 | 3.28 | 3.45 | 3.65 | 3.23 | 3.33 | 3.36 |  | 0 | 2 |
| Milepost | 335 | to | 336 | 2 | 167.89 | 10.30 | 0.22 | 2 | 123.39 | 9.70 | 0.21 | 2.64 | 3.47 | 3.13 | 3.53 | 2.64 | 3.25 |  | 2 | 0 |
| Milepost | 336 | to | 337 | 2 | 150.87 | 4.70 | 0.21 | 2 | 147.59 | 3.50 | 0.18 | 2.82 | 4.07 | 2.85 | 4.26 | 2.82 | 2.85 |  | 2 | 2 |
| Milepost | 337 | to | 338 | 2 | 104.34 | 2.90 | 0.19 | 2 | 106.70 | 1.80 | 0.25 | 3.36 | 4.33 | 3.33 | 4.42 | 3.65 | 3.66 |  | 0 | 0 |
| Milepost | 338 | to | 339 | 2 | 134.32 | 8.80 | 0.18 | 2 | 107.76 | 12.13 | 0.16 | 3.00 | 3.65 | 3.32 | 3.35 | 3.20 | 3.34 |  | 0 | 2 |
| Milepost | 339 | to | 340 | 2 | 125.13 | 12.30 | 0.14 | 2 | 124.26 | 13.50 | 0.11 | 3.11 | 3.35 | 3.12 | 3.25 | 3.18 | 3.16 |  | 2 | 2 |
| Total |  |  |  | 20 |  |  |  | 20 |  |  |  |  |  |  |  |  |  |  |  | 24 |
| Weighted Average |  |  |  |  |  |  |  |  |  |  |  | 3.15 | 3.69 | 3.25 | 3.46 | 3.25 | 3.20 |  |  |  |
| Factor |  |  |  |  |  |  |  |  |  |  |  | 1.00 |  | 1.00 |  |  |  |  |  |  |
| Indicator Score |  |  |  |  |  |  |  |  |  |  |  | 3.15 |  | 3.25 |  |  |  |  |  | 60.0\% |
| Pavement Index |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 3.23 |  |  |
| Segment 7 |  | Interstate? |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Milepost | 300 | to | 301 | 1 | 138.79 | 22.60 | 0.27 | 1 | 126.64 | 14.80 | 0.11 | 2.95 | 2.39 | 3.09 | 3.13 | 2.39 | 3.10 |  | 1 | 1 |
| Milepost | 301 | to | 302 | 1 | 141.09 | 26.30 | 0.29 | 1 | 138.65 | 22.00 | 0.21 | 2.93 | 2.09 | 2.95 | 2.49 | 2.09 | 2.49 |  | 1 | 1 |
| Milepost | 302 | to | 303 | 1 | 147.46 | 21.50 | 0.25 | 1 | 123.68 | 13.80 | 0.20 | 2.86 | 2.49 | 3.13 | 3.17 | 2.49 | 3.14 |  | 1 | 1 |
| Milepost | 303 | to | 304 | 1 | 176.01 | 18.80 | 0.22 | 1 | 130.39 | 28.40 | 0.20 | 2.56 | 2.73 | 3.05 | 2.04 | 2.56 | 2.04 |  | 1 | 1 |
| Milepost | 304 | to | 305 | 1 | 185.95 | 21.50 | 0.25 | 1 | 145.12 | 23.50 | 0.17 | 2.47 | 2.49 | 2.88 | 2.41 | 2.47 | 2.41 |  | 1 | 1 |
| Milepost | 305 | to | 306 | 1 | 153.63 | 12.80 | 0.19 | 1 | 131.47 | 14.10 | 0.15 | 2.79 | 3.27 | 3.03 | 3.18 | 2.79 | 3.08 |  | 1 | 1 |
| Milepost | 306 | to | 307 | 1 | 200.36 | 19.60 | 0.25 | 1 | 174.03 | 17.90 | 0.19 | 2.34 | 2.64 | 2.58 | 2.83 | 2.34 | 2.58 |  | 1 | 1 |
| Milepost | 307 | to | 308 | 1 | 180.01 | 23.50 | 0.25 | 1 | 182.51 | 16.80 | 0.24 | 2.52 | 2.35 | 2.50 | 2.88 | 2.52 | 2.50 |  | 1 | 1 |
| Milepost | 308 | to | 309 | 1 | 208.74 | 20.30 | 0.20 | 1 | 189.61 | 19.00 | 0.23 | 2.26 | 2.63 | 2.43 | 2.71 | 2.26 | 2.43 |  | 1 | 1 |
| Milepost | 309 | to | 310 | 1 | 124.53 | 18.60 | 0.29 | 1 | 115.33 | 17.40 | 0.19 | 3.11 | 2.67 | 3.23 | 2.87 | 2.67 | 2.98 |  | 1 | 1 |
| Milepost | 310 | to | 311 | 1 | 134.57 | 19.80 | 0.27 | 1 | 113.57 | 17.90 | 0.18 | 3.00 | 2.60 | 3.25 | 2.84 | 2.60 | 2.96 |  | 1 | 1 |
| Milepost | 311 | to | 312 | 1 | 155.05 | 20.10 | 0.21 | 1 | 136.63 | 17.30 | 0.17 | 2.77 | 2.64 | 2.98 | 2.89 | 2.77 | 2.95 |  | 1 | 1 |
| Milepost | 312 | to | 313 | 1 | 144.03 | 19.50 | 0.17 | 1 | 142.72 | 18.90 | 0.17 | 2.89 | 2.72 | 2.91 | 2.77 | 2.72 | 2.86 |  | 1 | 1 |
| Milepost | 313 | to | 314 | 1 | 208.71 | 22.40 | 0.21 | 1 | 133.19 | 14.90 | 0.19 | 2.26 | 2.46 | 3.01 | 3.08 | 2.26 | 3.03 |  | 1 | 1 |
| Milepost | 314 | to | 315 | 1 | 133.22 | 20.67 | 0.16 | 1 | 109.13 | 6.89 | 0.14 | 3.01 | 2.63 | 3.30 | 3.89 | 2.63 | 3.48 |  | 1 | 0 |
| Milepost | 326 | to | 327 | 1 | 95.77 | 6.44 | 0.13 | 1 | 78.45 | 8.67 | 0.13 | 3.47 | 3.95 | 3.71 | 3.71 | 3.62 | 3.71 |  | 0 | 0 |
| Milepost | 327 | to | 328 | 1 | 106.55 | 12.09 | 0.14 | 1 | 84.79 | 9.09 | 0.11 | 3.34 | 3.37 | 3.62 | 3.67 | 3.36 | 3.66 |  | 1 | 0 |
| Milepost | 328 | to | 329 | 1 | 92.36 | 11.00 | 0.14 | 1 | 75.99 | 8.10 | 0.12 | 3.52 | 3.47 | 3.75 | 3.77 | 3.48 | 3.76 |  | 1 | 0 |
| Milepost | 329 | to | 330 | 1 | 97.59 | 14.30 | 0.21 | 1 | 116.32 | 13.50 | 0.18 | 3.45 | 3.12 | 3.21 | 3.21 | 3.22 | 3.21 |  | 1 | 1 |
| Total |  |  |  | 19 |  |  |  | 19 |  |  |  |  |  |  |  |  |  |  |  | 33 |



|  |  |  |  | Direction 1 (Northound) |  |  |  | Direction 2 (Southbound) |  |  |  | Direction 1 (Northbound) |  | Direction 2 (Southbound) |  | Composite |  | Pavement Index | \% Pavement Failure |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | \# of Lanes | IRI | Cracking | Rutting | \# of Lanes | IRI | Cracking | Rutting | PSR | PDI | PSR | PDI | $\begin{aligned} & \hline \text { Dir } 1 \\ & \text { (NB) } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { Dir } 2 \\ & \text { (SB) } \end{aligned}$ |  | $\begin{aligned} & \hline \text { Dir } 1 \\ & \text { (NB) } \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline \text { Dir } 2 \\ & \text { (SB) } \end{aligned}$ |
| Milepost | 287 | to | 288 | 1 | 126.80 | 16.20 | 0.19 | 1 | 111.29 | 18.20 | 0.28 | 3.09 | 2.97 | 3.28 | 2.72 | 3.01 | 2.72 |  | 1 | 1 |
| Milepost | 288 | to | 289 | 1 | 105.30 | 17.50 | 0.19 | 1 | 79.66 | 18.70 | 0.31 | 3.35 | 2.87 | 3.69 | 2.63 | 3.01 | 2.63 |  | 1 | 1 |
| Milepost | 289 | to | 290 | 1 | 98.84 | 18.30 | 0.18 | 1 | 96.43 | 17.60 | 0.28 | 3.43 | 2.81 | 3.47 | 2.76 | 3.00 | 2.97 |  | 1 | 1 |
| Milepost | 290 | to | 291 | 1 | 87.12 | 17.50 | 0.12 | 1 | 93.03 | 18.10 | 0.26 | 3.59 | 2.90 | 3.51 | 2.74 | 3.11 | 2.74 |  | 1 | 1 |
| Milepost | 291 | to | 292 | 1 | 111.84 | 19.70 | 0.12 | 1 | 104.90 | 19.50 | 0.23 | 3.27 | 2.73 | 3.36 | 2.67 | 2.73 | 2.67 |  | 1 | 1 |
| Milepost | 292 | to | 293 | 1 | 145.43 | 10.43 | 0.21 | 1 | 104.98 | 18.57 | 0.19 | 2.88 | 3.47 | 3.36 | 2.78 | 2.88 | 2.95 |  | 1 | 1 |
|  |  |  | otal | 19 |  |  |  | 19 |  |  |  |  |  |  |  |  |  |  |  | 30 |
|  |  |  | Weighte | Average |  |  |  |  |  |  |  | 3.10 | 2.81 | 3.35 | 2.75 | 2.69 | 2.69 |  |  |  |
|  |  |  | Factor |  |  |  |  |  |  |  |  | 1.00 |  | 1.00 |  |  |  |  |  |  |
|  |  |  | dicato | core |  |  |  |  |  |  |  | 3.10 |  | 3.35 |  |  |  |  |  | 78.9\% |
|  |  |  | aveme | Index |  |  |  |  |  |  |  |  |  |  |  |  |  | 2.69 |  |  |
| Segment 11 |  |  | state? | No |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Milepost | 270 | to | 271 | 1 | 133.24 | 17.50 | 0.14 | 1 | 103.12 | 6.60 | 0.10 | 3.01 | 2.90 | 3.38 | 3.94 | 2.93 | 3.55 |  | 1 | 0 |
| Milepost | 271 | to | 272 | 1 | 97.07 | 25.75 | 0.15 | 1 | 113.70 | 19.63 | 0.16 | 3.46 | 2.26 | 3.25 | 2.71 | 2.26 | 2.71 |  | 1 | 1 |
| Milepost | 272 | to | 273 | 1 | 120.95 | 29.80 | 0.20 | 1 | 130.61 | 29.20 | 0.22 | 3.16 | 1.94 | 3.04 | 1.97 | 1.94 | 1.97 |  | 1 | 1 |
| Milepost | 273 | to | 274 | 1 | 97.20 | 28.50 | 0.19 | 1 | 96.26 | 31.50 | 0.21 | 3.46 | 2.04 | 3.47 | 1.82 | 2.04 | 1.82 |  | 1 | 1 |
|  |  |  | otal | 4 |  |  |  | 4 |  |  |  |  |  |  |  |  |  |  |  | 7 |
|  |  |  | Weighted | Average |  |  |  |  |  |  |  | 3.27 | 2.28 | 3.28 | 2.61 | 2.29 | 2.51 |  |  |  |
|  |  |  | actor |  |  |  |  |  |  |  |  | 1.00 |  | 1.00 |  |  |  |  |  |  |
|  |  |  | ndicato | core |  |  |  |  |  |  |  | 3.27 |  | 3.28 |  |  |  |  |  | 87.5\% |
|  |  |  | Paveme | Index |  |  |  |  |  |  |  |  |  |  |  |  |  | 2.40 |  |  |
| Segment |  |  | state? | No |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Milepost | 255 | to | 256 | 1 | 91.72 | 10.22 | 0.22 | 1 | 109.77 | 5.89 | 0.20 | 3.53 | 3.48 | 3.29 | 3.94 | 3.49 | 3.49 |  | 1 | 0 |
| Milepost | 256 | to | 257 | 1 | 97.25 | 10.60 | 0.20 | 1 | 98.29 | 12.90 | 0.17 | 3.46 | 3.46 | 3.44 | 3.28 | 3.46 | 3.33 |  | 1 | 1 |
| Milepost | 257 | to | 258 | 1 | 120.21 | 10.00 | 0.22 | 1 | 107.64 | 13.60 | 0.21 | 3.17 | 3.49 | 3.32 | 3.18 | 3.26 | 3.22 |  | 0 | 1 |
| Milepost | 258 | to | 259 | 1 | 137.73 | 11.20 | 0.11 | 1 | 142.32 | 12.00 | 0.10 | 2.96 | 3.46 | 2.91 | 3.39 | 3.11 | 3.05 |  | 1 | 1 |
| Milepost | 259 | to | 260 | 1 | 107.51 | 11.30 | 0.14 | 1 | 86.23 | 10.50 | 0.14 | 3.32 | 3.44 | 3.60 | 3.52 | 3.41 | 3.54 |  | 1 | 1 |
| Milepost | 260 | to | 261 | 1 | 84.39 | 0.90 | 0.10 | 1 | 88.33 | 1.80 | 0.12 | 3.63 | 4.74 | 3.57 | 4.57 | 3.96 | 3.87 |  | 0 | 0 |
| Milepost | 261 | to | 262 | 1 | 97.36 | 4.10 | 0.11 | 1 | 77.29 | 2.30 | 0.14 | 3.45 | 4.24 | 3.73 | 4.47 | 3.69 | 3.95 |  | 0 | 0 |
| Milepost | 262 | to | 263 | 1 | 129.04 | 5.70 | 0.14 | 1 | 90.10 | 2.20 | 0.16 | 3.06 | 4.02 | 3.55 | 4.47 | 3.35 | 3.83 |  | 0 | 0 |
| Milepost | 263 | to | 264 | 1 | 110.42 | 5.30 | 0.16 | 1 | 71.29 | 1.70 | 0.11 | 3.29 | 4.06 | 3.81 | 4.58 | 3.52 | 4.04 |  | 0 | 0 |
| Milepost | 264 | to | 265 | 1 | 93.34 | 4.70 | 0.14 | 1 | 70.83 | 6.60 | 0.13 | 3.51 | 4.14 | 3.82 | 3.93 | 3.70 | 3.90 |  | 0 | 0 |
| Milepost | 265 | to | 266 | 1 | 108.75 | 1.80 | 0.12 | 1 | 70.36 | 0.50 | 0.11 | 3.31 | 4.56 | 3.83 | 4.82 | 3.68 | 4.12 |  | 0 | 0 |
| Milepost | 266 | to | 267 | 1 | 141.17 | 21.00 | 0.14 | 1 | 96.58 | 3.60 | 0.11 | 2.92 | 2.62 | 3.46 | 4.31 | 2.62 | 3.72 |  | 1 | 0 |
| Milepost | 267 | to | 268 | 1 | 167.42 | 17.60 | 0.14 | 1 | 117.95 | 4.20 | 0.10 | 2.65 | 2.89 | 3.19 | 4.23 | 2.65 | 3.50 |  | 1 | 0 |
| Milepost | 268 | to | 269 | 1 | 63.19 | 0.00 | 0.14 | 1 | 64.04 | 0.00 | 0.16 | 3.93 | 4.82 | 3.92 | 4.78 | 4.20 | 4.18 |  | 0 | 0 |
| Milepost | 269 | to | 270 | 1 | 134.85 | 3.30 | 0.14 | 1 | 97.85 | 2.20 | 0.11 | 3.00 | 4.32 | 3.45 | 4.51 | 3.39 | 3.77 |  | 0 | 0 |



|  |  |  |  | Direction 1 (Northound) |  |  |  | Direction 2 (Southbound) |  |  |  | Direction 1 (Northbound) |  | Direction 2 (Southbound) |  | Composite |  | Pavement Index | \% Pavement Failure |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | \# of Lanes | IRI | Cracking | Rutting | \# of Lanes | IRI | Cracking | Rutting | PSR | PDI | PSR | PDI | $\begin{aligned} & \hline \text { Dir } 1 \\ & \text { (NB) } \end{aligned}$ | $\begin{aligned} & \hline \text { Dir } 2 \\ & \text { (SB) } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \hline \text { Dir } 1 \\ & \text { (NB) } \end{aligned}$ | $\begin{aligned} & \hline \text { Dir } 2 \\ & \text { (SB) } \end{aligned}$ |
| Milepost | 238 | to | 239 | 1 | 132.73 | 23.60 | 0.25 | 1 | 97.66 | 19.00 | 0.23 | 3.02 | 2.33 | 3.45 | 2.71 | 2.33 | 2.71 |  | 1 | 1 |
| Milepost | 239 | to | 240 | 1 | 106.31 | 17.64 | 0.20 | 1 | 91.31 | 16.80 | 0.21 | 3.34 | 2.84 | 3.53 | 2.90 | 2.99 | 3.09 |  | 1 | 1 |
| Milepost | 240 | to | 241 | 1 | 152.11 | 24.50 | 0.22 | 1 | 78.76 | 18.55 | 0.13 | 2.81 | 2.30 | 3.71 | 2.82 | 2.30 | 3.08 |  | 1 | 1 |
| Milepost | 241 | to | 242 | 1 | 114.50 | 20.73 | 0.16 | 1 | 88.26 | 17.82 | 0.17 | 3.24 | 2.63 | 3.58 | 2.85 | 2.63 | 3.07 |  | 1 | 1 |
| Milepost | 242 | to | 243 | 1 | 87.02 | 17.50 | 0.16 | 1 | 75.47 | 10.20 | 0.16 | 3.59 | 2.89 | 3.75 | 3.53 | 3.10 | 3.60 |  | 1 | 1 |
| Total |  |  |  | 16 |  |  |  | 16 |  |  |  |  |  |  |  |  |  |  |  | 14 |
| Weighted Average |  |  |  |  |  |  |  |  |  |  |  | 3.66 | 3.78 | 3.82 | 3.99 | 3.56 | 3.79 |  |  |  |
| Factor |  |  |  |  |  |  |  |  |  |  |  | 1.00 |  | 1.00 |  |  |  |  |  |  |
| Indicator Score |  |  |  |  |  |  |  |  |  |  |  | 3.66 |  | 3.82 |  |  |  |  |  | 43.8\% |
| Pavement Index |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 3.68 |  |  |
| Segment 15 |  | Interstate? |  | No |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Milepost | 225 | to | 226 | 1 | 78.40 | 0.10 | 0.11 | 1 | 76.20 | 0.10 | 0.14 | 3.71 | 4.92 | 3.74 | 4.89 | 4.07 | 4.09 |  | 0 | 0 |
| Milepost | 226 | to | 227 | 1 | 79.87 | 0.89 | 0.08 | 1 | 89.56 | 0.00 | 0.10 | 3.69 | 4.75 | 3.56 | 4.89 | 4.01 | 3.96 |  | 0 | 0 |
| Total |  |  |  | 2 |  |  |  | 2 |  |  |  |  |  |  |  |  |  |  |  | 0 |
| Weighted Average |  |  |  |  |  |  |  |  |  |  |  | 3.70 | 4.83 | 3.65 | 4.89 | 4.04 | 4.02 |  |  |  |
| Factor |  |  |  |  |  |  |  |  |  |  |  | 1.00 |  | 1.00 |  |  |  |  |  |  |
| Indicator Score |  |  |  |  |  |  |  |  |  |  |  | 3.70 |  | 3.65 |  |  |  |  |  | 0.0\% |
| Pavement Index |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 4.03 |  |  |
| Segment 16 |  | Interstate? |  | No |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Milepost | 223 | to | 224 | 1 | 42.13 | 0.00 | 0.10 | 1 | 42.38 | 0.00 | 0.16 | 4.26 | 4.88 | 4.26 | 4.77 | 4.70 | 4.62 |  | 0 | 0 |
| Milepost | 224 | to | 225 | 1 | 46.95 | 2.20 | 0.11 | 1 | 56.22 | 0.00 | 0.16 | 4.18 | 4.51 | 4.04 | 4.78 | 4.41 | 4.26 |  | 0 | 0 |
| Total |  |  |  | 2 |  |  |  | 2 |  |  |  |  |  |  |  |  |  |  |  | 0 |
| Weighted Average |  |  |  |  |  |  |  |  |  |  |  | 4.22 | 4.70 | 4.15 | 4.78 | 4.55 | 4.44 |  |  |  |
| Factor |  |  |  |  |  |  |  |  |  |  |  | 1.00 |  | 1.00 |  |  |  |  |  |  |
| Indicator Score |  |  |  |  |  |  |  |  |  |  |  | 4.22 |  | 4.15 |  |  |  |  |  | 0.0\% |
| Pavement Index |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 4.50 |  |  |
| Segment 17 |  | Interstate? |  | No |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Milepost | 212 | to | 213 | 2 | 84.53 | 16.70 | 0.15 | 2 | 85.70 | 20.50 | 0.21 | 3.63 | 2.96 | 3.61 | 2.61 | 3.16 | 2.61 |  | 2 | 2 |
| Milepost | 213 | to | 214 | 2 | 88.75 | 9.58 | 0.12 | 2 | 74.12 | 17.33 | 0.12 | 3.57 | 3.62 | 3.77 | 2.92 | 3.60 | 3.17 |  | 0 | 2 |
| Milepost | 214 | to | 215 | 2 | 79.56 | 3.60 | 0.10 | 2 | 59.86 | 13.70 | 0.12 | 3.70 | 4.31 | 3.98 | 3.23 | 3.88 | 3.46 |  | 0 | 2 |
| Milepost | 215 | to | 216 | 2 | 58.21 | 11.70 | 0.13 | 2 | 47.57 | 17.20 | 0.11 | 4.01 | 3.41 | 4.17 | 2.93 | 3.59 | 3.30 |  | 2 | 2 |
| Milepost | 216 | to | 217 | 2 | 58.68 | 13.30 | 0.15 | 2 | 49.74 | 15.90 | 0.14 | 4.00 | 3.25 | 4.14 | 3.03 | 3.48 | 3.36 |  | 2 | 2 |
| Milepost | 217 | to | 218 | 2 | 61.99 | 12.90 | 0.13 | 2 | 53.85 | 17.00 | 0.15 | 3.95 | 3.30 | 4.07 | 2.94 | 3.49 | 3.28 |  | 2 | 2 |
| Milepost | 218 | to | 219 | 2 | 55.53 | 12.90 | 0.11 | 2 | 54.52 | 11.30 | 0.12 | 4.05 | 3.30 | 4.06 | 3.45 | 3.53 | 3.63 |  | 2 | 2 |
| Milepost | 219 | to | 220 | 2 | 56.84 | 16.30 | 0.11 | 2 | 62.39 | 3.30 | 0.09 | 4.03 | 3.01 | 3.94 | 4.35 | 3.31 | 4.23 |  | 2 | 0 |
| Milepost | 220 | to | 221 | 2 | 52.89 | 12.90 | 0.09 | 2 | 62.78 | 14.10 | 0.13 | 4.09 | 3.31 | 3.94 | 3.19 | 3.54 | 3.41 |  | 2 | 2 |


|  |  |  |  |  | ction 1 | Northound) |  |  | tion 2 ( | outhbound |  |  | tion 1 ound) |  | tion 2 bound) |  | site |  | \% Paven | t Failure |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | \# of Lanes | IRI | Cracking | Rutting | \# of Lanes | IRI | Cracking | Rutting | PSR | PDI | PSR | PDI | $\begin{aligned} & \hline \text { Dir } 1 \\ & \text { (NB) } \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline \text { Dir } 2 \\ & (S B) \\ & \hline \end{aligned}$ | Pavement Index | $\begin{aligned} & \hline \text { Dir } 1 \\ & \text { (NB) } \\ & \hline \end{aligned}$ | Dir 2 <br> (SB) |
| Milepost | 221 | to | 222 | 2 | 54.06 | 15.30 | 0.09 | 2 | 53.77 | 7.70 | 0.10 | 4.07 | 3.09 | 4.08 | 3.82 | 3.39 | 3.90 |  | 2 | 0 |
| Milepost | 222 | to | 223 | 1 | 40.37 | 4.33 | 0.13 | 1 | 49.89 | 1.10 | 0.12 | 4.29 | 4.20 | 4.14 | 4.68 | 4.23 | 4.52 |  | 0 | 0 |
|  |  |  | Total | 21 |  |  |  | 21 |  |  |  |  |  |  |  |  |  |  |  | 32 |
| Weighted Average |  |  |  |  |  |  |  |  |  |  |  | 3.93 | 3.40 | 3.99 | 3.32 | 3.53 | 3.49 |  |  |  |
| Factor |  |  |  |  |  |  |  |  |  |  |  | 1.00 |  | 1.00 |  |  |  |  |  |  |
| Indicator Score |  |  |  |  |  |  |  |  |  |  |  | 3.93 |  | 3.99 |  |  |  |  |  | 76.2\% |
| Pavement Index |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 3.51 |  |  |


| Segment 18 |  | Interstate? |  | $\begin{gathered} \text { No } \\ \hline \end{gathered}$ |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Milepost | 205 | to | 206 |  | 85.21 | 6.70 | 0.12 | 2 | 66.56 | 12.90 | 0.16 | 3.62 | 3.92 | 3.88 | 3.28 | 3.83 | 3.46 |  | 0 | 2 |
| Milepost | 206 | to | 207 | 2 | 93.59 | 17.70 | 0.13 | 2 | 58.61 | 10.50 | 0.15 | 3.50 | 2.89 | 4.00 | 3.51 | 3.07 | 3.66 |  | 2 | 2 |
| Milepost | 207 | to | 208 | 2 | 86.46 | 17.55 | 0.13 | 2 | 78.48 | 16.70 | 0.15 | 3.60 | 2.90 | 3.71 | 2.96 | 3.11 | 3.18 |  | 2 | 2 |
| Milepost | 208 | to | 209 | 2 | 73.05 | 16.33 | 0.13 | 2 | 68.26 | 16.00 | 0.14 | 3.79 | 3.00 | 3.86 | 3.02 | 3.24 | 3.27 |  | 2 | 2 |
| Milepost | 209 | to | 210 | 2 | 90.67 | 16.40 | 0.12 | 2 | 65.90 | 15.50 | 0.13 | 3.54 | 3.00 | 3.89 | 3.07 | 3.16 | 3.32 |  | 2 | 2 |
| Milepost | 210 | to | 211 | 2 | 75.04 | 17.44 | 0.12 | 2 | 58.32 | 15.44 | 0.09 | 3.76 | 2.91 | 4.01 | 3.08 | 3.16 | 3.36 |  | 2 | 2 |
| Milepost | 211 | to | 212 | 2 | 90.91 | 16.40 | 0.18 | 2 | 97.41 | 13.60 | 0.19 | 3.54 | 2.96 | 3.45 | 3.20 | 3.14 | 3.27 |  | 2 | 2 |
|  |  |  | otal | 14 |  |  |  | 14 |  |  |  |  |  |  |  |  |  |  |  | 26 |
| Weighted Average |  |  |  |  |  |  |  |  |  |  |  | 3.62 | 3.08 | 3.83 | 3.16 | 3.24 | 3.36 |  |  |  |
| Factor |  |  |  |  |  |  |  |  |  |  |  | 1.00 |  | 1.00 |  |  |  |  |  |  |
| Indicator Score |  |  |  |  |  |  |  |  |  |  |  | 3.62 |  | 3.83 |  |  |  |  |  | 92.9\% |
| Pavement Index |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 3.30 |  |  |


| Segment 19 |  | Interstate? |  | $\begin{gathered} \text { No } \\ \hline 2 \end{gathered}$ |  |  | 0.15 | 2 | 101.64 | 11.88 | 0.16 | 3.58 | 3.66 | 3.40 | 3.38 | 3.63 | 3.38 | 0 |  | 2 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Milepost | 199 | to | 200 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Milepost | 200 | to | 201 | 2 | 82.45 | 7.67 | 0.16 | 2 | 65.15 | 15.70 | 0.13 | 3.66 | 3.79 | 3.90 | 3.05 | 3.75 | 3.31 |  | 0 | 2 |
| Milepost | 201 | to | 202 | 2 | 90.92 | 15.20 | 0.20 | 2 | 77.20 | 1.90 | 0.14 | 3.54 | 3.05 | 3.73 | 4.53 | 3.20 | 3.97 |  | 2 | 0 |
| Milepost | 202 | to | 203 | 2 | 99.95 | 8.40 | 0.15 | 2 | 84.48 | 4.80 | 0.14 | 3.42 | 3.72 | 3.63 | 4.13 | 3.51 | 3.78 |  | 0 | 0 |
| Milepost | 203 | to | 204 | 2 | 84.96 | 9.80 | 0.12 | 2 | 87.68 | 0.50 | 0.06 | 3.62 | 3.59 | 3.58 | 4.85 | 3.60 | 3.96 |  | 0 | 0 |
| Milepost | 204 | to | 205 | 2 | 84.70 | 10.00 | 0.10 | 2 | 83.94 | 17.00 | 0.16 | 3.62 | 3.58 | 3.63 | 2.93 | 3.59 | 3.14 |  | 0 | 2 |
| Total |  |  |  | 12 |  |  |  | 12 |  |  |  |  |  |  |  |  |  |  |  | 8 |
| Weighted Average |  |  |  |  |  |  |  |  |  |  |  | 3.57 | 3.57 | 3.65 | 3.81 | 3.55 | 3.59 |  |  |  |
| Factor |  |  |  |  |  |  |  |  |  |  |  | 1.00 |  | 1.00 |  |  |  |  |  |  |
| Indicator Score |  |  |  |  |  |  |  |  |  |  |  | 3.57 |  | 3.65 |  |  |  |  |  | 33.3\% |
| Pavement Index |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 3.57 |  |  |


|  |  |  |  |  | tion 1 | (orthound) |  |  | ion 2 (S | uthbound |  |  | ion 1 ound) |  | ion 2 ound) | Com | site |  | \% Paver | Failure |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | \# of Lanes | IRI | Cracking | Rutting | \# of Lanes | IRI | Cracking | Rutting | PSR | PDI | PSR | PDI | $\begin{aligned} & \hline \text { Dir } 1 \\ & \text { (NB) } \end{aligned}$ | $\begin{aligned} & \hline \text { Dir } 2 \\ & (\mathrm{SB}) \\ & \hline \end{aligned}$ | Pavement Index | $\begin{aligned} & \hline \text { Dir } 1 \\ & \text { (NB) } \end{aligned}$ | $\begin{aligned} & \hline \text { Dir } 2 \\ & (S B) \end{aligned}$ |
| Segment |  |  | rstate? | No |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Milepost | 194 | to | 195 | 2 | 71.23 | 0.20 | 0.10 | 2 | 69.98 | 0.00 | 0.09 | 3.81 | 4.90 | 3.83 | 4.89 | 4.14 | 4.15 |  | 0 | 0 |
| Milepost | 195 | to | 196 | 2 | 66.02 | 0.20 | 0.10 | 2 | 73.54 | 0.20 | 0.10 | 3.89 | 4.90 | 3.78 | 4.90 | 4.19 | 4.12 |  | 0 | 0 |
| Milepost | 196 | to | 197 | 2 | 70.69 | 0.00 | 0.12 | 2 | 82.33 | 0.10 | 0.09 | 3.82 | 4.85 | 3.66 | 4.93 | 4.13 | 4.04 |  | 0 | 0 |
| Milepost | 197 | to | 198 | 2 | 65.25 | 0.00 | 0.10 | 2 | 62.05 | 0.10 | 0.09 | 3.90 | 4.88 | 3.95 | 4.94 | 4.19 | 4.25 |  | 0 | 0 |
| Milepost | 198 | to | 199 | 2 | 64.70 | 0.00 | 0.10 | 2 | 62.29 | 0.10 | 0.09 | 3.91 | 4.88 | 3.95 | 4.93 | 4.20 | 4.24 |  | 0 | 0 |
|  |  |  | Total | 10 |  |  |  | 10 |  |  |  |  |  |  |  |  |  |  |  | 0 |
| Weighted Average |  |  |  |  |  |  |  |  |  |  |  | 3.87 | 4.88 | 3.83 | 4.92 | 4.17 | 4.16 |  |  |  |
| Factor |  |  |  |  |  |  |  |  |  |  |  | 1.00 |  | 1.00 |  |  |  |  |  |  |
| Indicator Score |  |  |  |  |  |  |  |  |  |  |  | 3.87 |  | 3.83 |  |  |  |  |  | 0.0\% |
| Pavement Index |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 4.17 |  |  |

## Aロロт

Bridge Performance Area Data

| Structure Name (A209) | Structure \# (N8) | Milepost <br> (A232) | Area (A225) | Bridge <br> Sufficiency <br> Sufficiency <br> Rating | Bridge Index |  |  |  |  | Functionally Obsolete Bridges Deck Area on Func Obsolete | Bridge Rating | Hot Spots on Bridge Index map |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | $\begin{aligned} & \text { Deck } \\ & \text { (N58) } \end{aligned}$ | Sub (N59) | $\begin{aligned} & \hline \text { Super } \\ & \text { (N60) } \end{aligned}$ | Eval (N67) | Lowest |  |  |  |
| Segment 1 |  |  |  |  |  |  |  |  |  |  |  |  |
| Moffet Wash Bridge | 297 | 6.44 | 2592 | 87.80 | 6.00 | 7.00 | 6.00 | 6.00 | 6.0 | 0 |  |  |
| Total |  |  | 2,592 |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | 87.80 |  |  |  |  | 6.00 | 0.00\% |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | 87.80 |  |  |  |  |  | 0.00\% | 6 |  |
| Bridge Index |  |  |  |  |  |  |  |  | 6.00 |  |  |  |
| Segment 2 |  |  |  |  |  |  |  |  |  |  |  |  |
| Bridge | 291 | 45.46 | 1892 | 80.10 | 6.00 | 6.00 | 6.00 | 6.00 | 6.0 | 0 |  |  |
| Cochise UPRR OP | 157 | 62.88 | 3302 | 63.00 | 5.00 | 6.00 | 5.00 | 5.00 | 5.0 | 0 |  |  |
| Total |  |  | 5,194 |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | 69.23 |  |  |  |  | 5.36 | 0.00\% |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | 69.23 |  |  |  |  |  | 0.00\% | 5 |  |
| Bridge Index |  |  |  |  |  |  |  |  | 5.36 |  |  |  |
| Segment 3 |  |  |  |  |  |  |  |  |  |  |  |  |
| Monk Draw Bridge SB | 292 | 89.28 | 3584 | 87.60 | 6.00 | 6.00 | 7.00 | 5.00 | 5.0 | 0 |  |  |
| Monk Draw Br NB | 2572 | 89.29 | 3718 | 99.80 | 6.00 | 6.00 | 7.00 | 6.00 | 6.0 | 0 |  |  |
| Total |  |  | 7,302 |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | 93.81 |  |  |  |  | 5.51 | 0.00\% |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | 93.81 |  |  |  |  |  | 0.00\% | 5 |  |
| Bridge Index |  |  |  |  |  |  |  |  | 5.51 |  |  |  |
| Segment 4 |  |  |  |  |  |  |  |  |  |  |  |  |
| Stockton Wash Bridge | 201 | 111.11 | 6514 | 69.50 | 6.00 | 6.00 | 7.00 | 6.00 | 6.0 | 0 |  |  |
| Total |  |  | 6,514 |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | 69.50 |  |  |  |  | 6.00 | 0.00\% |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | 69.50 |  |  |  |  |  | 0.00\% | 6 |  |
| Bridge Index |  |  |  |  |  |  |  |  | 6.00 |  |  |  |

## Aロロт

| Structure Name (A209) | Structure \# (N8) | Milepost <br> (A232) | Area (A225) | BridgeSufficiency | Bridge Index |  |  |  |  | Functionally Obsolete Bridges <br> Deck Area on Func Obsolete | Bridge Rating | Hot Spots on Bridge Index map |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | $\begin{aligned} & \hline \text { Deck } \\ & \text { (N58) } \end{aligned}$ | Sub (N59) | $\begin{aligned} & \hline \text { Super } \\ & \text { (N60) } \end{aligned}$ | Eval (N67) | Lowest |  |  |  |
| Segment 5 |  |  |  |  |  |  |  |  |  |  |  |  |
| \#N/A | - | \#N/A | \#N/A | \#N/A | \#N/A | \#N/A | \#N/A | \#N/A | \#N/A | \#N/A |  |  |
| Total |  |  | \#N/A |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | \#N/A |  |  |  |  | \#N/A | \#N/A |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | \#N/A |  |  |  |  |  | \#N/A | \#N/A |  |
| Bridge Index |  |  |  |  |  |  |  |  | \#N/A |  |  |  |
| Segment 6 |  |  |  |  |  |  |  |  |  |  |  |  |
| Cottonwood Wash Br | 305 | 330.14 | 10647 | 68.10 | 6.00 | 6.00 | 6.00 | 6.00 | 6.0 | 0 |  |  |
| Total |  |  | 10,647 |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | 68.10 |  |  |  |  | 6.00 | 0.00\% |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | 68.10 |  |  |  |  |  | 0.00\% | 6 |  |
| Bridge Index |  |  |  |  |  |  |  |  | 6.00 |  |  |  |
| Segment 7 |  |  |  |  |  |  |  |  |  |  |  |  |
| Goodwin Wash Bridge | 2736 | 301.87 | 8245 | 80.00 | 7.00 | 8.00 | 8.00 | 8.00 | 7.0 | 0 |  |  |
| Holyoak Wash Bridge | 514 | 302.53 | 4185 | 66.70 | 5.00 | 5.00 | 5.00 | 5.00 | 5.0 | 0 |  |  |
| Fine Wash Bridge | 515 | 304.85 | 5040 | 78.70 | 6.00 | 6.00 | 6.00 | 6.00 | 6.0 | 0 |  |  |
| Ft Thomas Ped UP | 560 | 306.59 | 566 | -2.00 | 7.00 | 6.00 | 7.00 | N | 6.0 | 0 |  |  |
| Black Rock Wash Br | 545 | 306.76 | 9522 | 67.00 | 6.00 | 6.00 | 5.00 | 5.00 | 5.0 | 0 |  |  |
| Hunzinger Wash Br | 561 | 313.62 | 3715 | 69.00 | 6.00 | 6.00 | 5.00 | 5.00 | 5.0 | 0 |  |  |
| Matthewsville Wash Br | 394 | 326.25 | 7740 | 66.80 | 6.00 | 6.00 | 6.00 | 6.00 | 6.0 | 0 |  |  |
| Patterson Wash Br | 1421 | 327.72 | 1118 | 66.00 | 6.00 | 6.00 | 5.00 | 5.00 | 5.0 | 0 |  |  |
| Total |  |  | 40,131 |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | 70.25 |  |  |  |  | 5.74 | 0.00\% |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | 70.25 |  |  |  |  |  | 0.00\% | 5 |  |
| Bridge Index |  |  |  |  |  |  |  |  | 5.74 |  |  |  |


| Structure Name (A209) | Structure \# (N8) | Milepost <br> (A232) | Area (A225) | Bridge Sufficiency <br> Sufficiency Rating | Bridge Index |  |  |  |  | Functionally Obsolete Bridges Deck Area on Func Obsolete | Bridge Rating | Hot Spots on Bridge Index map |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | $\begin{aligned} & \hline \text { Deck } \\ & \text { (N58) } \end{aligned}$ | Sub (N59) | $\begin{aligned} & \text { Super } \\ & \text { (N60) } \\ & \hline \end{aligned}$ | Eval (N67) | Lowest |  |  |  |
| Segment 8 |  |  |  |  |  |  |  |  |  |  |  |  |
| Bridge | 513 | 299.51 | 4850 | 73.00 | 6.00 | 6.00 | 7.00 | 6.00 | 6.0 | 0 |  |  |
| Total |  |  | 4,850 |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | 73.00 |  |  |  |  | 6.00 | 0.00\% |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | 73.00 |  |  |  |  |  | 0.00\% | 6 |  |
| Bridge Index |  |  |  |  |  |  |  |  | 6.00 |  |  |  |
| Segment 9 |  |  |  |  |  |  |  |  |  |  |  |  |
| \#N/A |  | \#N/A | \#N/A | \#N/A | \#N/A | \#N/A | \#N/A | \#N/A | \#N/A | \#N/A |  |  |
| Total |  |  | \#N/A |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | \#N/A |  |  |  |  | \#N/A | \#N/A |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | \#N/A |  |  |  |  |  | \#N/A | \#N/A |  |
| Bridge Index |  |  |  |  |  |  |  |  | \#N/A |  |  |  |
| Segment 10 |  |  |  |  |  |  |  |  |  |  |  |  |
| Gila River Br Bylas | 2945 | 292.55 | 102258 | 80.00 | 7.00 | 8.00 | 8.00 | 8.00 | 7.0 | 0 |  |  |
| Total |  |  | 102,258 |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | 80.00 |  |  |  |  | 7.00 | 0.00\% |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | 80.00 |  |  |  |  |  | 0.00\% | 7 |  |
| Bridge Index |  |  |  |  |  |  |  |  | 7.00 |  |  |  |
| Segment 11 |  |  |  |  |  |  |  |  |  |  |  |  |
| Peridot RR OP | 477 | 271.27 | 9225 | 93.20 | 5.00 | 6.00 | 6.00 | 6.00 | 5.0 | 0 |  |  |
| San Carlos River Bridge | 2910 | 271.56 | 51095 | 80.00 | 7.00 | 8.00 | 8.00 | 8.00 | 7.0 | 0 |  |  |
| Total |  |  | 60,320 |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | 82.02 |  |  |  |  | 6.69 | 0.00\% |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | 82.02 |  |  |  |  |  | 0.00\% | 5 |  |
| Bridge Index |  |  |  |  |  |  |  |  | 6.69 |  |  |  |
| Segment 12 |  |  |  |  |  |  |  |  |  |  |  |  |
| Gilson Wash Br | 464 | 259.55 | 7599 | 52.90 | 6.00 | 6.00 | 6.00 | 6.00 | 6.0 | 0 |  |  |
| Total |  |  | 7,599 |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | 52.90 |  |  |  |  | 6.00 | 0.00\% |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | 52.90 |  |  |  |  |  | 0.00\% | 6 |  |
| Bridge Index |  |  |  |  |  |  |  |  | 6.00 |  |  |  |


| Structure Name (A209) | Structure \# <br> (N8) | Milepost (A232) | Area (A225) | BridgeSufficiency | Bridge Index |  |  |  |  | Functionally Obsolete Bridges Deck Area on Func Obsolete | Bridge Rating | Hot Spots on Bridge Index map |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | $\begin{aligned} & \hline \text { Deck } \\ & \text { (N58) } \end{aligned}$ | Sub (N59) | $\begin{aligned} & \text { Super } \\ & \text { (N60) } \end{aligned}$ | Eval (N67) | Lowest |  |  |  |
| Segment 13 |  |  |  |  |  |  |  |  |  |  |  |  |
| Bloody Tanks Bridge | 173 | 243.71 | 6294 | 77.20 | 5.00 | 5.00 | 6.00 | 5.00 | 5.0 | 0 |  |  |
| Pinal Creek Bridge | 266 | 249.64 | 9963 | 44.90 | 4.00 | 4.00 | 5.00 | 4.00 | 4.0 | 0 |  |  |
| Pinal Creek Bridge | 36 | 249.80 | 7558 | 53.50 | 5.00 | 5.00 | 6.00 | 5.00 | 5.0 | 0 |  |  |
| Central Sch Ped OP | 1788 | 250.34 | 3380 | -2.00 | 7.00 | 8.00 | 7.00 | 7.00 | 7.0 | 0 |  |  |
| Pinal Creek Bridge | 549 | 250.37 | 14382 | 78.30 | 5.00 | 5.00 | 6.00 | 5.00 | 5.0 | 0 |  |  |
| Pinal Creek Bridge | 1785 | 250.53 | 17453 | 94.60 | 6.00 | 6.00 | 7.00 | 6.00 | 6.0 | 0 |  |  |
| Maple Street OP | 1786 | 250.75 | 7038 | 94.60 | 6.00 | 7.00 | 7.00 | 7.00 | 6.0 | 0 |  |  |
| Globe Viaduct | 1787 | 250.90 | 71113 | 85.10 | 5.00 | 6.00 | 7.00 | 6.00 | 5.0 | 0 |  |  |
| Globe School Ped OP | 488 | 251.27 | 774 | -2.00 | 7.00 | 7.00 | 7.00 | 7.00 | 7.0 | 0 |  |  |
| McMillen Wash Br | 1028 | 251.75 | 7605 | 81.40 | 5.00 | 5.00 | 5.00 | 5.00 | 5.0 | 0 |  |  |
| Pinal SPRR UP | 562 | 253.63 | 1107 | -2.00 | N | 6.00 | 7.00 | N | 6.0 | 0 |  |  |
| Total |  |  | 146,667 |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | 78.01 |  |  |  |  | 5.16 | 0.00\% |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | 78.01 |  |  |  |  |  | 0.00\% | 4 |  |
| Bridge Index |  |  |  |  |  |  |  |  | 5.16 |  |  |  |
| Segment 14 |  |  |  |  |  |  |  |  |  |  |  |  |
| Queen Creek Bridge | 406 | 227.71 | 19618 | 19.40 | 4.00 | 3.00 | 4.00 | 3.00 | 3.0 | 0 |  |  |
| Queen Creek Tunnel | 407 | 228.47 | 4491 | -2.00 | N | N | N | N |  |  |  |  |
| Waterfall Canyon Br | 328 | 229.50 | 4176 | 40.30 | 6.00 | 5.00 | 4.00 | 4.00 | 4.0 | 0 |  |  |
| Devils Canyon Bridge | 261 | 232.49 | 2035 | 81.60 | 6.00 | 6.00 | 6.00 | 6.00 | 6.0 | 0 |  |  |
| Pinto Creek Bridge | 20077 | 238.25 | 32793 | 100.00 | 8.00 | 8.00 | 8.00 | 8.00 | 8.0 | 0 |  |  |
| Bloody Tanks Wash Br | 45 | 242.72 | 2812 | 68.00 | 6.00 | 6.00 | 5.00 | 5.00 | 5.0 | 0 |  |  |
| Total |  |  | 65,925 |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | 68.13 |  |  |  |  | 5.52 | 0.00\% |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | 68.13 |  |  |  |  |  | 0.00\% | 3 |  |
| Bridge Index |  |  |  |  |  |  |  |  | 5.52 |  |  |  |

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| Structure Name (A209) | Structure \# (N8) | Milepost <br> (A232) | Area (A225) | Bridge Sufficiency <br> Sufficiency Rating | Bridge Index |  |  |  |  | Functionally Obsolete Bridges Deck Area on Func Obsolete | Bridge Rating | Hot Spots on Bridge Index map |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | $\begin{aligned} & \hline \text { Deck } \\ & \text { (N58) } \\ & \hline \end{aligned}$ | Sub (N59) | $\begin{aligned} & \text { Super } \\ & \text { (N60) } \\ & \hline \end{aligned}$ | Eval (N67) | Lowest |  |  |  |
| Segment 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| Queen Creek Bridge | 436 | 226.14 | 12936 | 77.00 | 6.00 | 6.00 | 7.00 | 6.00 | 6.0 | 0 |  |  |
| Stone Ave OP | 20113 | 226.62 | 8400 | 91.50 | 7.00 | 7.00 | 8.00 | 7.00 | 7.0 | 0 |  |  |
| Route 177 TI UP | 438 | 226.85 | 5099 | 89.80 | 6.00 | 6.00 | 7.00 | 6.00 | 6.0 | 0 |  |  |
| Total |  |  | 26,435 |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | 84.08 |  |  |  |  | 6.32 | 0.00\% |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | 84.08 |  |  |  |  |  | 0.00\% | 6 |  |
| Bridge Index |  |  |  |  |  |  |  |  | 6.32 |  |  |  |
| Segment 16 |  |  |  |  |  |  |  |  |  |  |  |  |
| Silver King Wash Br | 318 | 223.70 | 3432 | 86.30 | 6.00 | 7.00 | 7.00 | 5.00 | 5.0 | 0 |  |  |
| No Name Wash Br WB | 319 | 224.64 | 1568 | 86.70 | 6.00 | 6.00 | 6.00 | 5.00 | 5.0 | 0 |  |  |
| Total |  |  | 5,000 |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | 86.43 |  |  |  |  | 5.00 | 0.00\% |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | 86.43 |  |  |  |  |  | 0.00\% | 5 |  |
| Bridge Index |  |  |  |  |  |  |  |  | 5.00 |  |  |  |
| Segment 17 |  |  |  |  |  |  |  |  |  |  |  |  |
| US 60 EB OP Bridge | 2663 | 212.25 | 9094 | 97.40 | 7.00 | 7.00 | 7.00 | 7.00 | 7.0 | 0 |  |  |
| US 60 WB OP Bridge | 2664 | 212.25 | 9094 | 97.00 | 6.00 | 7.00 | 7.00 | 7.00 | 6.0 | 0 |  |  |
| Reymert Wash Bridge EB | 286 | 219.85 | 1892 | 87.20 | 7.00 | 7.00 | 6.00 | 5.00 | 5.0 | 0 |  |  |
| Reymert Wash Bridge WB | 2846 | 220.00 | 3624 | 97.20 | 7.00 | 8.00 | 8.00 | 8.00 | 7.0 | 0 |  |  |
| Queen Creek Bridge EB | 2847 | 222.25 | 12647 | 98.20 | 7.00 | 8.00 | 7.00 | 7.00 | 7.0 | 0 |  |  |
| Queen Creek Br WB | 20029 | 222.25 | 9900 | 97.20 | 7.00 | 8.00 | 8.00 | 8.00 | 7.0 | 0 |  |  |
| Wash Bridge | 288 | 222.87 | 2322 | 65.80 | 6.00 | 6.00 | 6.00 | 5.00 | 5.0 | 0 |  |  |
| Total |  |  | 48,573 |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | 95.57 |  |  |  |  | 6.64 | 0.00\% |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | 95.57 |  |  |  |  |  | 0.00\% | 5 |  |
| Bridge Index |  |  |  |  |  |  |  |  | 6.64 |  |  |  |


| Structure Name (A209) | Structure \# (N8) | Milepost <br> (A232) | Area (A225) | BridgeSufficiency | Bridge Index |  |  |  |  | Functionally Obsolete Bridges Deck Area on Func Obsolete | Bridge Rating | Hot Spots on Bridge Index map |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | $\begin{aligned} & \hline \text { Deck } \\ & \text { (N58) } \end{aligned}$ | Sub (N59) | $\begin{aligned} & \hline \text { Super } \\ & \text { (N60) } \end{aligned}$ | Eval (N67) | Lowest |  |  |  |
| Segment 18 |  |  |  |  |  |  |  |  |  |  |  |  |
| Queen Creek Br EB | 2056 | 210.83 | 13424 | 98.90 | 7.00 | 7.00 | 7.00 | 7.00 | 7.0 | 0 |  |  |
| Queen Creek Br WB | 841 | 210.83 | 10630 | 79.40 | 5.00 | 7.00 | 7.00 | 7.00 | 5.0 | 0 |  |  |
| Sand Tanks Wsh Br EB | 435 | 208.75 | 3942 | 79.30 | 5.00 | 5.00 | 6.00 | 5.00 | 5.0 | 0 |  |  |
| N/A - No Bridges | 7178 |  |  |  |  |  |  |  |  |  |  |  |
| Bridge EB | 578 | 207.98 | 3942 | 97.40 | 6.00 | 6.00 | 6.00 | 6.00 | 6.0 | 0 |  |  |
| Bridge WB | 857 | 207.98 | 3942 | 85.30 | 5.00 | 5.00 | 6.00 | 5.00 | 5.0 | 0 |  |  |
| Bridge EB | 1533 | 206.08 | 781 | 89.50 | 6.00 | 6.00 | 7.00 | 6.00 | 6.0 | 0 |  |  |
| Bridge over Wash EB | 434 | 205.37 | 4928 | 90.10 | 6.00 | 6.00 | 6.00 | 6.00 | 6.0 | 0 |  |  |
| Bridge over Wash WB | 856 | 205.37 | 4844 | 97.40 | 6.00 | 6.00 | 7.00 | 6.00 | 6.0 | 0 |  |  |
| Total |  |  | 46,433 |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | 90.24 |  |  |  |  | 5.89 | 0.00\% |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | 90.24 |  |  |  |  |  | 0.00\% | 5 |  |
| Bridge Index |  |  |  |  |  |  |  |  | 5.89 |  |  |  |
| Segment 19 |  |  |  |  |  |  |  |  |  |  |  |  |
| Bridge EB | 433 | 203.09 | 1792 | 87.60 | 6.00 | 6.00 | 6.00 | 5.00 | 5.0 | 0 |  |  |
| Bridge WB | 855 | 203.09 | 1798 | 97.20 | 6.00 | 6.00 | 7.00 | 6.00 | 6.0 | 0 |  |  |
| Bridge EB | 432 | 200.58 | 2688 | 91.20 | 6.00 | 6.00 | 6.00 | 6.00 | 6.0 | 0 |  |  |
| Bridge WB | 799 | 200.58 | 2910 | 94.10 | 6.00 | 6.00 | 7.00 | 6.00 | 6.0 | 0 |  |  |
| Siphon Draw Br EB | 2199 | 199.12 | 9085 | 96.20 | 7.00 | 7.00 | 7.00 | 7.00 | 7.0 | 0 |  |  |
| Siphon Draw Br WB | 2200 | 199.12 | 9085 | 85.50 | 7.00 | 5.00 | 7.00 | 5.00 | 5.0 | 0 |  |  |
| Total |  |  | 27,358 |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | 91.43 |  |  |  |  | 5.93 | 0.00\% |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | 91.43 |  |  |  |  |  | 0.00\% | 5 |  |
| Bridge Index |  |  |  |  |  |  |  |  | 5.93 |  |  |  |

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| Structure Name (A209) | Structure \# <br> (N8) | Milepost <br> (A232) | Area (A225) | Bridge Sufficiency <br> Sufficiency Rating | Bridge Index |  |  |  |  | Functionally Obsolete Bridges Deck Area on Func Obsolete | Bridge Rating | Hot Spots on Bridge Index map |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | $\begin{aligned} & \hline \text { Deck } \\ & \text { (N58) } \\ & \hline \end{aligned}$ | Sub (N59) | $\begin{aligned} & \text { Super } \\ & \text { (N60) } \end{aligned}$ | Eval (N67) | Lowest |  |  |  |
| Segment 20 |  |  |  |  |  |  |  |  |  |  |  |  |
| Goldfield Road TI OP | 2068 | 198.40 | 19150 | 93.80 | 7.00 | 6.00 | 7.00 | 6.00 | 6.0 | 0 |  |  |
| Tomahawk Rd TI OP | 2067 | 197.41 | 19150 | 94.00 | 7.00 | 6.00 | 7.00 | 6.00 | 6.0 | 0 |  |  |
| Idaho Rd TI OP | 2066 | 196.41 | 19150 | 94.00 | 7.00 | 6.00 | 7.00 | 6.00 | 6.0 | 0 |  |  |
| Ironwood Dr TI OP | 2065 | 195.39 | 19998 | 94.00 | 7.00 | 6.00 | 6.00 | 6.00 | 6.0 | 0 |  |  |
| \#N/A | null |  |  |  |  |  |  |  |  |  |  |  |
| N/A - No Bridges | 7327 |  |  |  |  |  |  |  |  |  |  |  |
| \#N/A | null |  |  |  |  |  |  |  |  |  |  |  |
| Tota |  |  | 77,448 |  |  |  |  |  |  |  |  |  |
| Weighted Average |  |  |  | 93.95 |  |  |  |  | 6.00 | 0.00\% |  |  |
| Factor |  |  |  | 1.00 |  |  |  |  | 1.00 | 1.00 |  |  |
| Indicator Score |  |  |  | 93.95 |  |  |  |  |  | 0.00\% | 6 |  |
| Bridge Index |  |  |  |  |  |  |  |  | 6.00 |  |  |  |

## Mobility Performance Area Data

|  |  | $\sum_{\substack{0 \\ \hline \mathbf{D}}}$ |  |  | $\begin{aligned} & \stackrel{0}{2} \\ & \text { 2 } \\ & \text { z } \\ & \text { 은 } \end{aligned}$ |  |  |  |  |  | $\begin{aligned} & \text { EB/SB Right Shoulder } \\ & \text { Width } \end{aligned}$ |  | ЧІр!М גәрınoчs भәา gs/gヨ | $\begin{aligned} & \text { Lo } \\ & \frac{1}{k} \\ & \infty \\ & \frac{\infty}{\infty} \end{aligned}$ |  |  | $\begin{aligned} & \stackrel{\vdots}{U} \\ & \stackrel{\pi}{4} \\ & \stackrel{4}{4} \end{aligned}$ | $\begin{aligned} & \text { 믐 } \\ & \stackrel{\sim}{\circ} \\ & 0 \end{aligned}$ |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 191-1 | 0 | 24 | 24 | Rural | Interrupted | Level | 2 | Urban/Rural Single or Multilane Signalized | 12.00 | 6.78 | 6.81 | N/A | N/A | 1040 | 1053 | 2093 | 0.10 | 0.53 | 0.15 | 55 | Undivided | 7 | 12\% |
| 191-2 | 24 | 67 | 43 | Rural | Interrupted | Level | 2 | Rural TwoLane, NonSignalized | 12.00 | 2.32 | 2.37 | N/A | N/A | 628 | 906 | 1534 | 0.11 | 0.62 | 0.27 | 55 | Undivided | 8 | 26\% |
| 191-3 | 87 | 104 | 17 | Rural | Uninterrupted | Level | 4 | Multilane Highway | 12.00 | 10.29 |  |  | 4.05 | 1316 | 1301 | 2617 | 0.07 | 0.50 | 0.21 | 55 | Divided | 2 | 3\% |
| 191-4 | 104 | 116 | 12 | Rural | Uninterrupted | Level | 4 | Rural TwoLane, NonSignalized | 12.00 | 7.93 | 7.81 | N/A | N/A | 2184 | 2159 | 4343 | 0.08 | 0.50 | 0.16 | 65 | Undivided | 13 | 30\% |
| 191-5 | 116 | 121 | 5 | Urba n | Interrupted | Level | 4 | Urban/Rural Single or Multilane Signalized | 12.00 | 2.97 | 2.29 | N/A | N/A | 3828 | 4075 | 7903 | 0.07 | 0.56 | 0.10 | 40 | Undivided | N/A | 13\% |
| 70-6 | 330 | 339 | 9 | Urba n | Interrupted | Level | 4 | Urban/Rural Single or Multilane Signalized | 12.00 | 2.89 | 2.82 | N/A | N/A | 5888 | 5665 | 11553 | 0.09 | 0.51 | 0.13 | 40 | Undivided | N/A | 0\% |
| 70-7 | 300 | 330 | 30 | Rural | Uninterrupted | Level | 2 | Rural TwoLane, NonSignalized | 12.00 | 6.68 | 6.67 | N/A | N/A | 1671 | 1446 | 3116 | 0.07 | 0.54 | 0.27 | 55 | Undivided | 9 | 13\% |
| 70-8 | 298 | 300 | 2 | Rural | Uninterrupted | Level | 2 | Rural TwoLane, NonSignalized | 12.00 | 5.00 | 5.00 | N/A | N/A | 1599 | 1150 | 2749 | 0.07 | 0.58 | 0.25 | 65 | Undivided | 7 | 6\% |
| 70-9 | 293 | 298 | 5 | Rural | Uninterrupted | Level | 2 | Rural TwoLane, NonSignalized | 12.00 | 5.82 | 5.92 | N/A | N/A | 1599 | 1150 | 2749 | 0.07 | 0.58 | 0.25 | 50 | Undivided | 14 | 53\% |
| 70-10 | 274 | 293 | 19 | Rural | Uninterrupted | Level | 2 | Rural TwoLane, NonSignalized | 12.00 | 5.07 | 5.08 | N/A | N/A | 1599 | 1150 | 2749 | 0.07 | 0.58 | 0.25 | 55 | Undivided | 2 | 37\% |

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|  | $\sum_{\substack{c \\ 0 \\ \infty \\ \infty}}^{\infty}$ | $\sum_{\text {O }}^{0}$ <br> © |  |  | $\begin{aligned} & \text { zo } \\ & \frac{0}{4} \\ & \\ & \end{aligned}$ |  | $\begin{aligned} & \text { ¿o } \\ & \dot{\text { B }} \end{aligned}$ |  | $\begin{aligned} & \stackrel{5}{5} \\ & \frac{5}{3} \end{aligned}$ |  |  |  |  | $\sum_{\substack{\infty}}^{\frac{1}{2}}$ | $\begin{aligned} & \text { 邑占 } \\ & \text { © } \end{aligned}$ |  | $\begin{aligned} & \stackrel{\vdots}{4} \\ & \stackrel{4}{\sim} \\ & \underline{\sim} \end{aligned}$ |  | $\begin{aligned} & \stackrel{\vdots}{0} \\ & \stackrel{U}{\sim} \\ & \stackrel{\sim}{1} \end{aligned}$ |  |  |  | ì |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 70－11 | 270 | 274 | 4 | Rural | Uninterrupted | Level | 2 | Rural Two－ Lane，Non－ Signalized | 12.00 | 4.54 | 4.70 | N／A | N／A | 1638 | 1213 | 2850 | 0.07 | 0.58 | 0.25 | 55 | Undivided | 9 | 77\％ |
| 70－12 | 255 | 270 | 15 | Rural | Uninterrupted | Level | 2 | Rural Two－ Lane，Non－ Signalized | 12.00 | 5.43 | 5.33 | N／A | N／A | 2774 | 3016 | 5790 | 0.08 | 0.52 | 0.17 | 60 | Undivided | 4 | 10\％ |
| $\begin{gathered} 70 / 60 \\ \mathrm{E}-13 \end{gathered}$ | 243 | 255 | 12 | Urban | Interrupted | Level | 4 | Urban／Rural Single or Multilane Signalized | 12.00 | 5.04 | 4.15 | N／A | N／A | 5623 | 5520 | 11143 | 0.08 | 0.52 | 0.11 | 45 | Undivided | N／A | 0\％ |
| $\begin{gathered} 60 \mathrm{E}- \\ 14 \end{gathered}$ | 227 | 243 | 16 | Rural | Uninterrupted | Mou <br> ntain <br> ous | 2 | Rural Two－ Lane，Non－ Signalized | 12.00 | 4.86 | 4.78 | N／A | N／A | 4152 | 5976 | 10128 | 0.09 | 0.59 | 0.11 | 50 | Undivided | 4 | 68\％ |
| $\begin{gathered} 60 \mathrm{E}- \\ 15 \\ \hline \end{gathered}$ | 225 | 227 | 2 | Rural | Uninterrupted | $\begin{gathered} \hline \text { Rolli } \\ \text { ng } \\ \hline \end{gathered}$ | 4 | Multilane Highway | 12.00 | 7.88 | 7.87 | 7.87 | N／A | 3763 | 3726 | 7489 | 0.07 | 0.51 | 0.18 | 45 | Undivided | 23 | 98\％ |
| $\begin{gathered} 60 \mathrm{E}- \\ 16 \end{gathered}$ | 223 | 225 | 2 | Rural | Uninterrupted | Level | 4 | Multilane Highway | 12.00 | 7.75 | 7.25 | 7.25 | N／A | 5338 | 5355 | 10693 | 0.10 | 0.50 | 0.13 | 55 | Undivided | 0.3 | 55\％ |
| $\begin{gathered} 60 \mathrm{E}- \\ 17 \\ \hline \end{gathered}$ | 212 | 223 | 11 | Rural | Uninterrupted | Level | 4 | Multilane Highway | 12.00 | 10.12 | 10.00 | 10.00 | 4.78 | 5537 | 5511 | 11048 | 0.10 | 0.50 | 0.13 | 65 | Divided | 2 | 0\％ |
| $\begin{gathered} 60 \mathrm{E}- \\ 18 \end{gathered}$ | 205 | 212 | 7 | Rural | Uninterrupted | Level | 2 | Multilane Highway | 12.00 | 6.57 | 9.59 | 9.59 | 4.00 | 6755 | 7248 | 14003 | 0.08 | 0.52 | 0.13 | 65 | Divided | 2 | 0\％ |
| $\begin{gathered} 60 \mathrm{E}- \\ 19 \end{gathered}$ | 199 | 205 | 6 | Urban | Interrupted | Level | 2 | Urban／Rural Single or Multilane Signalized | 12.00 | 6.00 | 9.05 | N／A | N／A | 9049 | 9603 | 18653 | 0.08 | 0.53 | 0.11 | 55 | Divided | N／A | 0\％ |
| $\begin{gathered} 60 \mathrm{E}- \\ 20 \end{gathered}$ | $\begin{gathered} 194 . \\ 3 \end{gathered}$ | 199 | $\begin{gathered} \hline 4 . \\ 7 \\ \hline \end{gathered}$ | Urban | Uninterrupted | Level | 2 | Freeway Segment | 12.00 | 10.00 | 10.00 | N／A | N／A | 23754 | 24754 | 48507 | 0.08 | 0.51 | 0.09 | 65 | Divided | N／A | 0\％ |

## LOTTR and TTTR - Northbound/Westbound



| $\begin{aligned} & \stackrel{\rightharpoonup}{\mathbf{0}} \\ & \stackrel{y}{0} \\ & \stackrel{4}{0} \\ & \dot{\sim} \end{aligned}$ | $\sum_{i}^{\cup}$ |  | $\dot{\circ}$ $\mathbf{2}$ $\mathbf{0}$ $\mathbf{0}$ $\boldsymbol{\sim}$ |  | $\frac{\boldsymbol{y}}{\dot{\Sigma}}$ |  |  |  |  |  |  |  | $\underset{\sim}{\mathbf{N}}$ |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 115-05668 | 2 Mid Day | US-60 | S | 1.43 | 103 | 105 | 112 | 143 | 55 | 55 | 55 | 1.09 | 1.36 |  |  |  |  |  |
|  | 115-05668 | 3 PM Peak | US-60 | S | 1.43 | 100 | 103 | 109 | 135 | 55 | 55 | 55 | 1.09 | 1.32 |  |  |  |  |  |
|  | 115-05668 | 4 Weekend | US-60 | S | 1.43 | 101 | 105 | 111 | 135 | 55 | 55 | 55 | 1.10 | 1.29 |  |  |  |  |  |
|  | 115 N11100 | 1 AM Peak | US-60 | W | 0.23 | 17 | 17 | 19 | 23 | 55 | 55 | 55 | 1.15 | 1.35 | 1.15 | 1.35 | 2\% |  |  |
|  | 115 N 11100 | 2 Mid Day | US-60 | W | 0.23 | 16 | 17 | 18 | 23 | 55 | 55 | 55 | 1.09 | 1.35 |  |  |  |  |  |
|  | 115 N 11100 | 3 PM Peak | US-60 | W | 0.23 | 16 | 16 | 17 | 21 | 55 | 55 | 55 | 1.09 | 1.31 |  |  |  |  |  |
|  | 115 N 11100 | 4 Weekend | US-60 | W | 0.23 | 16 | 17 | 18 | 22 | 55 | 55 | 55 | 1.11 | 1.32 |  |  |  |  |  |
|  | 115-11100 | 1 AM Peak | US-60 | W | 1.92 | 137 | 137 | 161 | 203 | 55 | 55 | 55 | 1.17 | 1.48 | 1.17 | 1.62 | 16\% |  |  |
|  | 115-11100 | 2 Mid Day | US-60 | W | 1.92 | 134 | 136 | 149 | 210 | 55 | 55 | 55 | 1.11 | 1.54 |  |  |  |  |  |
|  | 115-11100 | 3 PM Peak | US-60 | W | 1.92 | 132 | 136 | 144 | 210 | 55 | 55 | 55 | 1.09 | 1.55 |  |  |  |  |  |
|  | 115-11100 | 4 Weekend | US-60 | W | 1.92 | 133 | 138 | 150 | 224 | 55 | 55 | 55 | 1.13 | 1.62 |  |  |  |  |  |
|  | 115-05669 | 1 AM Peak | US-60 | W | 4.23 | 280 | 281 | 308 | 363 | 55 | 55 | 55 | 1.10 | 1.29 | 1.10 | 1.30 | 35\% |  |  |
|  | 115-05669 | 2 Mid Day | US-60 | W | 4.23 | 277 | 279 | 299 | 363 | 55 | 55 | 55 | 1.08 | 1.30 |  |  |  |  |  |
|  | 115-05669 | 3 PM Peak | US-60 | W | 4.23 | 277 | 280 | 299 | 346 | 55 | 55 | 55 | 1.08 | 1.24 |  |  |  |  |  |
|  | 115-05669 | 4 Weekend | US-60 | W | 4.23 | 277 | 287 | 305 | 354 | 55 | 55 | 55 | 1.10 | 1.23 |  |  |  |  |  |
|  | 115-05671 | 1 AM Peak | US-60 | W | 4.32 | 297 | 299 | 331 | 458 | 55 | 55 | 55 | 1.11 | 1.53 | 1.13 | 1.76 | 36\% |  |  |
|  | 115-05671 | 2 Mid Day | US-60 | W | 4.32 | 294 | 296 | 325 | 521 | 55 | 55 | 55 | 1.11 | 1.76 |  |  |  |  |  |
|  | 115-05671 | 3 PM Peak | US-60 | W | 4.32 | 289 | 294 | 317 | 389 | 55 | 55 | 55 | 1.10 | 1.32 |  |  |  |  |  |
|  | 115-05671 | 4 Weekend | US-60 | W | 4.32 | 294 | 311 | 331 | 387 | 55 | 55 | 55 | 1.13 | 1.24 |  |  |  |  |  |
|  | 115+11100 | 1 AM Peak | US-60 | N | 1.43 | 130 | 149 | 156 | 206 | 50 | 50 | 50 | 1.20 | 1.38 | 1.38 | 1.60 | 34\% | 1.18 | 1.34 |
|  | 115+11100 | 2 Mid Day | US-60 | N | 1.43 | 137 | 151 | 171 | 214 | 50 | 50 | 50 | 1.24 | 1.42 |  |  |  |  |  |
|  | 115+11100 | 3 PM Peak | US-60 | N | 1.43 | 142 | 156 | 169 | 214 | 50 | 50 | 50 | 1.19 | 1.37 |  |  |  |  |  |
|  | 115+11100 | 4 Weekend | US-60 | N | 1.43 | 115 | 134 | 159 | 214 | 50 | 50 | 50 | 1.38 | 1.60 |  |  |  |  |  |
|  | 115-05667 | 1 AM Peak | US-60 | W | 2.42 | 164 | 164 | 174 | 193 | 50 | 50 | 50 | 1.06 | 1.18 | 1.08 | 1.20 | 57\% |  |  |
| 15 | 115-05667 | 2 Mid Day | US-60 | W | 2.42 | 162 | 164 | 174 | 194 | 50 | 50 | 50 | 1.07 | 1.18 |  |  |  |  |  |
| 15 | 115-05667 | 3 PM Peak | US-60 | W | 2.42 | 161 | 164 | 173 | 197 | 50 | 50 | 50 | 1.07 | 1.20 |  |  |  |  |  |
|  | 115-05667 | 4 Weekend | US-60 | W | 2.42 | 161 | 165 | 174 | 193 | 50 | 50 | 50 | 1.08 | 1.17 |  |  |  |  |  |
|  | 115N05668 | 1 AM Peak | US-60 | W | 0.41 | 31 | 31 | 34 | 38 | 50 | 50 | 50 | 1.09 | 1.21 | 1.09 | 1.23 | 10\% |  |  |
|  | 115N05668 | 2 Mid Day | US-60 | W | 0.41 | 31 | 31 | 33 | 37 | 50 | 50 | 50 | 1.07 | 1.20 |  |  |  |  |  |
|  | 115N05668 | 3 PM Peak | US-60 | W | 0.41 | 30 | 31 | 32 | 38 | 50 | 50 | 50 | 1.06 | 1.23 |  |  |  |  |  |
|  | 115N05668 | 4 Weekend | US-60 | W | 0.41 | 31 | 32 | 33 | 36 | 50 | 50 | 50 | 1.08 | 1.15 |  |  |  |  |  |
| 16 | 115-05666 | 1 AM Peak | US-60 | W | 1.82 | 97 | 98 | 102 | 109 | 50 | 50 | 50 | 1.04 | 1.12 | 1.05 | 1.14 | 100\% | 1.05 | 1.14 |
|  | 115-05666 | 2 Mid Day | US-60 | W | 1.82 | 98 | 99 | 102 | 111 | 50 | 50 | 50 | 1.04 | 1.12 |  |  |  |  |  |
|  | 115-05666 | 3 PM Peak | US-60 | W | 1.82 | 97 | 98 | 101 | 111 | 50 | 50 | 50 | 1.04 | 1.14 |  |  |  |  |  |
|  | 115-05666 | 4 Weekend | US-60 | W | 1.82 | 98 | 101 | 102 | 111 | 50 | 50 | 50 | 1.05 | 1.10 |  |  |  |  |  |
| 17 | 115-05662 | 1 AM Peak | US-60 | W | 1.39 | 74 | 75 | 77 | 89 | 65 | 65 | 65 | 1.04 | 1.20 | 1.06 | 1.20 | 13\% | 1.05 | 1.15 |
|  | 115-05662 | 2 Mid Day | US-60 | W | 1.39 | 74 | 76 | 77 | 86 | 65 | 65 | 65 | 1.04 | 1.14 |  |  |  |  |  |


| $\begin{aligned} & \stackrel{\rightharpoonup}{\mathbf{0}} \\ & \stackrel{y}{0} \\ & \stackrel{4}{0} \\ & \dot{\sim} \end{aligned}$ | $\sum_{i}^{\cup}$ |  | $\dot{\circ}$ $\mathbf{2}$ $\mathbf{0}$ $\mathbf{0}$ $\boldsymbol{\sim}$ | $\begin{aligned} & \text { İ } \\ & \text { O} \\ & \text { O} \\ & 0 \\ & \hline \end{aligned}$ | $\frac{\mathscr{y}}{\bar{\Sigma}}$ |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 115-05662 | 3 PM Peak | US-60 | W | 1.39 | 74 | 76 | 77 | 89 | 65 | 65 | 65 | 1.05 | 1.18 |  |  |  |  |  |
|  | 115-05662 | 4 Weekend | US-60 | W | 1.39 | 74 | 77 | 78 | 86 | 65 | 65 | 65 | 1.06 | 1.12 |  |  |  |  |  |
|  | 115N05663 | 1 AM Peak | US-60 | W | 0.09 | 5 | 5 | 5 | 6 | 65 | 65 | 65 | 1.05 | 1.17 | 1.05 | 1.17 | 1\% |  |  |
|  | 115N05663 | 2 Mid Day | US-60 | W | 0.09 | 5 | 5 | 5 | 5 | 65 | 65 | 65 | 1.04 | 1.14 |  |  |  |  |  |
|  | 115N05663 | 3 PM Peak | US-60 | W | 0.09 | 5 | 5 | 5 | 5 | 65 | 65 | 65 | 1.05 | 1.16 |  |  |  |  |  |
|  | 115N05663 | 4 Weekend | US-60 | W | 0.09 | 5 | 5 | 5 | 5 | 65 | 65 | 65 | 1.05 | 1.12 |  |  |  |  |  |
|  | 115-05663 | 1 AM Peak | US-60 | W | 1.59 | 84 | 85 | 88 | 95 | 65 | 65 | 65 | 1.05 | 1.13 | 1.05 | 1.14 | 15\% |  |  |
|  | 115-05663 | 2 Mid Day | US-60 | W | 1.59 | 85 | 85 | 88 | 95 | 65 | 65 | 65 | 1.04 | 1.12 |  |  |  |  |  |
|  | 115-05663 | 3 PM Peak | US-60 | W | 1.59 | 84 | 85 | 87 | 97 | 65 | 65 | 65 | 1.04 | 1.14 |  |  |  |  |  |
|  | 115-05663 | 4 Weekend | US-60 | W | 1.59 | 84 | 87 | 88 | 97 | 65 | 65 | 65 | 1.05 | 1.11 |  |  |  |  |  |
|  | 115-05664 | 1 AM Peak | US-60 | W | 5.62 | 302 | 304 | 316 | 349 | 65 | 65 | 65 | 1.05 | 1.15 | 1.05 | 1.15 | 52\% |  |  |
|  | 115-05664 | 2 Mid Day | US-60 | W | 5.62 | 303 | 307 | 316 | 351 | 65 | 65 | 65 | 1.04 | 1.14 |  |  |  |  |  |
|  | 115-05664 | 3 PM Peak | US-60 | W | 5.62 | 298 | 304 | 312 | 349 | 65 | 65 | 65 | 1.04 | 1.15 |  |  |  |  |  |
|  | 115-05664 | 4 Weekend | US-60 | W | 5.62 | 300 | 312 | 316 | 349 | 65 | 65 | 65 | 1.05 | 1.12 |  |  |  |  |  |
|  | 115-05665 | 1 AM Peak | US-60 | W | 0.96 | 51 | 52 | 54 | 59 | 65 | 65 | 65 | 1.05 | 1.15 | 1.06 | 1.19 | 9\% |  |  |
|  | 115-05665 | 2 Mid Day | US-60 | W | 0.96 | 52 | 52 | 54 | 60 | 65 | 65 | 65 | 1.04 | 1.16 |  |  |  |  |  |
|  | 115-05665 | 3 PM Peak | US-60 | W | 0.96 | 51 | 52 | 53 | 61 | 65 | 65 | 65 | 1.05 | 1.19 |  |  |  |  |  |
|  | 115-05665 | 4 Weekend | US-60 | W | 0.96 | 51 | 53 | 55 | 60 | 65 | 65 | 65 | 1.06 | 1.14 |  |  |  |  |  |
|  | 115N05662 | 1 AM Peak | US-60 | W | 1.13 | 60 | 61 | 63 | 68 | 65 | 65 | 65 | 1.05 | 1.12 | 1.05 | 1.12 | 11\% |  |  |
|  | 115N05662 | 2 Mid Day | US-60 | W | 1.13 | 60 | 62 | 63 | 68 | 65 | 65 | 65 | 1.04 | 1.10 |  |  |  |  |  |
|  | 115N05662 | 3 PM Peak | US-60 | W | 1.13 | 60 | 62 | 63 | 69 | 65 | 65 | 65 | 1.05 | 1.12 |  |  |  |  |  |
|  | 115N05662 | 4 Weekend | US-60 | W | 1.13 | 60 | 63 | 63 | 68 | 65 | 65 | 65 | 1.05 | 1.08 |  |  |  |  |  |
| 18 | 115-05661 | 1 AM Peak |  | W | 3.44 | 186 | 185 | 197 | 203 | 65 | 65 | 65 | 1.06 | 1.10 | 1.06 | 1.10 | 45\% | 1.12 | 1.32 |
|  | 115-05661 | 2 Mid Day |  | W | 3.44 | 185 | 188 | 192 | 207 | 65 | 65 | 65 | 1.04 | 1.10 |  |  |  |  |  |
|  | 115-05661 | 3 PM Peak |  | W | 3.44 | 182 | 188 | 188 | 207 | 65 | 65 | 65 | 1.04 | 1.10 |  |  |  |  |  |
|  | 115-05661 | 4 Weekend |  | W | 3.44 | 182 | 191 | 194 | 207 | 65 | 65 | 65 | 1.06 | 1.08 |  |  |  |  |  |
|  | 115-05660 | 1 AM Peak |  | W | 4.14 | 235 | 233 | 249 | 293 | 65 | 65 | 65 | 1.06 | 1.25 | 1.17 | 1.50 | 55\% |  |  |
|  | 115-05660 | 2 Mid Day |  | W | 4.14 | 236 | 238 | 249 | 317 | 65 | 65 | 65 | 1.06 | 1.33 |  |  |  |  |  |
|  | 115-05660 | 3 PM Peak |  | W | 4.14 | 232 | 237 | 245 | 304 | 65 | 65 | 65 | 1.05 | 1.29 |  |  |  |  |  |
|  | 115-05660 | 4 Weekend |  | W | 4.14 | 235 | 242 | 276 | 364 | 65 | 65 | 65 | 1.17 | 1.50 |  |  |  |  |  |
| 19 | 115N05660 | 1 AM Peak | US-60 | N | 0.07 | 5 | 5 | 6 | 9 | 55 | 55 | 55 | 1.18 | 1.86 | 1.33 | 2.43 | 1\% | 1.20 | 1.74 |
|  | 115N05660 | 2 Mid Day | US-60 | N | 0.07 | 5 | 5 | 6 | 11 | 55 | 55 | 55 | 1.21 | 2.08 |  |  |  |  |  |
|  | 115N05660 | 3 PM Peak | US-60 | N | 0.07 | 5 | 5 | 6 | 10 | 55 | 55 | 55 | 1.19 | 1.96 |  |  |  |  |  |
|  | 115N05660 | 4 Weekend | US-60 | N | 0.07 | 5 | 5 | 7 | 12 | 55 | 55 | 55 | 1.33 | 2.43 |  |  |  |  |  |
|  | 115-05659 | 1 AM Peak |  | N | 1.42 | 104 | 102 | 119 | 150 | 55 | 55 | 55 | 1.14 | 1.47 | 1.21 | 1.96 | 24\% |  |  |
|  | 115-05659 | 2 Mid Day |  | N | 1.42 | 112 | 111 | 129 | 172 | 55 | 55 | 55 | 1.15 | 1.55 |  |  |  |  |  |
|  | 115-05659 | 3 PM Peak |  | N | 1.42 | 103 | 108 | 119 | 169 | 55 | 55 | 55 | 1.16 | 1.56 |  |  |  |  |  |





| $\begin{aligned} & \stackrel{\rightharpoonup}{\overleftarrow{D}} \\ & \stackrel{\text { En }}{0} \\ & \text { in } \end{aligned}$ | $\sum_{i}^{\cup}$ |  |  |  | $\frac{\ddot{y}}{\bar{\Sigma}}$ |  |  |  |  |  |  |  |  | $\stackrel{\sim}{E}$ |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 115-05668 | 3 PM Peak | US-60 | S | 1.43 | 100 | 103 | 109 | 135 | 55 | 55 | 55 | 1.09 | 1.32 |  |  |  |  |  |
|  | 115-05668 | 4 Weekend | US-60 | S | 1.43 | 101 | 105 | 111 | 135 | 55 | 55 | 55 | 1.10 | 1.29 |  |  |  |  |  |
|  | 115P11100 | 1 AM Peak | US-60 | E | 0.23 | 21 | 26 | 26 | 35 | 55 | 55 | 55 | 1.25 | 1.36 | 1.47 | 1.68 | 1\% |  |  |
|  | 115P11100 | 2 Mid Day | US-60 | E | 0.23 | 23 | 26 | 29 | 38 | 55 | 55 | 55 | 1.28 | 1.45 |  |  |  |  |  |
|  | 115P11100 | 3 PM Peak | US-60 | E | 0.23 | 24 | 28 | 29 | 38 | 55 | 55 | 55 | 1.22 | 1.36 |  |  |  |  |  |
|  | 115P11100 | 4 Weekend | US-60 | E | 0.23 | 19 | 22 | 28 | 36 | 55 | 55 | 55 | 1.47 | 1.68 |  |  |  |  |  |
|  | 115+05669 | 1 AM Peak | US-60 | E | 1.92 | 169 | 198 | 207 | 268 | 55 | 55 | 55 | 1.22 | 1.35 | 1.42 | 1.64 | 12\% |  |  |
|  | 115+05669 | 2 Mid Day | US-60 | E | 1.92 | 180 | 202 | 228 | 288 | 55 | 55 | 55 | 1.27 | 1.43 |  |  |  |  |  |
|  | 115+05669 | 3 PM Peak | US-60 | E | 1.92 | 190 | 213 | 231 | 293 | 55 | 55 | 55 | 1.22 | 1.37 |  |  |  |  |  |
|  | 115+05669 | 4 Weekend | US-60 | E | 1.92 | 154 | 173 | 220 | 283 | 55 | 55 | 55 | 1.42 | 1.64 |  |  |  |  |  |
|  | 115+05671 | 1 AM Peak | US-60 | E | 4.23 | 307 | 325 | 343 | 431 | 55 | 55 | 55 | 1.11 | 1.33 | 1.19 | 1.33 | 26\% |  |  |
|  | 115+05671 | 2 Mid Day | US-60 | E | 4.23 | 316 | 329 | 359 | 423 | 55 | 55 | 55 | 1.14 | 1.29 |  |  |  |  |  |
|  | 115+05671 | 3 PM Peak | US-60 | E | 4.23 | 319 | 338 | 358 | 423 | 55 | 55 | 55 | 1.12 | 1.25 |  |  |  |  |  |
|  | 115+05671 | 4 Weekend | US-60 | E | 4.23 | 299 | 317 | 354 | 421 | 55 | 55 | 55 | 1.19 | 1.33 |  |  |  |  |  |
|  | 115+05674 | 1 AM Peak | US-60 | E | 4.32 | 297 | 305 | 322 | 451 | 55 | 55 | 55 | 1.09 | 1.48 | 1.10 | 1.71 | 26\% |  |  |
|  | 115+05674 | 2 Mid Day | US-60 | E | 4.32 | 300 | 305 | 331 | 521 | 55 | 55 | 55 | 1.10 | 1.71 |  |  |  |  |  |
|  | 115+05674 | 3 PM Peak | US-60 | E | 4.32 | 298 | 305 | 322 | 380 | 55 | 55 | 55 | 1.08 | 1.24 |  |  |  |  |  |
|  | 115+05674 | 4 Weekend | US-60 | E | 4.32 | 294 | 305 | 321 | 374 | 55 | 55 | 55 | 1.09 | 1.23 |  |  |  |  |  |
|  | 115+05675 | 1 AM Peak | US-60 | E | 4.23 | 302 | 307 | 325 | 399 | 55 | 55 | 55 | 1.07 | 1.30 |  |  |  |  |  |
|  | 115+05675 | 2 Mid Day | US-60 | E | 4.23 | 307 | 311 | 331 | 391 | 55 | 55 | 55 | 1.08 | 1.26 |  |  |  |  |  |
|  | 115+05675 | 3 PM Peak | US-60 | E | 4.23 | 305 | 310 | 330 | 391 | 55 | 55 | 55 | 1.08 | 1.26 | 1.10 | 1.30 | 26\% |  |  |
|  | 115+05675 | 4 Weekend | US-60 | E | 4.23 | 299 | 305 | 329 | 394 | 55 | 55 | 55 | 1.10 | 1.29 |  |  |  |  |  |
| 15 | 115P05668 | 1 AM Peak | US-60 | E | 0.41 | 33 | 34 | 36 | 44 | 50 | 50 | 50 | 1.10 | 1.28 | 1.14 | 1.29 | 100\% | 1.14 | 1.29 |
|  | 115P05668 | 2 Mid Day | US-60 | E | 0.41 | 34 | 34 | 38 | 44 | 50 | 50 | 50 | 1.11 | 1.27 |  |  |  |  |  |
|  | 115P05668 | 3 PM Peak | US-60 | E | 0.41 | 34 | 34 | 37 | 42 | 50 | 50 | 50 | 1.11 | 1.23 |  |  |  |  |  |
|  | 115P05668 | 4 Weekend | US-60 | E | 0.41 | 32 | 34 | 37 | 44 | 50 | 50 | 50 | 1.14 | 1.29 |  |  |  |  |  |
| 16 | 115+05668 | 1 AM Peak | US-60 | E | 2.43 | 174 | 175 | 189 | 236 | 50 | 50 | 50 | 1.09 | 1.35 | 1.12 | 1.42 | 100\% | 1.12 | 1.42 |
|  | 115+05668 | 2 Mid Day | US-60 | E | 2.43 | 175 | 178 | 192 | 240 | 50 | 50 | 50 | 1.10 | 1.35 |  |  |  |  |  |
|  | 115+05668 | 3 PM Peak | US-60 | E | 2.43 | 173 | 176 | 194 | 250 | 50 | 50 | 50 | 1.12 | 1.42 |  |  |  |  |  |
|  | 115+05668 | 4 Weekend | US-60 | E | 2.43 | 172 | 178 | 190 | 230 | 50 | 50 | 50 | 1.10 | 1.29 |  |  |  |  |  |
| 17 | 115+05663 | 1 AM Peak | US-60 | E | 1.43 | 78 | 78 | 80 | 87 | 65 | 65 | 65 | 1.04 | 1.12 | 1.05 | 1.13 | 12\% | 1.09 | 1.20 |
|  | 115+05663 | 2 Mid Day | US-60 | E | 1.43 | 78 | 79 | 81 | 88 | 65 | 65 | 65 | 1.04 | 1.11 |  |  |  |  |  |
|  | 115+05663 | 3 PM Peak | US-60 | E | 1.43 | 77 | 79 | 81 | 89 | 65 | 65 | 65 | 1.05 | 1.13 |  |  |  |  |  |
|  | 115+05663 | 4 Weekend | US-60 | E | 1.43 | 78 | 81 | 82 | 89 | 65 | 65 | 65 | 1.05 | 1.10 |  |  |  |  |  |
|  | 115+05664 | 1 AM Peak | US-60 | E | 1.69 | 92 | 92 | 95 | 103 | 65 | 65 | 65 | 1.03 | 1.12 | 1.06 | 1.14 | 15\% |  |  |
|  | 115+05664 | 2 Mid Day | US-60 | E | 1.69 | 92 | 93 | 96 | 103 | 65 | 65 | 65 | 1.04 | 1.11 |  |  |  |  |  |
|  | 115+05664 | 3 PM Peak | US-60 | E | 1.69 | 91 | 92 | 96 | 105 | 65 | 65 | 65 | 1.06 | 1.14 |  |  |  |  |  |


|  | $\sum_{i}^{U}$ |  |  |  | $\stackrel{\check{\varkappa}}{\stackrel{\rightharpoonup}{\Sigma}}$ |  |  |  |  |  |  |  | $\underset{\sim}{\mathbf{q}}$ | $\stackrel{\sim}{E}$ |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 115+05664 | 4 Weekend | US-60 | E | 1.69 | 92 | 95 | 96 | 105 | 65 | 65 | 65 | 1.05 | 1.10 |  |  |  |  |  |
|  | 115+05665 | 1 AM Peak | US-60 | E | 5.63 | 325 | 337 | 348 | 406 | 65 | 65 | 65 | 1.07 | 1.20 | 1.12 | 1.26 | 49\% |  |  |
|  | 115+05665 | 2 Mid Day | US-60 | E | 5.63 | 329 | 336 | 359 | 414 | 65 | 65 | 65 | 1.09 | 1.23 |  |  |  |  |  |
|  | 115+05665 | 3 PM Peak | US-60 | E | 5.63 | 331 | 344 | 363 | 415 | 65 | 65 | 65 | 1.10 | 1.21 |  |  |  |  |  |
|  | 115+05665 | 4 Weekend | US-60 | E | 5.63 | 317 | 335 | 356 | 422 | 65 | 65 | 65 | 1.12 | 1.26 |  |  |  |  |  |
|  | 115+05666 | 1 AM Peak | US-60 | E | 0.95 | 51 | 52 | 53 | 59 | 65 | 65 | 65 | 1.04 | 1.14 | 1.06 | 1.16 | 8\% |  |  |
|  | 115+05666 | 2 Mid Day | US-60 | E | 0.95 | 52 | 52 | 54 | 59 | 65 | 65 | 65 | 1.05 | 1.14 |  |  |  |  |  |
|  | 115+05666 | 3 PM Peak | US-60 | E | 0.95 | 51 | 52 | 54 | 60 | 65 | 65 | 65 | 1.05 | 1.16 |  |  |  |  |  |
|  | 115+05666 | 4 Weekend | US-60 | E | 0.95 | 52 | 53 | 55 | 62 | 65 | 65 | 65 | 1.06 | 1.16 |  |  |  |  |  |
|  | 115+05667 | 1 AM Peak | US-60 | E | 1.81 | 102 | 103 | 106 | 119 | 65 | 65 | 65 | 1.04 | 1.15 |  |  |  |  |  |
|  | 115+05667 | 2 Mid Day | US-60 | E | 1.81 | 102 | 103 | 108 | 119 | 65 | 65 | 65 | 1.06 | 1.15 |  |  |  |  |  |
|  | 115+05667 | 3 PM Peak | US-60 | E | 1.81 | 102 | 103 | 109 | 121 | 65 | 65 | 65 | 1.07 | 1.17 | 1.07 | 1.17 | 16\% |  |  |
|  | 115+05667 | 4 Weekend | US-60 | E | 1.81 | 101 | 105 | 109 | 121 | 65 | 65 | 65 | 1.07 | 1.16 |  |  |  |  |  |
| 18 | 115 P 05662 | 1 AM Peak | US-60 | E | 1.10 | 59 | 60 | 61 | 66 | 65 | 65 | 65 | 1.03 | 1.10 | 1.05 | 1.10 | 24\% | 1.05 | 1.13 |
|  | 115P05662 | 2 Mid Day | US-60 | E | 1.10 | 59 | 60 | 61 | 66 | 65 | 65 | 65 | 1.03 | 1.10 |  |  |  |  |  |
|  | 115P05662 | 3 PM Peak | US-60 | E | 1.10 | 59 | 60 | 61 | 66 | 65 | 65 | 65 | 1.04 | 1.10 |  |  |  |  |  |
|  | 115P05662 | 4 Weekend | US-60 | E | 1.10 | 59 | 61 | 62 | 67 | 65 | 65 | 65 | 1.05 | 1.10 |  |  |  |  |  |
|  | 115+05662 | 1 AM Peak | US-60 | E | 3.44 | 188 | 188 | 195 | 208 | 65 | 65 | 65 | 1.04 | 1.10 | 1.05 | 1.14 | 76\% |  |  |
|  | 115+05662 | 2 Mid Day | US-60 | E | 3.44 | 188 | 188 | 196 | 214 | 65 | 65 | 65 | 1.04 | 1.14 |  |  |  |  |  |
|  | 115+05662 | 3 PM Peak | US-60 | E | 3.44 | 187 | 189 | 195 | 210 | 65 | 65 | 65 | 1.05 | 1.11 |  |  |  |  |  |
|  | 115+05662 | 4 Weekend | US-60 | E | 3.44 | 188 | 191 | 197 | 214 | 65 | 65 | 65 | 1.05 | 1.12 |  |  |  |  |  |
| 19 | (1) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 1.14 | 1.52 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
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|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |



Closure Data

|  |  | Total miles of closures |  | Average Occurrences/Mile/Year |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Segment | Length (miles) | \# of closures | NB (or WB) | SB (or EB) | NB (or WB) | SB (or EB) |
| $191-1$ | 24 | 7 | 5.0 | 2.0 | 0.04 | 0.02 |
| $191-2$ | 43 | 9 | 7.0 | 2.1 | 0.03 | 0.01 |
| $191-3$ | 17 | 2 | 2.0 | 0.0 | 0.02 | 0.00 |
| $191-4$ | 12 | 9 | 5.0 | 4.0 | 0.08 | 0.07 |
| $191-5$ | 5 | 10 | 5.0 | 5.0 | 0.20 | 0.20 |
| $70-6$ | 9 | 3 | 1.0 | 2.0 | 0.02 | 0.04 |
| $70-7$ | 30 | 8 | 6.0 | 2.0 | 0.04 | 0.01 |
| $70-8$ | 2 | 1 | 1.0 | 0.0 | 0.10 | 0.00 |
| $70-9$ | 5 | 2 | 1.0 | 1.0 | 0.04 | 0.04 |
| $70-10$ | 19 | 12 | 7.0 | 5.0 | 0.07 | 0.05 |
| $70-11$ | 4 | 0 | 0.0 | 0.0 | 0.00 | 0.00 |
| $70-12$ | 15 | 3 | 13.0 | 0.0 | 0.17 | 0.00 |
| $70 \mid 60-13$ | 12 | 29 | 13.0 | 21.0 | 0.22 | 0.35 |
| $60 \mathrm{E}-14$ | 16 | 99 | 53.2 | 147.4 | 0.67 | 1.84 |
| $60 \mathrm{E}-15$ | 2 | 7 | 0.0 | 9.0 | 0.00 | 0.90 |
| $60 \mathrm{E}-16$ | 2 | 9 | 6.0 | 1.5 | 0.60 | 0.15 |
| $60 \mathrm{E}-17$ | 11 | 12 | 2.0 | 12.8 | 0.04 | 0.23 |
| $60 \mathrm{E}-18$ | 7 | 8 | 0.0 | 8.0 | 0.00 | 0.23 |
| $60 \mathrm{E}-19$ | 6 | 12 | 3.0 | 9.0 | 0.10 | 0.30 |
| $60 \mathrm{E}-20$ | 5 | 18 | 16.0 | 2.0 | 0.68 | 0.09 |

## Aロロт

| Segment | ITIS Category Description |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Closures |  | Incidents/Accidents |  | Incidents/Crashes |  | Obstruction Hazards |  | Winds |  | Winter Storm Codes |  |
|  | NB (or WB) | SB (or EB) | NB (or WB) | SB (or EB) | NB (or WB) | SB (or EB) | NB (or WB) | SB (or EB) | NB (or WB) | SB (or EB) | NB (or WB) | SB (or EB) |
| 191-1 | 5 | 2 | 0 | 0 | 5 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| 191-2 | 7 | 2 | 0 | 0 | 5 | 1 | 1 | 1 | 0 | 0 | 0 | 0 |
| 191-3 | 2 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 191-4 | 5 | 4 | 1 | 1 | 2 | 2 | 2 | 1 | 0 | 0 | 0 | 0 |
| 191-5 | 5 | 5 | 3 | 1 | 1 | 4 | 0 | 0 | 0 | 0 | 0 | 0 |
| 70-6 | 1 | 2 | 0 | 0 | 1 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| 70-7 | 6 | 2 | 1 | 0 | 4 | 2 | 1 | 0 | 0 | 0 | 0 | 0 |
| 70-8 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 70-9 | 1 | 1 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
| 70-10 | 7 | 5 | 1 | 0 | 6 | 5 | 0 | 0 | 0 | 0 | 0 | 0 |
| 70-11 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 70-12 | 3 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 70\|60-13 | 13 | 16 | 2 | 4 | 8 | 9 | 0 | 1 | 0 | 0 | 1 | 1 |
| 60E-14 | 52 | 47 | 6 | 6 | 43 | 28 | 2 | 7 | 0 | 0 | 0 | 6 |
| 60E-15 | 0 | 7 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 4 |
| 60E-16 | 6 | 3 | 2 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 3 |
| 60E-17 | 2 | 10 | 1 | 2 | 1 | 7 | 0 | 0 | 0 | 0 | 0 | 0 |
| 60E-18 | 0 | 8 | 0 | 2 | 0 | 4 | 0 | 1 | 0 | 0 | 0 | 0 |
| 60E-19 | 3 | 9 | 1 | 2 | 1 | 5 | 0 | 0 | 0 | 0 | 0 | 0 |
| 60E-20 | 16 | 2 | 3 | 1 | 11 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |

HPMS Data

| SEGMENT | MP_FROM | MP_TO | WEIGHTED AVERAGE NB/WB AADT | WEIGHTED AVERAGE SB/EB AADT | WEIGHTED AVERAGE AADT | NB/WB <br> AADT | SB/EB <br> AADT | $\begin{aligned} & 2015 \\ & \text { AADT } \end{aligned}$ | K Factor | D-Factor | T-Factor |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 191-1 | 0 | 24 | 988 | 1003 | 1991 | 1040 | 1053 | 2093 | 10 | 53 | 15 |
| 191-2 | 24 | 67 | 695 | 811 | 1506 | 628 | 906 | 1534 | 11 | 62 | 27 |
| 191-3 | 87 | 104 | 1303 | 1285 | 2587 | 1316 | 1301 | 2617 | 7 | 50 | 21 |
| 191-4 | 104 | 116 | 2279 | 2262 | 4541 | 2184 | 2159 | 4343 | 8 | 50 | 16 |
| 191-5 | 116 | 121 | 4207 | 4427 | 8634 | 3828 | 4075 | 7903 | 7 | 56 | 10 |
| 70-6 | 339 | 330 | 6215 | 5968 | 12182 | 5888 | 5665 | 11553 | 9 | 51 | 13 |
| 70-7 | 330 | 300 | 1636 | 1497 | 3132 | 1671 | 1446 | 3116 | 7 | 54 | 27 |
| 70-8 | 300 | 298 | 1461 | 1180 | 2641 | 1599 | 1150 | 2749 | 7 | 58 | 25 |
| 70-9 | 298 | 293 | 1461 | 1180 | 2641 | 1599 | 1150 | 2749 | 7 | 58 | 25 |
| 70-10 | 293 | 274 | 1461 | 1180 | 2641 | 1599 | 1150 | 2749 | 7 | 58 | 25 |
| 70-11 | 274 | 270 | 1509 | 1240 | 2749 | 1638 | 1213 | 2850 | 7 | 58 | 25 |
| 70-12 | 270 | 255 | 2419 | 2508 | 4926 | 2774 | 3016 | 5790 | 8 | 52 | 17 |
| 70/60E-13 | 255 | 243 | 5575 | 5554 | 11128 | 5623 | 5520 | 11143 | 8 | 52 | 11 |
| 60E-14 | 243 | 227 | 4552 | 5025 | 9577 | 4152 | 5976 | 10128 | 9 | 59 | 11 |
| 60E-15 | 227 | 225 | 4017 | 3906 | 7923 | 3763 | 3726 | 7489 | 7 | 51 | 18 |
| 60E-16 | 225 | 223 | 5292 | 5200 | 10492 | 5338 | 5355 | 10693 | 10 | 50 | 13 |
| 60E-17 | 223 | 212 | 5342 | 5247 | 10589 | 5537 | 5511 | 11048 | 10 | 50 | 13 |
| 60E-18 | 205 | 212 | 7213 | 7123 | 14336 | 6755 | 7248 | 14003 | 8 | 52 | 13 |
| 60E-19 | 199 | 205 | 10565 | 10301 | 20867 | 9049 | 9603 | 18653 | 8 | 53 | 11 |
| 60E-20 | 194.3 | 199 | 24463 | 25455 | 49918 | 23754 | 24754 | 48507 | 8 | 51 | 9 |


| SEGMENT | Loc ID | BMP | EMP | Length | Pos Dir AADT | Neg Dir AADT | Corrected Pos Dir AADT | Corrected Neg Dir AADT | $\begin{aligned} & 2015 \\ & \text { AADT } \end{aligned}$ | K Factor | D-Factor | D-Factor Adjusted | T-Factor |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 191-1 | 102213 | 0.00 | 3.90 | 3.90 | 1640 | 1570 | 1640 | 1570 | 3210 | 10 | 60 | 51 | 12 |
|  | 102214 | 3.90 | 7.40 | 3.50 | 1544 | 1652 | 1544 | 1652 | 3196 | 10 | 67 | 52 | 9 |
|  | 102215 | 7.40 | 18.33 | 10.93 | 873 | 962 | 873 | 962 | 1835 | 10 | 66 | 52 | 13 |
|  | 102216 | 18.33 | 24.53 | 6.20 | 671 | 551 | 671 | 551 | 1222 | 10 | 64 | 55 | 21 |
| 191-2 | 102217 | 24.53 | 38.14 | 13.61 | 831 | 785 | 831 | 785 | 1616 | 9 | 63 | 51 | 33 |
|  | 102218 | 38.14 | 42.95 | 4.81 | 843 | 881 | 843 | 881 | 1724 | 8 | 52 | 51 | 31 |
|  | 102219 | 42.95 | 48.04 | 5.09 | 535 | 539 | 535 | 539 | 1074 | 11 | 50 | 50 | 18 |
|  | 102220 | 48.04 | 55.67 | 7.63 | 986 | 997 | 986 | 997 | 1983 | 11 | 59 | 50 | 17 |
|  | 102221 | 55.67 | 66.26 | 10.59 | 56 | 1183 | 56 | 1183 | 1239 | 13 | 59 | 95 | 29 |
| 191-3 | 102222 | 87.43 | 89.98 | 2.55 | 1133 | 1160 | 1133 | 1160 | 2293 | 9 | 57 | 51 | 20 |
|  | 102223 | 89.98 | 104.36 | 14.38 | 1348 | 1326 | 1348 | 1326 | 2674 | 7 | 57 | 50 | 21 |
| 191-4 | 102224 | 104.36 | 113.68 | 9.32 | 1372 | 1340 | 1372 | 1340 | 2712 | 7 | 55 | 51 | 19 |
|  | 102225 | 113.68 | 118.83 | 5.15 | 3653 | 3642 | 3653 | 3642 | 7295 | 9 | 54 | 50 | 11 |
| 191-5 | 102226 | 118.83 | 120.32 | 1.49 | 3111 | 3957 | 3111 | 3957 | 7068 | 7 | 63 | 56 | 10 |
|  | 102228 | 120.32 | 121.02 | 0.70 | 5354 | 4327 | 5354 | 4327 | 9681 | 6 | 54 | 55 | 11 |
| 70-6 | 102032 | 330.32 | 330.75 | 0.43 | 2654 | 2579 | 2654 | 2579 | 5233 | 9 | 50 | 51 | 15 |
|  | 102034 | 330.75 | 331.30 | 0.55 | 3319 | 3236 | 3319 | 3236 | 6555 | 8 | 51 | 51 | 22 |
|  | 102036 | 331.30 | 331.80 | 0.50 | 4404 | 4413 | 4404 | 4413 | 8817 | 8 | 54 | 50 | 17 |
|  | 102038 | 331.80 | 335.50 | 3.70 | 4656 | 4401 | 4656 | 4401 | 9057 | 8 | 56 | 51 | 14 |
|  | 102040 | 335.50 | 335.98 | 0.48 | 5580 | 5359 | 5580 | 5359 | 10939 | 8 | 54 | 51 | 9 |
|  | 102042 | 335.98 | 336.62 | 0.64 | 6247 | 6268 | 6247 | 6268 | 12515 | 8 | 56 | 50 | 9 |
|  | 102044 | 336.62 | 337.94 | 1.32 | 8414 | 8131 | 8414 | 8131 | 16545 | 10 | 55 | 51 | 11 |
|  | 102046 | 337.94 | 338.32 | 0.38 | 9620 | 9333 | 9620 | 9333 | 18953 | 10 | 52 | 51 | 16 |
|  | 102048 | 338.32 | 338.97 | 0.65 | 9574 | 9216 | 9574 | 9216 | 18790 | 10 | 51 | 51 | 7 |
|  | 102050 | 338.97 | 339.46 | 0.49 | 7473 | 7248 | 7473 | 7248 | 14721 | 9 | 50 | 51 | 8 |
|  | 102052 | 339.46 | 339.75 | 0.29 | 6233 | 5628 | 6233 | 5628 | 11861 | 10 | 56 | 53 | 10 |
| 70-7 | 102029 | 301.52 | 313.45 | 11.93 | 1638 | 1654 | 1638 | 1654 | 3292 | 7 | 53 | 50 | 28 |
|  | 102030 | 313.45 | 328.90 | 15.45 | 1761 | 1752 | 1761 | 1752 | 3513 | 6 | 52 | 50 | 27 |
|  | 102031 | 328.90 | 330.32 | 1.42 | 2424 | 2389 | 2424 | 2389 | 4813 | 9 | 53 | 50 | 26 |
|  | 102028 | 272.55 | 301.52 | 28.97 | 1599 | 1150 | 1599 | 1150 | 2749 | 7 | 63 | 58 | 25 |
| 70-8 | 102028 | 272.55 | 301.52 | 28.97 | 1599 | 1150 | 1599 | 1150 | 2749 | 7 | 63 | 58 | 25 |
| 70-9 | 102028 | 272.55 | 301.52 | 28.97 | 1599 | 1150 | 1599 | 1150 | 2749 | 7 | 63 | 58 | 25 |
| 70-10 | 102028 | 272.55 | 301.52 | 28.97 | 1599 | 1150 | 1599 | 1150 | 2749 | 7 | 63 | 58 | 25 |
| $70-11$ | 102026 | 272.01 | 272.55 | 0.54 | - | - | 1911 | 1911 | 3822 | 22 | 56 | 50 | 14 |
|  | 102027 | 271.27 | 272.01 | 0.74 | 2947 | 3154 | 2947 | 3154 | 6101 | 8 | 56 | 52 | 14 |
|  | 102028 | 272.55 | 301.52 | 28.97 | 1599 | 1150 | 1599 | 1150 | 2749 | 7 | 63 | 58 | 25 |

April 2023

| 70-12 | 102025 | 259.46 | 271.27 | 11.81 | 2774 | 3016 | 2774 | 3016 | 5790 | 8 | 55 | 52 | 17 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 70/60-13 | 101909 | 244.34 | 245.60 | 1.26 | 6647 | 6829 | 6647 | 6829 | 13476 | 6 | 54 | 51 | 6 |
|  | 101911 | 245.60 | 247.05 | 1.45 | 9132 | 7336 | 9132 | 7336 | 16468 | 8 | 56 | 55 | 5 |
|  | 101912 | 247.05 | 248.12 | 1.07 | 5580 | 7423 | 5580 | 7423 | 13003 | 8 | 59 | 57 | 13 |
|  | 101914 | 248.12 | 249.46 | 1.34 | 14066 | 11692 | 14066 | 11692 | 25758 | 8 | 54 | 55 | 6 |
|  | 101916 | 249.46 | 250.06 | 0.60 | 13448 | 12534 | 13448 | 12534 | 25982 | 8 | 54 | 52 | 6 |
|  | 101918 | 250.06 | 250.46 | 0.40 | 7930 | 11322 | 7930 | 11322 | 19252 | 8 | 58 | 59 | 7 |
|  | 101920 | 250.46 | 251.03 | 0.57 | 8475 | 8533 | 8475 | 8533 | 17008 | 6 | 53 | 50 | 8 |
|  | 101922 | 251.03 | 252.14 | 1.11 | 7443 | 7446 | 7443 | 7446 | 14889 | 10 | 56 | 50 | 10 |
|  | 101924 | 252.14 | 255.56 | 3.42 | 1639 | 1767 | 1639 | 1767 | 3406 | 7 | 52 | 52 | 14 |
|  | 102019 | 252.14 | 252.79 | 0.65 | 6000 | 5355 | 6000 | 5355 | 11355 | 9 | 55 | 53 | 10 |
|  | 102021 | 252.79 | 253.38 | 0.59 | 5200 | 5053 | 5200 | 5053 | 10253 | 9 | 55 | 51 | 9 |
|  | 102023 | 253.38 | 254.10 | 0.72 | 3629 | 3442 | 3629 | 3442 | 7071 | 8 | 53 | 51 | 16 |
|  | 102024 | 254.10 | 259.46 | 5.36 | 3415 | 3605 | 3415 | 3605 | 7020 | 8 | 51 | 51 | 15 |
| 60E-14 | 101908 | 242.70 | 244.34 | 1.64 | 4152 | 5976 | 4152 | 5976 | 10128 | 9 | 51 | 59 | 11 |
| 60E-15 | 101906 | 225.82 | 226.89 | 1.07 | 3774 | 2769 | 3774 | 2769 | 6543 | 6 | 60 | 58 | 42 |
|  | 101907 | 226.89 | 242.70 | 15.81 | 3762 | 3791 | 3762 | 3791 | 7553 | 7 | 61 | 50 | 16 |
| 60E-16 | 101905 | 213.50 | 225.82 | 12.32 | 5338 | 5355 | 5338 | 5355 | 10693 | 10 | 63 | 50 | 13 |
| 60E-17 | 101904 | 212.23 | 213.50 | 1.27 | 7470 | 7027 | 7470 | 7027 | 14497 | 5 | 62 | 52 | 14 |
|  | 101905 | 213.50 | 225.82 | 12.32 | 5338 | 5355 | 5338 | 5355 | 10693 | 10 | 63 | 50 | 13 |
| 60E-18 | 101903 | 208.25 | 212.23 | 3.98 | 7324 | 7443 | 7324 | 7443 | 14767 | 10 | 66 | 50 | 14 |
|  | 101902 | 204.18 | 208.25 | 4.07 | 6198 | 7058 | 6198 | 7058 | 13256 | 6 | 56 | 53 | 13 |
| 60E-19 | 101902 | 204.18 | 208.25 | 4.07 | 6198 | 7058 | 6198 | 7058 | 13256 | 6 | 56 | 53 | 13 |
|  | 101901 | 202.70 | 204.18 | 1.48 | 8960 | 7026 | 8960 | 7026 | 15986 | 9 | 72 | 56 | 10 |
|  | 101900 | 202.05 | 202.70 | 0.65 | 7765 | 11436 | 7765 | 11436 | 19201 | 6 | 60 | 60 | 9 |
|  | 101899 | 201.17 | 202.05 | 0.88 | 10953 | 11191 | 10953 | 11191 | 22144 | 8 | 52 | 51 | 9 |
|  | 101898 | 199.05 | 201.17 | 2.12 | 14189 | 15068 | 14189 | 15068 | 29257 | 12 | 60 | 52 | 8 |
| 60E-20 | 101897 | 198.42 | 199.05 | 0.63 | 13600 | 12001 | 13600 | 12001 | 25601 | 8 | 60 | 53 | 8 |
|  | 101896 | 197.41 | 198.42 | 1.01 | 14833 | 14514 | 14833 | 14514 | 29347 | 7 | 58 | 51 | 10 |
|  | 101895 | 196.41 | 197.41 | 1.00 | 15432 | 16379 | 15432 | 16379 | 31811 | 7 | 62 | 51 | 10 |
|  | 101894 | 195.41 | 196.41 | 1.00 | 22704 | 22509 | 22704 | 22509 | 45213 | 8 | 58 | 50 | 9 |
|  | 101893 | 193.41 | 195.41 | 2.00 | 36143 | 39251 | 36143 | 39251 | 75394 | 8 | 53 | 52 | 8 |

Bicycle Accommodation Data

| Segment | BMP | EMP | Divided or Non | NB/EB Right Shoulder Width | SB/WB <br> Right <br> Shoulder <br> Width | NB/EB Left Shoulder Width | SB/WB Left <br> Shoulder Width | NB/EB <br> Effective <br> Length of <br> Shoulder | SB/WB <br> Effective <br> Length of <br> Shoulder | \% Bicycle Accommodation |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 191-1 | 0 | 24 | Undivided | 6.8 | 6.8 | N/A | N/A | 15.7 | 15.8 |
| 2 | 191-2 | 24 | 67 | Undivided | 2.3 | 2.4 | N/A | N/A | 43.0 | 43.0 |
| 3 | 191-3 | 87 | 104 | Divided | 10.3 |  | 4.0 |  | 16.5 | 0.0 |
| 4 | 191-4 | 104 | 116 | Undivided | 7.9 | 7.8 | N/A | N/A | 11.8 | 11.3 |
| 5 | 191-5 | 116 | 121 | Undivided | 3.0 | 2.3 | N/A | N/A | 1.6 | 1.1 |
| 6 | 70-6 | 330 | 339 | Undivided | 2.9 | 2.8 | N/A | N/A | 4.2 | 4.1 |
| 7 | 70-7 | 300 | 330 | Undivided | 6.7 | 6.7 | N/A | N/A | 43.5 | 0.0 |
| 8 | 70-8 | 298 | 300 | Undivided | 5.0 | 5.0 | N/A | N/A | 0.0 | 0.0 |
| 9 | 70-9 | 293 | 298 | Undivided | 5.8 | 5.9 | N/A | N/A | 1.2 | 1.4 |
| 10 | 70-10 | 274 | 293 | Undivided | 5.1 | 5.1 | N/A | N/A | 0.8 | 0.9 |
| 11 | 70-11 | 270 | 274 | Undivided | 4.5 | 4.7 | N/A | N/A | 0.0 | 0.3 |
| 12 | 70-12 | 255 | 270 | Undivided | 5.4 | 5.3 | N/A | N/A | 4.1 | 2.8 |
| 13 | $\begin{gathered} 70 / 60 \mathrm{E}- \\ 13 \\ \hline \end{gathered}$ | 243 | 255 | Undivided | 5.04 | 4.1 | N/A | N/A | 7.8 | 5.2 |
| 14 | 60E-14 | 227 | 243 | Undivided | 4.86 | 4.8 | N/A | N/A | 7.7 | 7.8 |
| 15 | 60E-15 | 225 | 227 | Undivided | 7.88 | 7.9 | N/A | N/A | 1.9 | 1.9 |
| 16 | 60E-16 | 223 | 225 | Undivided | 7.75 | 7.2 | N/A | N/A | 1.8 | 1.7 |
| 17 | 60E-17 | 212 | 223 | Divided | 10.12 | 10.0 | 4.8 | 6.0 | 11.0 | 10.2 |
| 18 | 60E-18 | 205 | 212 | Divided | 6.57 | 9.6 | 4.0 | 6.0 | 7.0 | 7.0 |
| 19 | 60E-19 | 199 | 205 | Divided | 6.00 | 9.0 | 4.0 | 8.7 | 0.7 | 4.3 |
| 20 | 60E-20 | 194.3 | 199 | Divided | 10.00 | 10.0 | 8.0 | 8.0 | 4.7 | 4.7 |

## AZTDM Data

| SEGMENT | Growth Rate | \% Non-SOV |
| :---: | :---: | :---: |
| $191-1$ | $1.5 \%$ | $15.0 \%$ |
| $191-2$ | $2.7 \%$ | $16.6 \%$ |
| $191-3$ | $1.1 \%$ | $8.8 \%$ |
| $191-4$ | $1.1 \%$ | $8.3 \%$ |
| $191-5$ | $1.0 \%$ | $21.2 \%$ |
| $70-6$ | $1.0 \%$ | $17.8 \%$ |
| $70-7$ | $1.0 \%$ | $15.8 \%$ |
| $70-8$ | $1.1 \%$ | $12.8 \%$ |
| $70-9$ | $1.0 \%$ | $11.2 \%$ |
| $70-10$ | $1.1 \%$ | $7.7 \%$ |
| $70-11$ | $1.0 \%$ | $11.3 \%$ |
| $70-12$ | $1.0 \%$ | $12.5 \%$ |
| $70 / 60 \mathrm{E}-13$ | $1.2 \%$ | $16.6 \%$ |
| $60 \mathrm{E}-14$ | $1.9 \%$ | $14.0 \%$ |
| $60 \mathrm{E}-15$ | $3.9 \%$ | $10.5 \%$ |
| $60 \mathrm{E}-16$ | $3.9 \%$ | $7.7 \%$ |
| $60 \mathrm{E}-17$ | $3.9 \%$ | $8.9 \%$ |
| $60 \mathrm{E}-18$ | $2.3 \%$ | $12.0 \%$ |
| $60 \mathrm{E}-19$ | $-1.3 \%$ | $17.8 \%$ |
| $60 \mathrm{E}-20$ | $0.9 \%$ | $17.2 \%$ |

HERS Capacity Calculation Data

|  |  |  |  |  |  |  |  |  | $\sum_{\underset{\sim}{u n}}^{\stackrel{\text { Lu}}{\infty}}$ |  | 롬 | 亗 | ㄴ | 4 | 4 | $\stackrel{u}{\square 0}$ | 4 | $\stackrel{3}{4}$ | $\underline{L}$ | $4{ }^{\circ}$ | $\begin{aligned} & \stackrel{\sim}{u} \\ & \stackrel{\sim}{巴} \\ & \stackrel{\sim}{2} \end{aligned}$ | $\stackrel{\sim}{4}$ $\stackrel{\sim}{\infty}$ $\stackrel{\infty}{\infty}$ |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 191-1 | 3 | Rural | Level | 12.00 | 6.78 | 6.81 | 1.0 | N/A | N/A | N/A | 0.9 | 2 | 0.873 | N/A | N/A | 0.55 | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | 821.14 | 15,641 |
| 191-2 | 4 | Rural | Level | 12.00 | 2.32 | 2.37 | 2.6 | N/A | N/A | N/A | 0.88 | 1.9 | 0.805 | N/A | 2 | N/A | 1 | 1.20 | N/A | N/A | N/A | N/A | N/A | N/A | 876.54 | 16,696 |
| 191-3 | 2 | Rural | Level | 12.00 | 10.29 |  | 0.0 | 0 | 0.4 | N/A | 0.88 | 1.5 | 0.905 | 0 | 0.5 | N/A | N/A | N/A | N/A | N/A | N/A | N/A | 3330 | 3318 | N/A | 63,438 |
| 191-4 | 4 | Rural | Level | 12.00 | 7.93 | 7.81 | 0.0 | N/A | N/A | N/A | 0.88 | 1.5 | 0.925 | N/A | 3.25 | N/A | 1 | 2.75 | N/A | N/A | N/A | N/A | N/A | N/A | 1521.48 | 28,981 |
| 191-5 | 3 | Urban | Level | 12.00 | 2.97 | 2.29 | 1.0 | N/A | N/A | N/A | 0.9 | 2 | 0.910 | N/A | N/A | 0.55 | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | 1711.67 | 32,603 |
| 70-6 | 3 | Urban | Level | 12.00 | 2.89 | 2.82 | 1.0 | N/A | N/A | N/A | 0.9 | 2 | 0.886 | N/A | N/A | 0.55 | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | 1666.32 | 31,739 |
| 70-7 | 4 | Rural | Level | 12.00 | 6.68 | 6.67 | 0.0 | N/A | N/A | N/A | 0.88 | 1.9 | 0.807 | N/A | 2.25 | N/A | 1 | 0.70 | N/A | N/A | N/A | N/A | N/A | N/A | 1008.78 | 19,215 |
| 70-8 | 4 | Rural | Level | 12.00 | 5.00 | 5.00 | 0.0 | N/A | N/A | N/A | 0.88 | 1.9 | 0.814 | N/A | 1.75 | N/A | 1 | 1.10 | N/A | N/A | N/A | N/A | N/A | N/A | 1483.06 | 28,249 |
| 70-9 | 4 | Rural | Level | 12.00 | 5.82 | 5.92 | 0.0 | N/A | N/A | N/A | 0.88 | 1.9 | 0.814 | N/A | 3.5 | N/A | 1 | 1.70 | N/A | N/A | N/A | N/A | N/A | N/A | 682.71 | 13,004 |
| 70-10 | 4 | Rural | Level | 12.00 | 5.07 | 5.08 | 0.0 | N/A | N/A | N/A | 0.88 | 1.9 | 0.814 | N/A | 0.5 | N/A | 1 | 1.70 | N/A | N/A | N/A | N/A | N/A | N/A | 1051.75 | 20,033 |
| 70-11 | 4 | Rural | Level | 12.00 | 4.54 | 4.70 | 0.0 | N/A | N/A | N/A | 0.88 | 1.9 | 0.817 | N/A | 2.25 | N/A | 1 | 2.80 | N/A | N/A | N/A | N/A | N/A | N/A | 923.61 | 17,593 |
| 70-12 | 4 | Rural | Level | 12.00 | 5.43 | 5.33 | 0.0 | N/A | N/A | N/A | 0.88 | 1.5 | 0.921 | N/A | 1 | N/A | 1 | 2.20 | N/A | N/A | N/A | N/A | N/A | N/A | 1400.02 | 26,667 |
| $\begin{gathered} \hline 70 / 60 \mathrm{E}- \\ 13 \\ \hline \end{gathered}$ | 3 | Urban | Level | 12.00 | 5.04 | 4.15 | 1.0 | N/A | N/A | N/A | 0.9 | 2 | 0.899 | N/A | N/A | 0.55 | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | 1691.51 | 32,219 |
| 60E-14 | 4 | Rural | Mountainous | 12.00 | 4.86 | 4.78 | 0.0 | N/A | N/A | N/A | 0.88 | 7.2 | 0.584 | N/A | 1 | N/A | 0.87 | 2.55 | N/A | N/A | N/A | N/A | N/A | N/A | 473.84 | 9,025 |
| 60E-15 | 2 | Rural | Rolling | 12.00 | 7.88 | 7.87 | 0.0 | 0 | 0 | N/A | 0.88 | 2.5 | 0.787 | 1.6 | 5.75 | N/A | N/A | N/A | N/A | N/A | N/A | N/A | 2427 | 2427 | N/A | 46,222 |
| 60E-16 | 2 | Rural | Level | 12.00 | 7.75 | 7.25 | 0.0 | 0 | 0 | N/A | 0.88 | 1.5 | 0.937 | 1.6 | 0.07 | N/A | N/A | N/A | N/A | N/A | N/A | N/A | 3410 | 3410 | N/A | 64,943 |
| 60E-17 | 2 | Rural | Level | 12.00 | 10.12 | 10.00 | 0.0 | 0 | 0.4 | N/A | 0.88 | 1.5 | 0.937 | 0 | 0.5 | N/A | N/A | N/A | N/A | N/A | N/A | N/A | 3629 | 3629 | N/A | 69,129 |
| 60E-18 | 2 | Rural | Level | 12.00 | 6.57 | 9.59 | 0.0 | 0 | 0.4 | N/A | 0.88 | 1.5 | 0.938 | 0 | 0.57 | N/A | N/A | N/A | N/A | N/A | N/A | N/A | 1816 | 1816 | N/A | 34,584 |
| 60E-19 | 3 | Urban | Level | 12.00 | 6.00 | 9.05 | 1.0 | N/A | N/A | N/A | 0.9 | 2 | 0.904 | N/A | N/A | 0.55 | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | 850.28 | 16,196 |
| 60E-20 | 1 | Urban | Level | 12.00 | 10.00 | 10.00 | 0.0 | 0 | 0 | 1.76 | 0.94 | 1.5 | 0.958 | N/A | N/A | N/A | N/A | N/A | N/A | N/A | 70.22 | 70.22 | 2161 | 2161 | N/A | 41,163 |

Safety Performance Area Data

| Segment | Operating Environment | Segment Length (miles) | NB/WB Fatal Crashes 2015-2019 | SB/EB Fatal Crashes 2015-2019 | NB/WB Suspected Serious Injury Crashes | SB/EB Suspected Serious Injury Crashes | Fatal + Suspected Serious Injury Crashes Involving SHSP <br> Top 5 Emphasis Areas Behaviors |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 191-1 | 2 or 3 Lane Undivided Highway | 24 | 0 | 1 | 1 | 1 | 3 |
| 191-2 | 2 or 3 Lane Undivided Highway | 43 | 1 | 1 | 2 | 0 | 4 |
| 191-3 | 2 or 3 or 4 Lane Divided Highway | 17 | 0 | 1 | 0 | 3 | 4 |
| 191-4 | 2 or 3 Lane Undivided Highway | 12 | 1 | 0 | 2 | 2 | 5 |
| 191-5 | 4 or 5 Lane Undivided Highway | 5 | 0 | 0 | 2 | 0 | 2 |
| 70-6 | 4 or 5 Lane Undivided Highway | 9 | 1 | 0 | 8 | 3 | 12 |
| 70-7 | 2 or 3 Lane Undivided Highway | 30 | 2 | 1 | 1 | 1 | 5 |
| 70-8 | 2 or 3 Lane Undivided Highway | 2 | 1 | 0 | 0 | 0 | 1 |
| 70-9 | 2 or 3 Lane Undivided Highway | 5 | 0 | 1 | 0 | 0 | 1 |
| 70-10 | 2 or 3 Lane Undivided Highway | 19 | 1 | 3 | 1 | 2 | 7 |
| 70-11 | 2 or 3 Lane Undivided Highway | 4 | 2 | 0 | 0 | 0 | 2 |
| 70-12 | 2 or 3 Lane Undivided Highway | 15 | 4 | 3 | 0 | 2 | 9 |
| 60\|70-13 | 2 or 3 Lane Undivided Highway | 12 | 4 | 3 | 9 | 8 | 24 |
| 60-14 | 2 or 3 Lane Undivided Highway | 16 | 2 | 3 | 10 | 11 | 26 |
| 60-15 | 2 or 3 Lane Undivided Highway | 2 | 0 | 0 | 0 | 0 | 0 |
| 60-16 | 2 or 3 Lane Undivided Highway | 2 | 0 | 0 | 1 | 1 | 2 |
| 60-17 | 2 or 3 or 4 Lane Divided Highway | 11 | 3 | 1 | 3 | 2 | 9 |
| 60-18 | 2 or 3 or 4 Lane Divided Highway | 7 | 1 | 0 | 3 | 2 | 6 |
| 60-19 | 2 or 3 or 4 Lane Divided Highway | 6 | 2 | 0 | 2 | 6 | 10 |
| 60-20 | Urban 4 Lane Freeway | 5 | 3 | 1 | 0 | 2 | 6 |


| Segment | Operating Environment | Fatal + Suspected Serious Injury Crashes at Intersections | Fatal + Suspected Serious Injury Crashes Involving Lane Departures | Fatal + Suspected Serious Injury Crashes Involving Pedestrians | Fatal + Suspected Serious Injury Crashes Involving Trucks | Fatal + Suspected Serious Injury Crashes Involving Bicycles | Weighted 5-Year (2015-2019) <br> Average SB/WB AADT | Weighted 5-Year (20152019) Average Total AADT |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 191-1 | 2 or 3 Lane Undivided Highway | 1 | 1 | 0 | 0 | 0 | 988 | 1003 |
| 191-2 | 2 or 3 Lane Undivided Highway | 1 | 1 | 1 | 0 | 0 | 695 | 811 |
| 191-3 | 2 or 3 or 4 Lane Divided Highway | 0 | 4 | 0 | 0 | 0 | 1303 | 1285 |
| 191-4 | 2 or 3 Lane Undivided Highway | 2 | 3 | 0 | 0 | 0 | 2279 | 2262 |
| 191-5 | 4 or 5 Lane Undivided Highway | 0 | 2 | 0 | 0 | 0 | 4207 | 4427 |
| 70-6 | 4 or 5 Lane Undivided Highway | 5 | 3 | 1 | 0 | 1 | 6215 | 5968 |
| 70-7 | 2 or 3 Lane Undivided Highway | 0 | 3 | 1 | 0 | 0 | 1636 | 1497 |
| 70-8 | 2 or 3 Lane Undivided Highway | 0 | 0 | 0 | 0 | 0 | 1461 | 1180 |
| 70-9 | 2 or 3 Lane Undivided Highway | 0 | 0 | 1 | 0 | 0 | 1461 | 1180 |
| 70-10 | 2 or 3 Lane Undivided Highway | 0 | 5 | 0 | 0 | 1 | 1461 | 1180 |
| 70-11 | 2 or 3 Lane Undivided Highway | 0 | 0 | 2 | 0 | 0 | 1509 | 1240 |
| 70-12 | 2 or 3 Lane Undivided Highway | 0 | 2 | 3 | 0 | 0 | 2419 | 2508 |
| 60\|70-13 | 2 or 3 Lane Undivided Highway | 8 | 5 | 1 | 0 | 0 | 5575 | 5554 |
| 60-14 | 2 or 3 Lane Undivided Highway | 1 | 21 | 0 | 1 | 1 | 4552 | 5025 |
| 60-15 | 2 or 3 Lane Undivided Highway | 0 | 0 | 0 | 0 | 0 | 4017 | 3906 |
| 60-16 | 2 or 3 Lane Undivided Highway | 1 | 0 | 0 | 1 | 0 | 5292 | 5200 |
| 60-17 | 2 or 3 or 4 Lane Divided Highway | 1 | 7 | 1 | 1 | 0 | 5342 | 5247 |
| 60-18 | 2 or 3 or 4 Lane Divided Highway | 1 | 1 | 0 | 0 | 0 | 7213 | 7123 |
| 60-19 | 2 or 3 or 4 Lane Divided Highway | 4 | 6 | 0 | 0 | 0 | 10565 | 10301 |
| 60-20 | Urban 4 Lane Freeway | 0 | 3 | 0 | 0 | 0 | 24463 | 25455 |

HPMS Data

| 2011-2015 Weighted Average |  |  |  |  |  | 2016 |  |  | 2017 |  |  | 2018 |  |  | 2019 |  |  | 2020 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $\begin{aligned} & \circ_{1} \\ & \stackrel{0}{\Sigma} \end{aligned}$ | WEIGHTED <br> AVERAGE NB/WB AADT | WEIGHTED <br> AVERAGE <br> SB/EB AADT | WEIGHTED AVERAGE AADT | $\sum_{i=1}^{\infty}$ | $\stackrel{\sim}{\underset{\sim}{\sim}}$ | $\begin{aligned} & \text { b } \\ & \frac{1}{4} \\ & 0 \\ & 0 \\ & \text { N } \end{aligned}$ | $\sum_{i=1}^{\infty}$ | $\stackrel{\oplus}{\underset{\sim}{\mathscr{M}}}$ | $\begin{aligned} & \text { Le } \\ & \stackrel{y}{4} \\ & \text { Nì } \end{aligned}$ | $\sum_{\substack{\infty}}^{\infty}$ | $\stackrel{\sim}{\sim}$ |  | $\sum_{i=1}^{\infty} \frac{1}{2}$ | $\stackrel{\sim}{\underset{\sim}{\sim}}$ |  | $\sum_{i=1}^{\infty} \frac{1}{2}$ | $\stackrel{\sim}{\sim}$ | $\begin{aligned} & \text { Le } \\ & \text { Q } \\ & \text { N } \\ & \text { N } \end{aligned}$ |
| 191-1 | 0 | 24 | 988 | 1003 | 1991 | 1012 | 1012 | 2025 | 974 | 974 | 1947 | 949 | 994 | 1943 | 967 | 982 | 1949 | 1040 | 1053 | 2093 |
| 191-2 | 24 | 67 | 695 | 811 | 1506 | 692 | 693 | 1384 | 748 | 750 | 1498 | 785 | 769 | 1555 | 623 | 938 | 1561 | 628 | 906 | 1534 |
| 191-3 | 87 | 104 | 1303 | 1285 | 2587 | 1382 | 1382 | 2765 | 1159 | 1113 | 2271 | 1326 | 1312 | 2638 | 1330 | 1316 | 2646 | 1316 | 1301 | 2617 |
| 191-4 | 104 | 116 | 2279 | 2262 | 4541 | 2375 | 2375 | 4750 | 2362 | 2362 | 4723 | 2220 | 2187 | 4407 | 2254 | 2227 | 4481 | 2184 | 2159 | 4343 |
| 191-5 | 116 | 121 | 4207 | 4427 | 8634 | 4370 | 4422 | 8791 | 4704 | 4618 | 9323 | 3974 | 4598 | 8572 | 4157 | 4424 | 8581 | 3828 | 4075 | 7903 |
| 70-6 | 339 | 330 | 6215 | 5968 | 12182 | 6230 | 6095 | 12324 | 6025 | 5843 | 11868 | 6443 | 6108 | 12551 | 6487 | 6129 | 12616 | 5888 | 5665 | 11553 |
| 70-7 | 330 | 300 | 1636 | 1497 | 3132 | 1685 | 1664 | 3349 | 1369 | 1374 | 2743 | 1725 | 1498 | 3223 | 1729 | 1501 | 3230 | 1671 | 1446 | 3116 |
| 70-8 | 300 | 298 | 1461 | 1180 | 2641 | 1536 | 1485 | 3021 | 943 | 943 | 1885 | 1612 | 1160 | 2772 | 1617 | 1163 | 2780 | 1599 | 1150 | 2749 |
| 70-9 | 298 | 293 | 1461 | 1180 | 2641 | 1536 | 1485 | 3021 | 943 | 943 | 1885 | 1612 | 1160 | 2772 | 1617 | 1163 | 2780 | 1599 | 1150 | 2749 |
| 70-10 | 293 | 274 | 1461 | 1180 | 2641 | 1536 | 1485 | 3021 | 943 | 943 | 1885 | 1612 | 1160 | 2772 | 1617 | 1163 | 2780 | 1599 | 1150 | 2749 |
| 70-11 | 274 | 270 | 1509 | 1240 | 2749 | 1583 | 1527 | 3111 | 1019 | 1011 | 2030 | 1651 | 1223 | 2874 | 1656 | 1226 | 2882 | 1638 | 1213 | 2850 |
| 70-12 | 270 | 255 | 2419 | 2508 | 4926 | 2208 | 2208 | 4416 | 2338 | 2338 | 4676 | 2434 | 2434 | 4867 | 2339 | 2543 | 4882 | 2774 | 3016 | 5790 |
| $\begin{gathered} 70 / 60 \mathrm{E}- \\ 13 \\ \hline \end{gathered}$ | 255 | 243 | 5575 | 5554 | 11128 | 5827 | 5675 | 11502 | 5604 | 5445 | 11049 | 5397 | 5597 | 10994 | 5422 | 5532 | 10954 | 5623 | 5520 | 11143 |
| 60E-14 | 243 | 227 | 4552 | 5025 | 9577 | 4886 | 5692 | 10578 | 4470 | 4470 | 8940 | 4669 | 4486 | 9155 | 4581 | 4501 | 9082 | 4152 | 5976 | 10128 |
| 60E-15 | 227 | 225 | 4017 | 3906 | 7923 | 3652 | 3652 | 7304 | 4208 | 3913 | 8121 | 4170 | 4166 | 8337 | 4290 | 4072 | 8362 | 3763 | 3726 | 7489 |
| 60E-16 | 225 | 223 | 5292 | 5200 | 10492 | 4971 | 4771 | 9742 | 5160 | 4952 | 10112 | 5441 | 5377 | 10818 | 5551 | 5545 | 11096 | 5338 | 5355 | 10693 |
| 60E-17 | 223 | 212 | 5342 | 5247 | 10589 | 5032 | 4829 | 9861 | 5217 | 5010 | 10228 | 5411 | 5365 | 10776 | 5512 | 5519 | 11031 | 5537 | 5511 | 11048 |
| 60E-18 | 205 | 212 | 7213 | 7123 | 14336 | 6673 | 6683 | 13356 | 7088 | 7088 | 14176 | 7710 | 7236 | 14947 | 7837 | 7362 | 15199 | 6755 | 7248 | 14003 |
| 60E-19 | 199 | 205 | 10565 | 10301 | 20867 | 10477 | 10301 | 20778 | 10459 | 10277 | 20736 | 11334 | 10653 | 21987 | 11509 | 10673 | 22182 | 9049 | 9603 | 18653 |
| 60E-20 | 194.3 | 199 | 24463 | 25455 | 49918 | 23653 | 24561 | 48215 | 24199 | 25324 | 49524 | 25030 | 26120 | 51150 | 25679 | 26517 | 52195 | 23754 | 24754 | 48507 |

Freight Performance Area Data

|  |  | Total minutes of closures |  | Avg Mins/Mile/Year |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Segment | Length (miles) | \# of closures | NB (or WB) | SB (or EB) | NB (or WB) | SB (or EB) |
| $191-1^{*}$ | 24.00 | 7 | 362.0 | 120.0 | 3.02 | 1.00 |
| $191-2^{*}$ | 43.00 | 9 | 574.0 | 383.0 | 2.67 | 1.78 |
| $191-3^{\wedge}$ | 17.00 | 2 | 210.0 | 0.0 | 2.47 | 0.00 |
| $191-4^{\wedge}$ | 12.00 | 9 | 734.0 | 300.0 | 12.23 | 5.00 |
| $191-5^{*}$ | 5.00 | 10 | 652.0 | 424.0 | 26.08 | 16.96 |
| $70-6^{*}$ | 9.00 | 3 | 60.0 | 210.0 | 1.33 | 4.67 |
| $70-7^{\wedge}$ | 30.00 | 8 | 683.0 | 810.0 | 4.55 | 5.40 |
| $70-8^{\wedge}$ | 2.00 | 1 | 143.0 | 0.0 | 14.30 | 0.00 |
| $70-9^{\wedge}$ | 5.00 | 2 | 60.0 | 75.0 | 2.40 | 3.00 |
| $70-10^{\wedge}$ | 19.00 | 12 | 820.0 | 238.0 | 8.63 | 2.51 |
| $70-11^{\wedge}$ | 4.00 | 0 | 0.0 | 0.0 | 0.00 | 0.00 |
| $70-12^{\wedge}$ | 15.00 | 3 | 1304.0 | 0.0 | 17.39 | 0.00 |
| $70 / 60 \mathrm{E}-13^{*}$ | 12.00 | 29 | 1365.0 | 1591.0 | 22.75 | 26.52 |
| $60 \mathrm{E}-14^{\wedge}$ | 16.00 | 99 | 5088.0 | 27596.0 | 63.60 | 344.95 |
| $60 \mathrm{E}-15^{\wedge}$ | 2.00 | 7 | 0.0 | 905.0 | 0.00 | 90.50 |
| $60 \mathrm{E}-16^{\wedge}$ | 2.00 | 9 | 522.0 | 122.5 | 52.20 | 12.25 |
| $60 \mathrm{E}-17^{\wedge}$ | 11.00 | 12 | 180.0 | 3377.2 | 3.27 | 61.40 |
| $60 \mathrm{E}-18^{\wedge}$ | 7.00 | 8 | 0.0 | 780.0 | 0.00 | 22.29 |
| $60 \mathrm{E}-19^{*}$ | 6.00 | 12 | 420.0 | 609.0 | 14.00 | 20.30 |
| $60 \mathrm{E}-20^{\wedge}$ | 4.70 | 18 | 1761.0 | 167.0 | 74.94 | 7.11 |


| Segment | ITIS Category Description |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Closures |  | Incidents/Accidents |  | Incidents/Crashes |  | Obstruction Hazards |  | Winds |  | Winter Storm Codes |  |
|  | SB (or EB) | NB (or WB) | SB (or EB) | NB (or WB) | SB (or EB) | NB (or WB) | SB (or EB) | NB (or WB) | SB (or EB) | NB (or WB) | SB (or EB) | NB (or WB) |
| 191-1* | 5 | 2 | 0 | 0 | 5 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| 191-2* | 7 | 2 | 0 | 0 | 5 | 1 | 1 | 1 | 0 | 0 | 0 | 0 |
| 191-3^ | 2 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 191-4^ | 5 | 4 | 1 | 1 | 2 | 2 | 2 | 1 | 0 | 0 | 0 | 0 |
| 191-5* | 5 | 5 | 3 | 1 | 1 | 4 | 0 | 0 | 0 | 0 | 0 | 0 |
| 70-6* | 1 | 2 | 0 | 0 | 1 | 2 | 0 | 0 | 0 | 0 | 0 | 0 |
| 70-7^ | 6 | 2 | 1 | 0 | 4 | 2 | 1 | 0 | 0 | 0 | 0 | 0 |
| 70-8^ | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 70-9^ | 1 | 1 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
| 70-10^ | 7 | 5 | 1 | 0 | 6 | 5 | 0 | 0 | 0 | 0 | 0 | 0 |
| 70-11^ | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 70-12^ | 3 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 70/60E-13* | 13 | 16 | 2 | 4 | 8 | 9 | 0 | 1 | 0 | 0 | 1 | 1 |
| 60E-14^ | 52 | 47 | 6 | 6 | 43 | 28 | 2 | 7 | 0 | 0 | 0 | 6 |
| 60E-15^ | 0 | 7 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 4 |
| 60E-16^ | 6 | 3 | 2 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 3 |
| $60 \mathrm{E}-17^{\wedge}$ | 2 | 10 | 1 | 2 | 1 | 7 | 0 | 0 | 0 | 0 | 0 | 0 |
| 60E-18^ | 0 | 8 | 0 | 2 | 0 | 4 | 0 | 1 | 0 | 0 | 0 | 0 |
| 60E-19* | 3 | 9 | 1 | 2 | 1 | 5 | 0 | 0 | 0 | 0 | 0 | 0 |
| 60E-20^ | 16 | 2 | 3 | 1 | 11 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |

See the Mobility Performance Area Data section for other Freight Performance Area related data.

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Appendix D: Needs Analysis Contributing Factors and Scores

## Pavement Needs Assessment Methodology (Steps 1-3)

This section documents the approach for conducting the first three steps of a 5 -step needs assessment process for the Pavement Performance Area. After completion of Step 3 for all performance areas (Pavement, Bridge, Mobility, Safety, and Freight), Step 4 will review each corridor segment to quantify a total level of need that combines all performance areas. Corrido needs are then identified in Step 5 of the process. The 5 -step process is listed below:

- Step 1: Initial Needs
- Step 2: Final Needs
- Step 3: Contributing Factors
- Step 4: Segment Review
- Step 5: Corridor Needs


## Step 1: Initial Needs

The input required to populate the Step 1 template includes transferring the existing performance score for each segment to the appropriate "Performance Score" columns. This includes the primary and secondary measures for Pavement. As each performance score is input into the template, the Initial Need will populate based on the weighted scoring system for each measure.
The Level of Need for each performance measure has levels of "None" (score $=0$ ), "Low" (score =
1), "Medium" (score = 2), and "High" (score = 3). The assignment of these levels to individua performance measures for segments is determined by the table entitled "Needs Assessment Scales" within the Step 1 template.

To develop an aggregate Initial Need for each segment, the primary and secondary measures are combined by summing the weighted scored, with the primary measure having a weight of 1.0 while each secondary measure has a weight of 0.2 ( 0.1 each direction if directional). The Initia Need for each segment (combining the primary and secondary measures) has levels of "None" (score $<0.01$ ), "Low" (score $>0.01$ and $<1.5$ ), "Medium" (score $>1.5$ and $<2.5$ ), and "High" (score $\geq 2.5$ ).

The steps include:
Step 1.1
Enter the appropriate segment information into the columns titled "Segment", "Segment Length", Segment Mileposts" and "Facility Type".

## Step 1.2

Populate the Step 1 template with the existing (baseline) performance scores for all primary and secondary performance measures from Existing Performance Analysis into the appropriate "Performance Score" columns. Copy the performance score for each segment to the appropriate "Performance Score" column. Paste only the "values" and do not overwrite the formatting.

## Step 1.3

Indicate if Pavement is an Emphasis Area by selecting "Yes" or "No" in the row immediately below the segment information.
Step 1.4
Confirm that that the Step 1 template is generating the appropriate "Level of Need" for each primary and secondary measure by reviewing the relationship of baseline performance score to level of need.

## Step 2: Final Needs

The Initial Need will be carried over to Step 2. The steps required to complete Step 2 are as follows:

Step 2.1
Confirm that the template has properly populated the segment information and the initial needs from the Step 1 template to the "Initial Need" column of the Step 2 template.
Step 2.2
Note in the "Hot Spots" column any pavement failure hot spots identified as part of the baseline corridor performance. For each entry, include the milepost limits of the hot spot. Hot spots are identified in the Pavement Index spreadsheet by the red cells in the columns titled "\% Pavement Failure". These locations are based on the following criteria:
Interstates: IRI > 105 or Cracking > 10 or Rutting > 0.4
Non-Interstates: IRI > 142 or Cracking > 10 or Rutting > 0.4
Every segment that has a \% Pavement Failure greater than 0\% will have at least one hot spot. Hot spot locations should be described as extending over consecutive miles. For example, if there is a pavement failure location that extends 5 consecutive miles, it should be identified as one hot spot, not 5 separate hot spots.
Step 2.3
Identify recently completed or under construction paving projects in the "Previous Projects" column. Include only projects that were completed after the pavement condition data period (check dates in pavement condition data provided by ADOT) that would supersede the results of the performance system.

Step 2.5
Update the "Final Need" column using the following criteria:

- If "None" but have a hot spot (or hot spots), the Final Need = Low, and note the reason for the change in the "Comments" column (column H).
- If a recent project has superseded the performance rating data, change the Final Need to "None" and note the reason for the change in the "Comments" column.

| Pavement Index <br> (Interstates) <br> Performance <br> Thresholds |  | Initial Need | Description (Non-Emphasis Area) |
| :---: | :---: | :---: | :---: |
| 3.75 | Good | None | All of Good Performance and upper third of Fair Performance (>3.50) |
|  | Good |  |  |
|  | Good |  |  |
|  | Fair |  |  |
|  | Fair | Low | Middle third of Fair Perf. (3.25-3.5) |
| 3.0 | Fair | Medium | Lower third of Fair and top third of Poor |
|  | Poor |  | Performance (2.75-3.25) |
|  | Poor | High | Lower two-thirds of Poor Performance (<2.75) |
|  | Poor |  |  |

Need Scale for Interstates

| Measure | None >= | Low >= | $>$ Medium < |  | High <= |
| :--- | :---: | :---: | :---: | :---: | :---: |
| Pavement Index (corridor non-emphasis <br> area) | 3.5 | 3.25 | 3.25 | 2.75 | 2.75 |
| Pavement Index (corridor emphasis area) | 4.0 | 3.5 | 3.5 | 3.00 | 3.00 |
| Pavement Index (segments) | 3.5 | 3.25 | 3.25 | 2.75 | 2.75 |
| Directional PSR | 3.63 | 3.52 | 3.52 | 3.28 | 3.28 |
| \%Pavement Failure | $10 \%$ | $15 \%$ | $15 \%$ | $25 \%$ | $25 \%$ |

Need Scale for Highways (Non-Interstates)

| Measure | None >= | Low >= | $>$ Medium < |  | High <= |
| :--- | :---: | :---: | :---: | :---: | :---: |
| Pavement Index (corridor non-emphasis <br> area) | 3.33 | 3.07 | 3.07 | 2.53 | 2.53 |
| Pavement Index (corridor emphasis area) | 3.87 | 3.33 | 3.33 | 2.80 | 2.80 |
| Pavement Index (segments) | 3.33 | 3.07 | 3.07 | 2.53 | 2.53 |
| Directional PSR | 3.30 | 3.10 | 3.10 | 2.70 | 2.70 |
| \%Pavement Failure | $10 \%$ | $15 \%$ | $15 \%$ | $25 \%$ | $25 \%$ |

## Step 2.6

Note any programmed projects that could have the potential to mitigate pavement needs in in the "Comments" column. Programmed projects are provided as information and do not impact the need rating. The program information can be found in ADOT's 5 -year construction program. If there are other comments relevant to the needs analysis (such as information from previous reports), they can be entered in the "Comments" column. However, only include information related to needs that have been identified through this process. Do not add or create needs from other sources.

## Step 3: Contributing Factors

The Final Need ratings from Step 2 will populate into the Step 3 tab. The steps to complete Step 3 include:
Step 3.1
Input the level of historical investment for each segment. This will be determined from the numeric score from the Pavement History Table based on the following thresholds:

- Low $=<4.60$
- Medium $=4.60-6.60$
- High $=>6.60$

If the PeCoS data shows a high level of maintenance investment, increase the historical investment rating by one level.
Step 3.2
Note the milepost ranges of pavement failure hot spots into the column titled "Contributing Factors and Comments."
Step 3.3
Note any other information that may be contributing to the deficiency, or supplemental information, in the "Contributing Factors and Comments" column. This could come from discussions with ADOT District staff, ADOT Materials/Pavement Group, previous reports, or the historical investment data.

Step 3.4
Include any programmed projects from ADOT's 5-year construction program in the "Contributing Factors and Comments" column.

## Bridge Needs Assessment Methodology (Steps 1-3)

This section documents the approach for conducting the first three steps of a 5 -step needs assessment process for the Bridge Performance Area. After completion of Step 3 for all performance areas (Pavement, Bridge, Mobility, Safety, and Freight), Step 4 will review each corridor segment to quantify a total level of need that combines all performance areas. Corridor needs are then identified in Step 5 of the process. The 5 -step process is listed below:

- Step 1: Initial Needs
- Step 2: Final Needs
- Step 3: Contributing Factors
- Step 4: Segment Review
- Step 5: Corridor Needs


## Step 1: Initial Needs

The input required to populate the Step 1 template includes transferring the existing performance score for each segment to the appropriate "Performance Score" columns. This includes the primary and secondary measures for Bridge. As each performance score is input into the template, the Initial Need will populate based on the weighted scoring system for each measure.
The Level of Need for each performance measure has levels of "None" (score $=0$ ), "Low" (score $=$
1), "Medium" (score = 2), and "High" (score = 3). The assignment of these levels to individual performance measures for segments is determined by the table entitled "Needs Assessment Scales" within the Step 1 template.

To develop an aggregated Initial Need for each segment, the primary and secondary measures are combined by summing the weighted scored, with the primary measure having a weight of 1.0 while each secondary measure has a weight of 0.2 ( 0.1 each direction if directional). The Initial level of need for each segment (combining the primary and secondary measures) has levels of "None" (score $<0.01$ ), "Low" (score $\geq 0.01$ and $<1.5$ ), "Medium" (score $\geq 1.5$ and $<2.5$ ), and "High" (score $\geq 2.5$ ).
The steps include:
Step 1.1
Enter the appropriate segment information into the columns titled "Segment", "Segment Length", "Segment Mileposts" and "Number of Bridges."

## Step 1.2

Populate the Step 1 template with the existing (baseline) performance scores for all primary and secondary performance measures from Existing Performance Analysis into the appropriate "Performance Score" columns. Copy the performance score for each segment to the appropriate "Performance Score" column. Paste only the "values" and do not overwrite the formatting.

## Step 1.3

Indicate if Bridge is an Emphasis Area by selecting "Yes" or "No" in the row immediately below the segment information.
Step 1.4
Confirm that that the Step 1 template is generating the appropriate "Level of Need" for each primary and secondary measure by reviewing the relationship of baseline performance score to level of need.

## Step 2: Final Needs

The Initial Need will be carried over to Step 2. The steps required to complete Step 2 are as follows:
Step 2.1
Confirm that the template has properly populated the initial needs from the Step 1 template to the "Initial Need" column of the Step 2 template
Step 2.2
Note in the column titled "Hot Spots" any bridge hot spots identified as part of the baseline corrido performance. For each entry, note the specific location. Hot spots are identified as having any bridge rating of 4 or less, or multiple ratings of 5 in the deck, substructure, or superstructure ratings.
Step 2.3
Identify recently completed or under construction bridge projects in the "Previous Projects" column. Include only projects that were completed after the bridge condition data period (check dates in bridge condition data provided by ADOT) that would supersede the results of the performance system.
Step 2.4
Update the Final Need on each segment based on the following criteria:

- If the Initial Need is "None" and there is at least one hot spot located on the segment, change the Final Need to "Low".
- If a recent project has superseded the performance rating data, the performance data should be adjusted to increase the specific ratings and the resulting need should be reduced to account for the project.
- Note the reason for any change in the "Comments" column.


## Step 2.5

Historical bridge rating data was tabulated and graphed to find any bridges that had fluctuations in the ratings. Note in the "Historical Review" column any bridge that was identified as having a potential historical rating concern based on the following criteria:

- Ratings increase or decrease (bar chart) more than 2 times
- Sufficiency rating drops more than 20 points

This is for information only and does not affect the level of need.
Step 2.6
Note the number of functionally obsolete bridges in each segment in the column titled "\# Functionally Obsolete Bridges". This is for information only and does not affect the level of need. Step 2.7

Identify each bridge "of concern" in the "Comments" column. Note any programmed projects that could have the potential to mitigate bridge needs. Programmed projects are provided as information and do not impact the need rating. The program information can be found in ADOT's 5 -year construction program. If there are other comments relevant to the needs analysis (such as information from previous reports), they can be entered in the "Comments" column. However, only include information related to needs that have been identified through this process. Do not add or create needs from other sources.

## Example Scales for Level of Need

| Bridge Index Performance Thresholds | Level of Need |  | Description (Non-Emphasis Area) |
| :---: | :---: | :---: | :---: |
| 6.5 | Good | None | All of Good Performance and upper third of Fair Performance (>6.0) |
|  | Good |  |  |
|  | Good |  |  |
|  | Fair |  |  |
|  | Fair | Low | Middle third of Fair Performance (5.5-6.0) |
| 5.0 | Fair | Medium | Lower third of Fair and top third of Poor |
|  | Poor |  | Performance (4.5-5.5) |
|  | Poor | High | Lower two-thirds of Poor Performance$(<4.5)$ |
|  | Poor |  |  |

## Mobility Needs Assessment Methodology (Steps 1-3)

This section documents the approach for conducting the first three steps of a 5 -step needs assessment process for the Mobility Performance Area. After completion of Step 3 for all performance areas (Pavement, Bridge, Mobility, Safety, and Freight), Step 4 will review each corridor segment to quantify a total level of need that combines all performance areas. Corridor needs are then identified in Step 5 of the process. The 5 -step process is listed below:

- Step 1: Initial Needs
- Step 2: Refined Needs
- Step 3: Contributing Factors
- Step 4: Segment Review
- Step 5: Corridor Needs


## Step 1: Initial Needs

The input required to populate the Step 1 template includes transferring the existing performance score for each segment to the appropriate "Performance Score" columns from Existing Performance Analysis. This includes the primary and secondary measures for Mobility. As each performance score is input into the template, the Initial Need will populate based on the weighted scoring system for each measure.
The Level of Need for each performance measure has levels of "None" (score = 0), "Low" (score = 1), "Medium" (score $=2$ ), and "High" (score = 3). The assignment of these levels to individual performance measures for segments is determined by the table entitled "Needs Assessment Scales" in the Step 1 tab.

To develop an aggregated Initial Need for each segment, the primary and secondary measures are combined by summing the weighted scores, with the primary measure having a weight of 1.0 while each secondary measure has a weight of 0.2 ( 0.1 each direction if directional). The Initial Need for each segment (combining the primary and secondary measures) has levels of "None" (score < 0.01), "Low" (score $\geq 0.01$ and $<1.5$ ), "Medium" (score $\geq 1.5$ and $<2.5$ ), and "High" (score $\geq 2.5$ ).

The steps include:
Step 1.1
Input the accurate number of segments for your corridor in the column titled 'Segment' and the appropriate segment milepost limits and segment lengths in adjacent columns.
Step 1.2
Select the appropriate 'Environment Type' and 'Facility Operation Type' from the drop-down menus as defined in Existing Performance Analysis.

## Step 1.3

Select 'Yes' or 'No' from the drop-down list to not if the Mobility Performance Area is an Emphasis Area for your corridor.
Step 1.4
Populate the Step 1 template with the existing (baseline) performance scores for all primary and secondary performance measures from Existing Performance Analysis. Copy the performance score for each segment to the appropriate "Performance Score" column.
Step 1.5
Confirm that that the Step 1 template is generating the appropriate "Level of Need" for each primary and secondary measure by reviewing the relationship of baseline performance score to level of need.

## Step 2: Final Needs

The Initial Need will be carried over to Step 2 The steps required to complete Step 2 are as follows:

Step 2.1
Confirm that the template has properly populated the initial deficiencies from the Step 1 template to the Step 2 template.
Step 2.2
Identify recently completed or under construction projects that would be considered relevant to mobility performance. Include only projects that were constructed after the date for which the HPMS data used for traffic volumes would not include. Any completed or under construction roadway project after the HPMS data date that has the potential to mitigate a mobility issue on a corridor segment should be listed in the template. Such projects should include the construction of new travel lanes or speed limit changes on the main corridor only. Do not include projects involving frontage roads or crossings as they would not impact the corridor level performance.

Step 2.3
Update the Final Need using the following criteria:

- If a recent project has superseded the performance rating data and it is certain the project addressed the deficiency, change the need rating to "None".
- If a recent project has superseded the performance rating data but it is uncertain that a project addressed the need, maintain the current deficiency rating and note the uncertainty as a comment.

Step 2.4
Note any programmed or planned projects that have the potential to mitigate any mobility needy on the segment. Programmed and Planned projects are provided as information and do not impact the deficiency rating. Future projects will be reviewed in the development of solution sets for identified needs and deficiencies. The source of future projects can be found in ADOT's 5-year construction program or other planning documents. Other comments relevant to the needs analysis can be entered.

| Mobility Index (Urban and Fringe Urban) <br> Performance Thresholds | Initial Need |  | Description (Non-Emphasis Area) |
| :---: | :---: | :---: | :---: |
| 0.71 | Good | None | All of Good Performance and upper third of Fair Performance(<0.77) |
|  | Good |  |  |
|  | Good |  |  |
|  | Fair |  |  |
|  | Fair | Low | Middle third of Fair Performance (0.77-0.83) |
| 0.89 | Fair | Medium | Lower third of Fair and top third of Poor Performance (0.83- |
|  | Poor |  | $0.95)$ |
|  | Poor | High | Lower two-thirds of Poor Performance (>0.95) |
|  | Poor |  |  |


| Measure |  | None <= | Low <= | > M | m < | High >= |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mobility Index (Corridor Emphasis Area) |  | Weighted calculation for the segment totals in corridor (urban vs. rural) |  |  |  |  |
| Mobility Index (Corridor NonEmphasis Area) |  | Weighted calculation for the segment totals in corridor (urban vs. rural) |  |  |  |  |
| Mobility Index (Segment) | Urban | 0.77 | 0.83 | 0.83 | 0.95 | 0.95 |
|  | Rural | 0.63 | 0.69 | 0.69 | 0.83 | 0.83 |
| Future Daily V/C | Urban | 0.77 | 0.83 | 0.83 | 0.95 | 0.95 |
|  | Rural | 0.63 | 0.69 | 0.69 | 0.83 | 0.83 |
| Existing Peak Hour V/C | Urban | 0.77 | 0.83 | 0.83 | 0.95 | 0.95 |
|  | Rural | 0.63 | 0.69 | 0.69 | 0.83 | 0.83 |
| Closure Extent |  | 0.35 | 0.49 | 0.49 | 0.75 | 0.75 |
| Directional LOTTR | Uninterrupted | 1.27 | 1.38 | 1.38 | 1.62 | 1.62 |
|  | Interrupted | 1.27 | 1.38 | 1.38 | 1.62 | 1.62 |
| Bicycle Accommodation |  | 80\% | 70\% | 70\% | 50\% | 50\% |

## Step 3: Contributing Factors

The Final Need ratings from Step 2 will populate into the Step 3 tab. The steps to compete Step 3 include:
Step 3.1
Input data from Mobility Index worksheet and corridor observations in appropriate columns for Roadway Variables.
Step 3.2
Input traffic variable data in appropriate columns as indicated, Buffer Index scores will auto populate.

Step 3.3
Input relevant mobility related infrastructure located within each segment as appropriate Step 3.4
Input the Closure Extents that have occurred along the study corridor. Road closure information can be detailed out by the reason for the closure as documented in Highway Condition Reporting System (HCRS) data analyzed as part of the baseline corridor performance. Closure reasons include incident/accidents, winter storms, obstruction hazards, and undefined closures. Statewide average percentages for the various closure reasons have been calculated for most recent fiveyear period on ADOT's designated strategic corridors. Compare these statewide average percentages to the corridor percentages for the various closure reasons to identify higher than average percentages of one or more closure reasons on any given segment. Input the closures as follows and use red text to indicate that the segment percentage exceeds statewide averages:

- Total Number of Closures
- \% Incidents/Accidents
- \% Obstructions/Hazards
- \% Weather Related

Step 3.5
List the non-actionable conditions that are present within each segment by milepost if possible. Non-Actionable conditions are conditions that exist within the environment of each segment that cannot be improved through an engineered solution. For example, the border patrol check point in Segment 3 of $\mathrm{I}-19$ is a non-actionable condition.

Step 3.6
Considering all information input, identify and list the contributing factors to the Final Need score.

## Safety Needs Assessment Methodology (Steps 1-3)

This section documents the approach for conducting the first three steps of a 5 -step needs assessment process for the Safety Performance Area. After completion of Step 3 for all performance areas (Pavement, Bridge, Mobility, Safety, and Freight), Step 4 will review each corridor segment to quantify a total level of need that combines all performance areas. Corridor needs are then identified in Step 5 of the process. The 5 -step process is listed below:

- Step 1: Initial Needs
- Step 2: Final Needs
- Step 3: Contributing Factors
- Step 4: Segment Review
- Step 5: Corridor Needs


## Step 1: Initial Needs

The input required to populate the Step 1 template includes transferring the corridor characteristics and existing performance score for each segment to the appropriate "Performance Score" columns. This includes the primary and secondary measures for safety. As each performance score is input into the template, the Level of Need will populate based on the weighted scoring system for each measure.
The Level of Need for each performance measure has levels of "None" (score = 0), "Low" (score = 1), "Medium" (score $=2$ ), and "High" (score = 3). The assignment of these levels to individual performance measures for segments is determined by the table entitled "Needs Scale" within the Step 1 template.
To develop an aggregated Initial Need for each segment, the primary and secondary measures are combined by summing the weighted scored, with the primary measure having a weight of 1.0 while each secondary measure has a weight of 0.2 ( 0.1 each direction if directional). The Initial Need for each segment (combining the primary and secondary measures) has levels of "None" (score < 0.01), "Low" (score $\geq 0.01$ and $<1.5$ ), "Medium" (score $\geq 1.5$ and $<2.5$ ), and "High" (score $\geq 2.5$ ).
The steps include:
Step 1.1
Populate the Step 1 template with the corridor characteristics information. This includes segment operating environments and segment length. Also, specify if the safety performance area is an emphasis area as determined in Goals and Objectives. The "Level of Need" is dependent on the input of the operating environment and "Emphasis Area" as the thresholds dynamically update accordingly.

Input the existing (baseline) performance scores for all primary and secondary performance measures from Existing Performance Analysis. Copy the performance score (paste values only) for each segment to the appropriate "Performance Score" column and conditional formatting should color each cell green, yellow, or red based on the corresponding performance thresholds. Step 1.2
The thresholds for the corridor safety index are based on the segments' operating environments. To ensure that the correct corridor safety index threshold is applied, input the unique segment operating environments that exist with the corridor. Once the input is complete, the average of the Good/Fair and Fair/Poor thresholds for each of the operating environments is calculated and the "Level of Need" thresholds will be derived and applied to the main Step 1 Table.
Step 1.3
Confirm that the following criteria for "Insufficient Data" have been applied and that the resulting Level of Need has been shown as "N/A" where applicable.

- Crash frequency for a segment is less than 5 crashes over the 5 -year crash analysis period.
- The change in $+/-1$ crash results in the change of need level of 2 levels (i.e., changes from Above Average to Below Average or changes from Below Average to Above Average).
- The average segment crash frequency for the overall corridor (total fatal plus suspected serious injury crash frequency divided by the number of corridor segments) is less than 2 per segment over the 5 -year crash analysis period.

Step 1.4
Confirm that the Step 1 template is generating the appropriate "Level of Need" for each primary and secondary measure by reviewing the relationship of baseline performance score to level of need.

## Step 2: Final Needs

The Initial Need will be carried over to Step 2. The steps required to complete Step 2 are as follows:
Step 2.1
Confirm that the template has properly populated the initial needs from the Step 1 template to the Step 2 template.

Step 2.2
Using the crash concentration (hot spot) map developed as part of the baseline corridor performance, note the direction of travel and approximate milepost limits of each hot spot.

Step 2.3
Identify recently completed or under construction projects that would be considered relevant to safety performance. Include only projects that were not taken into account during the five-year crash data analysis period. Any completed or under construction roadway project after the crash analysis period that has the potential to mitigate a safety issue on a corridor segment should be listed in the template. Sources of recent or current project activity can include ADOT MPD staff, ADOT public notices, and ADOT District staff.

Step 2.4
Update the Final Need based on the following criteria:

- If there is a crash hot spot concentration on a "None" segment, upgrade the need rating to "Low."


## Step 2.5

Note any programmed projects that could have the potential to mitigate any safety need on the segment. Programmed projects are provided as information and do not impact the need rating Programmed projects will be reviewed in the development of solution sets for identified needs. The source of the programming information can be found in ADOT's 5 -year construction program Any other relevant issues identified in previous reports should also be reported.

| Example Scales for Level of Need |
| :--- |
| Safety Index (6 Lane <br> Highway) Performance <br> Thresholds Initial Need  Description (Non-Emphasis Area) |
| 0.30 |


| Measure |  | None < $=$ | Low < | < Medium > |  | High >= | Good/Fair Threshold | Fair/Poor Threshold |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Corridor Safety Index (Emphasis Area) |  | Weighted average based on operating environment type |  |  |  |  |  |  |
| Corridor Safety Index (Non-Emphasis Area) |  | \# Weighted average based on operating environment type |  |  |  |  | 0.92 | 1.08 |
| Safety Index and Directional Safety Index (Segment) | 2 or 3 Lane Undivided Highway | 0.97 | 1.02 | 1.02 | 1.13 | 1.13 | 0.92 | 1.08 |
|  | 2 or 3 or 4 Lane Divided Highway | 0.94 | 1.07 | 1.07 | 1.32 | 1.32 | 0.81 | 1.19 |
|  | 4 or 5 Lane Undivided Highway | 0.93 | 1.08 | 1.08 | 1.37 | 1.37 | 0.78 | 1.22 |
|  | 6 Lane Highway | 0.92 | 1.08 | 1.08 | 1.4 | 1.4 | 0.76 | 1.24 |
|  | Rural 4 Lane Freeway with Daily Volume < 25,000 | 0.95 | 1.06 | 1.06 | 1.27 | 1.27 | 0.84 | 1.16 |
|  | Rural 4 Lane Freeway with Daily Volume $>25,000$ | 0.93 | 1.08 | 1.08 | 1.37 | 1.37 | 0.78 | 1.22 |
|  | Urban 4 Lane Freeway | 0.91 | 1.09 | 1.09 | 1.45 | 1.45 | 0.73 | 1.27 |
|  | Urban or Rural 6 Lane Freeway | 0.88 | 1.11 | 1.11 | 1.58 | 1.58 | 0.65 | 1.35 |
|  | Urban > 6 Lane Freeway | 0.96 | 1.03 | 1.03 | 1.18 | 1.18 | 0.89 | 1.11 |
| \% of Fatal + Susp. <br> Serious Injury <br> Crashes at <br> Intersections | 2 or 3 Lane Undivided Highway | 13\% | 14\% | 14\% | 17\% | 17\% | 11\% | 16\% |
|  | 2 or 3 or 4 Lane Divided Highway | 25\% | 27\% | 27\% | 31\% | 31\% | 23\% | 29\% |
|  | 4 or 5 Lane Undivided Highway | 46\% | 48\% | 48\% | 52\% | 52\% | 44\% | 50\% |
|  | 6 Lane Highway | 63\% | 68\% | 68\% | 78\% | 78\% | 58\% | 73\% |
|  | Rural 4 Lane Freeway with Daily Volume < 25,000 | 0\% | 0\% | 0\% | 0\% | 0\% | 0\% | 0\% |
|  | Rural 4 Lane Freeway with Daily Volume $>25,000$ | 0\% | 0\% | 0\% | 0\% | 0\% | 0\% | 0\% |
|  | Urban 4 Lane Freeway | 0\% | 0\% | 0\% | 0\% | 0\% | 0\% | 0\% |
|  | Urban or Rural 6 Lane Freeway | 0\% | 0\% | 0\% | 0\% | 0\% | 0\% | 0\% |
|  | Urban > 6 Lane Freeway | 0\% | 0\% | 0\% | 0\% | 0\% | 0\% | 0\% |
| \% of Fatal + Susp. <br> Serious Injury <br> Crashes Involving <br> Lane Departures | 2 or 3 Lane Undivided Highway | 69\% | 72\% | 72\% | 77\% | 77\% | 67\% | 75\% |
|  | 2 or 3 or 4 Lane Divided Highway | 59\% | 62\% | 62\% | 68\% | 68\% | 56\% | 65\% |
|  | 4 or 5 Lane Undivided Highway | 25\% | 29\% | 29\% | 36\% | 36\% | 21\% | 32\% |
|  | 6 Lane Highway | 21\% | 30\% | 30\% | 47\% | 47\% | 12\% | 38\% |
|  | Rural 4 Lane Freeway with Daily Volume < 25,000 | 74\% | 75\% | 75\% | 78\% | 78\% | 73\% | 76\% |
|  | Rural 4 Lane Freeway with Daily Volume $>25,000$ | 72\% | 75\% | 75\% | 81\% | 81\% | 69\% | 78\% |
|  | Urban 4 Lane Freeway | 66\% | 72\% | 72\% | 84\% | 84\% | 61\% | 78\% |
|  | Urban or Rural 6 Lane Freeway | 58\% | 60\% | 60\% | 65\% | 65\% | 56\% | 63\% |
|  | Urban > 6 Lane Freeway | 41\% | 42\% | 42\% | 44\% | 44\% | 40\% | 43\% |
| \% of Fatal + Susp. <br> Serious Injury Crashes Involving Pedestrians | 2 or 3 Lane Undivided Highway | 5\% | 6\% | 6\% | 8\% | 8\% | 4\% | 7\% |
|  | 2 or 3 or 4 Lane Divided Highway | 3\% | 3\% | 3\% | 4\% | 4\% | 2\% | 4\% |
|  | 4 or 5 Lane Undivided Highway | 10\% | 12\% | 12\% | 15\% | 15\% | 9\% | 14\% |
|  | 6 Lane Highway | 4\% | 8\% | 8\% | 16\% | 16\% | 0\% | 12\% |
|  | Rural 4 Lane Freeway with Daily Volume < 25,000 | 2\% | 3\% | 3\% | 4\% | 4\% | 1\% | 3\% |
|  | Rural 4 Lane Freeway with Daily Volume $>25,000$ | 2\% | 3\% | 3\% | 6\% | 6\% | 1\% | 5\% |
|  | Urban 4 Lane Freeway | 2\% | 4\% | 4\% | 7\% | 7\% | 0\% | 5\% |
|  | Urban or Rural 6 Lane Freeway | 5\% | 6\% | 6\% | 9\% | 9\% | 4\% | 8\% |
|  | Urban > 6 Lane Freeway | 3\% | 4\% | 4\% | 6\% | 6\% | 2\% | 5\% |


| Measure |  | None <= | Low < | < Medium > |  | High >= | Good/Fair <br> Threshold | Fair/Poor <br> Threshold |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Corridor Safety Index (Emphasis Area) |  | Weighted average based on operating environment type |  |  |  |  |  |  |
| Corridor Safety Index (Non-Emphasis Area) |  | \# Weighted average based on operating environment type |  |  |  |  | 0.92 | 1.08 |
| \% of Fatal + Susp. <br> Serious Injury Crashes Involving Trucks | 2 or 3 Lane Undivided Highway | 5\% | 6\% | 6\% | 9\% | 9\% | 4\% | 8\% |
|  | 2 or 3 or 4 Lane Divided Highway | 6\% | 8\% | 8\% | 12\% | 12\% | 4\% | 10\% |
|  | 4 or 5 Lane Undivided Highway | 2\% | 4\% | 4\% | 7\% | 7\% | 1\% | 6\% |
|  | 6 Lane Highway | 5\% | 6\% | 6\% | 8\% | 8\% | 4\% | 8\% |
|  | Rural 4 Lane Freeway with Daily Volume < 25,000 | 20\% | 21\% | 21\% | 24\% | 24\% | 19\% | 23\% |
|  | Rural 4 Lane Freeway with Daily Volume $>25,000$ | 12\% | 15\% | 15\% | 22\% | 22\% | 9\% | 18\% |
|  | Urban 4 Lane Freeway | 9\% | 11\% | 11\% | 15\% | 15\% | 7\% | 12\% |
|  | Urban or Rural 6 Lane Freeway | 8\% | 11\% | 11\% | 16\% | 16\% | 5\% | 13\% |
|  | Urban > 6 Lane Freeway | 3\% | 4\% | 4\% | 6\% | 6\% | 2\% | 5\% |
| \% of Fatal + Susp. <br> Serious Injury Crashes Involving Bicycles | 2 or 3 Lane Undivided Highway | 1\% | 2\% | 2\% | 4\% | 4\% | 0\% | 3\% |
|  | 2 or 3 or 4 Lane Divided Highway | 1\% | 2\% | 2\% | 3\% | 3\% | 0\% | 2\% |
|  | 4 or 5 Lane Undivided Highway | 2\% | 3\% | 3\% | 5\% | 5\% | 1\% | 4\% |
|  | 6 Lane Highway | 2\% | 4\% | 4\% | 9\% | 9\% | 0\% | 7\% |
|  | Rural 4 Lane Freeway with Daily Volume < 25,000 | 0\% | 0\% | 0\% | 1\% | 1\% | 0\% | 1\% |
|  | Rural 4 Lane Freeway with Daily Volume > 25,000 | 0\% | 0\% | 0\% | 0\% | 0\% | 0\% | 0\% |
|  | Urban 4 Lane Freeway | 0\% | 0\% | 0\% | 0\% | 0\% | 0\% | 0\% |
|  | Urban or Rural 6 Lane Freeway | 0\% | 0\% | 0\% | 1\% | 1\% | 0\% | 1\% |
|  | Urban > 6 Lane Freeway | 0\% | 0\% | 0\% | 0\% | 0\% | 0\% | 0\% |

## Step 3: Contributing Factors

The Final Need ratings from Step 2 will populate into the Step 3 tab.

## Table 3 - Step 3 Template

A separate Crash Summary Sheet file contains summaries for 8 crash attributes for the entire corridor, for each corridor segment, and for statewide roadways with similar operating environments (the database of crashes on roadways with similar operating environments was developed in Existing Performance Analysis (the baseline corridor performance)). The crash attribute summaries are consistent with the annual ADOT Publication, Crash Facts. The 8 crash attribute summaries consist of the following:

- First Harmful Event (FHET)
- Crash Type (CT)
- Violation or Behavior (VB)
- Lighting Condition (LC)
- Roadway Surface Type (RST)
- First Unit Event (FUE)
- Driver Physical Condition (Impairment)
- Safety Device Usage (Safety Device)

Non-colored tabs in this spreadsheet auto-populate with filtered crash attributes. Each tab is described below:

- Step_3_Summary - This tab contains the filtered summary of crashes that exceed statewide thresholds for crashes on roadways with similar operating environments. Data in this tab are copied into the Step 3 template.
- Statewide - This tab contains a summary of statewide crashes from roadways with similar operating environments filtered by the 8 crash type summaries listed above. The crash type summaries calculate statewide crash thresholds (\% total for fatal plus incapacitating crashes). The crash thresholds were developed to provide a statewide expected proportion of crash attributes against which the corridor segments' crash attributes can be compared. The crash thresholds were developed using the Probability of Specific Crash Types Exceeding a Threshold Proportion as shown in the Highway Safety Manual, Volume 1 (2010). The thresholds are automatically calculated within the spreadsheet. The threshold proportion was calculated as follows:

$$
p *_{i}=\frac{\sum N_{\text {Observed }, i}}{\sum N_{\text {Observed }, i(\text { total })}}
$$

Where:
= Threshold proportion
$\sum N_{\text {Observed }, i}$
= Sum of observed target crash frequency within the population
$\sum N_{\text {Observed, } i(\text { total })}=$ Sum of total observed crash frequency within the population
A minimum crash sample size of 5 crashes over the 5 -year crash analysis period is required for a threshold exceedance to be displayed in the Step 3 template. The probability of exceeding the crash threshold was not calculated to simplify the process.

- Corridor - A summary of corridor-wide crashes filtered by the 8 crash attribute summaries listed above.
- Segment FHET - A segment-by-segment summary of crashes filtered by first harmful event attributes.
- Segment CT - A segment-by-segment summary of crashes filtered by crash type attributes.
- Segment VB - A segment-by-segment summary of crashes filtered by violation or behavior attributes.
- Segment LC - A segment-by-segment summary of crashes filtered by lighting condition attributes.
- Segment RST - A segment-by-segment summary of crashes filtered by roadway surface attributes.
- Segment FUE - A segment-by-segment summary of crashes filtered by first unit event attributes.
- Segment Impairment - A segment-by-segment summary of crashes filtered by driver physical condition attributes related to impairment.
- Segment Safety Device - A segment-by-segment summary of crashes filtered by safety device usage attributes.

The steps to compete Step 3 include:
Step 3.1
Using the Crash_Summary_Sheet.xlsx, go to the "Step_3_Summary" tab. Input the operating environments for each segment in the table.

## Step 3.2

Filter data from the ADOT database for the "CORRIDOR_DATA" tab by inserting the following data in the appropriate columns that are highlighted in gray for the "INPUT_CORRIDOR_DATA" tab:

- Incident ID
- Incident Crossing Feature (MP)
- Segment Number (Non-native ADOT data - must be manually assigned based on the location of the crash)
- Operating Environment (Non-native ADOT data - should already be assigned but if for some reason it isn't, it will need to be manually assigned)
- Incident Injury Severity
- Incident First Harmful Description
- Incident Collision Manner
- Incident Lighting Condition Description
- Unit Body Style
- Surface Condition
- First Unit Event Sequence
- Person Safety Equipment
- Personal Violation or Behavio
- Impairment

Note that columns highlighted in yellow perform a calculated input to aggregate specific crash descriptions. For example, crashes can contain various attributes for animal-involved crashes. The crash attributes that involve an animal were combined into a common attribute, such as "ANIMAL". This will allow the summaries to be consistent with the ADOT Crash Facts.
The data in the Impairment category contains blank descriptions if it was found that there was "No Apparent Influence" or if it was "Unknown". Using the crash data fields
"PersonPhysicalDescription" 0-99, fill in the blank columns to reflect if the physical description is described as "No Apparent Influence" or "Unknown". Note that the native physical description data from the ADOT database may need to be combined to a single column.
Step 3.3
Confirm that the crash database is being properly filtered by comparing crash frequencies from the summary tables with the frequencies developed in Existing Performance Analysis. For example, the lookup function will fail if the filter is for "NO IMPROPER ACTION" if the database has the attribute of "NO_IMPROPER_ACTION".
Step 3.4
Copy and paste the Step_3_Summary into the Safety Needs Assessment spreadsheet in the Step 3 tab. Paste values only and remove the summaries with " $0 \% \mathrm{~s}$ " for a clean display. Where duplicate values exist, go to the "Calcs" tab in the Crash_Summary_Sheet file to determine which categories have the same \%. If there are more crash types with the same \% than there is space in the table, select the crash type with the highest difference between the segment \% and the statewide average \%
Step 3.5
The Step 3 table in the Safety Needs Assessment spreadsheet should be similar to the Step 3 template. In the Segment Crash Summaries row, the top three crash attributes are displayed. Change the font color of the crash attributes that exceed the statewide crash threshold to red for emphasis. The attributes with a red font in the "Calcs" tab have exceeded statewide crash thresholds. Note that corridor-wide values are not compared to statewide values as corridor-
wide values are typically a blend of multiple similar operating environments while the statewide values apply to one specific similar operating environment.

## Step 3.6

Provide a summary of any observable patterns found within the crash Hot Spots, if any exist in the segments
Step 3.7
Input any historic projects (going no further back than 2000) that can be related to improving safety. Projects more than five years old may have exceeded their respective design life and could be contributing factors to safety performance needs.

## Step 3.8

Input key points from District interviews or any important information from past discussions with District staff that is consistent with needs and crash patterns identified as part of the performance and needs assessment as this may be useful in identifying contributing causes. This information may be obtained from District Maintenance personnel by requesting the mile post locations that may be considered safety issues.

Step 3.9
For segments with one or more of the following characteristics, review crashes of all severity levels (not just fatal and incapacitating injury crashes). Identify likely contributing factors and compare that to the above statewide average comparison findings already calculated for fatal and incapacitating injury crashes. Refine the contributing factors list accordingly

- Segments with Medium or High need
- Segments with a crash hot spot concentration (but only review crashes at the concentration areas)
- Segments with no apparent predominant contributing factors based on the comparison of fatal and incapacitating crashes to statewide averages if the segment has a Medium or High need.

Step 3.10
Considering all information in Steps 1-3, list the contributing factors using engineering judgment and the information on contributing factors available in Section 6.2 of the 2010 Highway Safety Manual. Additional sources for determining contributing factors may include aerial, "street view", and/or ADOT photologs. Other documents such as Design Concept Reports (DCR) or Road Safety Assessments can provide insight into the study corridor's contributing factors.
Add comments as needed on additional information related to contributing factors that may have been provided by input from ADOT staff.

## Freight Needs Assessment Methodology (Steps 1-3)

This section documents the approach for conducting the first three steps of a 5 -step needs assessment process for the Freight Performance Area. After completion of Step 3 for all performance areas (Pavement, Bridge, Mobility, Safety, and Freight), Step 4 will review each corridor segment to quantify a total level of need that combines all performance areas. Corridor needs are then identified in Step 5 of the process. The 5 -step process is listed below:

- Step 1: Initial Needs
- Step 2: Final Needs
- Step 3: Contributing Factors
- Step 4: Segment Review
- Step 5: Corridor Needs


## Step 1: Initial Needs

The input required to populate the Step 1 template includes transferring the existing performance score and color for each segment to the appropriate "Performance Score" columns. This includes the primary and secondary measures for Freight. As each performance score is input into the template, the Initial Need will populate based on the weighted scoring system for each measure.
The Level of Need for each performance measure has levels of "None" (score = 0), "Low" (score =
1), "Medium" (score = 2), and "High" (score = 3). The assignment of these levels to individual performance measures for segments is determined by the table entitled "Needs Assessment Scale" within the Step 1 template.

To develop an aggregated Initial Need for each segment, the primary and secondary measures are combined by summing the weighted score, with the primary measure having a weight of 1.0 while each secondary measure has a weight of 0.2 ( 0.1 each direction if directional). The Initial Need for each segment (combining the primary and secondary measures) has levels of "None" (score $<0.01$ ), "Low" (score $\geq 0.01$ and $<1.5$ ), "Medium" (score $\geq 1.5$ and $<2.5$ ), and "High" (score $\geq 2.5$ ).
The steps include:
Step 1.1
Populate the Step 1 template with the existing (baseline) performance scores for all primary and secondary performance measures from Existing Performance Analysis. Copy the performance score for each segment to the appropriate "Performance Score" column. Select the Facility Operations for each segment from the drop-down list and input whether or not the performance area is an emphasis area. The corridor needs assessment scales will be updated automatically

Step 1.2
Confirm that that the Step 1 template is generating the appropriate "Level of Need" for each primary and secondary measure by reviewing the relationship of baseline performance score to level of need.

## Step 2: Final Needs

The Initial Need will be carried over to Step 2. The steps required to complete Step 2 are as follows:

Step 2.1
Confirm that the template has properly populated the initial need from the Step 1 template to the Step 2 template

## Step 2.2

Note any truck height restriction hot spots (clearance < 16.25') identified as part of the baseline corridor performance. For each entry, note the milepost of the height restriction and if the heigh restriction can be detoured by ramping around the obstruction. If it is not possible for a truck to ramp around the height restriction, note the existing height as well.
Step 2.3
Identify recently completed or under construction projects that would be considered relevant to freight performance. Include only projects that were not taken into account during the freight data analysis period. Any completed or under construction roadway project after the date of the data that has the potential to mitigate a freight issue on a corridor segment should be listed in the template. Such projects can include the construction of climbing lanes or Dynamic Message Signs (DMS) installation. Sources of recent or current project activity can be ADOT MPD staff, ADOT public notices, and ADOT District staff.
Step 2.4
Update the Final Need using the following criteria:

- If there is at least one truck height restriction hot spot where a truck cannot ramp around on a 'None' segment, increase (i.e., worsen) the need rating to 'Low'.
- If a recent project has superseded the performance rating data and it is certain the project addressed the need, change the need rating to "None".
- If a recent project has superseded the performance rating data but it is uncertain that a project addressed the need, maintain the current need rating and note the uncertainty as a comment.


## Step 2.5

Note any programmed projects that could have the potential to mitigate any freight need on the segment. Programmed projects are provided as information and do not impact the need rating. Programmed projects will be reviewed in the development of solution sets for identified needs. The source of the programming information can be found in ADOT's 5 year construction program. If there are other comments relevant to the needs analysis, they can be entered in the right-most column.

Example Scales for Level of Need

| Freight Index <br> (Interrupted) <br> Performance <br> Score Thresholds | Performance <br> Level | Initial <br> Performance <br> Level of Need | Description (Non-emphasis Area) |
| :---: | :---: | :---: | :--- |
| 1.45 | Good | None | All levels of Good and the top third of <br> Fair (<1.58) |
|  | Good |  |  |
|  | Good |  | Low |
|  | Fair | Middle third of Fair (1.58-1.72) |  |
|  | Foir | Medium | Lower third of Fair and top third of Poor <br> (1.72-1.98) |
|  | Poor |  | Lower two-thirds of Poor (>1.98) |


| Needs Scale |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Measure | None <= | Low <= |  | > Medium < |  | High >= |
| Corridor Freight Index (Emphasis Area) | Dependent on weighted average of interrupted vs. uninterrupted segments |  |  |  |  |  |
| Corridor Freight Index (Non-Emphasis Area) | Dependent on weighted average of interrupted vs. uninterrupted segments |  |  |  |  |  |
| Freight Index (Segment) |  |  |  |  |  |  |
| Measure | None >= | > Low < |  | > Medium < |  | High <= |
| Interrupted | 1.58 | 1.72 | 1.72 | 1.98 | 1.98 | 1.58 |
| Uninterrupted | 1.22 | 1.28 | 1.28 | 1.42 | 1.42 | 1.22 |
| Measure | None <= | < Low > |  | < Medium > |  | High >= |
| Directional TTTR |  |  |  |  |  |  |
| Interrupted | 1.58 | 1.72 | 1.72 | 1.98 | 1.98 | 1.58 |
| Uninterrupted | 1.22 | 1.28 | 1.28 | 1.42 | 1.42 | 1.22 |
| Closure Duration |  |  |  |  |  |  |
| All Facility Operations | 71.07 | 97.97 | 97.97 | 151.75 | 151.75 | 71.07 |
| Measure | None >= | > Low $<$ |  | > Medium < |  | High <= |
| Bridge Clearance (feet) |  |  |  |  |  |  |
| All Bridges | 16.33 | 16.17 | 16.17 | 15.83 | 15.83 | 16.33 |

## Step 3: Contributing Factors

The Final Need ratings from Step 2 will populate into the Step 3 tab.
The steps to compete Step 3 include:

## Step 3.1

Input all roadway variable data that describe each segment into the appropriate columns. Note that this data can be copied from the Mobility Needs Assessment spreadsheet for Needs Assessment.

## Step 3.2

Input all traffic variables for each segment into the appropriate columns. Note that this data can be copied from the Mobility Needs Assessment spreadsheet for Needs Assessment.

## Step 3.3

Input any freight-related infrastructure that currently exists on the corridor for each segment. The relevant infrastructure can include DMS locations, weigh stations, Ports of Entry (POE), rest areas, parking areas, and climbing lanes. Include the mileposts of the listed infrastructure. This data can be extracted from the most recent Highway Log and the 2015 Climbing and Passing Lane Prioritization Study..

## Step 3.4

Input the Closure Extents that have occurred along the study corridor. Road closure information can be detailed out by the reason for the closure as documented in Highway Condition Reporting System (HCRS) data analyzed as part of the baseline corridor performance. Closure reasons include incident/accidents, winter storms, obstruction hazards, and undefined closures. Statewide average percentages for the various closure reasons have been calculated for the analysis period on ADOT's designated strategic corridors. Compare these statewide average percentages to the corridor percentages for the various closure reasons to identify higher than average percentages of one or more closure reasons on any given segment. Note that this data can be copied from the Mobility Needs Assessment spreadsheet for Needs Assessment. Input the closures as follows and use red text to indicate that the segment percentage exceeds statewide averages:

- Total Number of Closures
- \% Closures (No Reason)
- \% Incidents/Accidents
- \% Obstructions/Hazards
- \% Weather Related


## Step 3.5

List the non-actionable conditions that are present within each segment by milepost if possible. Non-Actionable conditions are conditions that exist within the environment of each segment that cannot be improved through an engineered solution. Examples of Non-Actionable conditions can include border patrol check points and other closures/restrictions not controlled by ADOT. Note that this data can be copied from the Mobility Needs Assessment spreadsheet for Needs Assessment.

Step 3.6
Input any programmed and planned projects or issues that have been identified from previous documents or studies that are relevant to the Final Need. Sources for this data include the current Highway Log, the 2015 Climbing and Passing Lane Prioritization Study, and ADOT's 5-year construction program.

Step 3.7
Considering all information in Steps 1-3, identify the contributing factors to the Final Need column. Potential contributing factors to freight performance needs include roadway vertical grade, number of lanes, traffic volume-to-capacity ratios, presence/lack of a climbing lanes, and road closures. Also, identify higher than average percentages of one or more closure reasons on any given segment

Pavement Performance Area - Need Analysis Step 1

| Segment \# | Segment Length (miles) | Segment Mileposts (MP) | Facility Type | Pavement Index |  |  | Directional PSR |  |  |  |  | \% Area Failure |  |  | Initial Need |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Performance | Performance | Level of | Performance Score |  | Performance Objective | Level of Need |  | Performance Score | Performance Objective | Level of Need |  |
|  |  |  |  | Score | Objective | Need | NB | SB |  | NB | SB |  |  |  |  |
| 191-1 | 24 | MPO-MP24 | Highway | 3.17 | Fair or Better | Low | 3.10 | 3.24 | Fair or Better | Low | Low | 71.00\% | Fair or Better | High | Medium |
| 191-2 | 43 | MP24-MP67 | Highway | 2.89 | Fair or Better | Medium | 3.44 | 3.38 | Fair or Better | None | None | 56.00\% | Fair or Better | High | High |
| 191-3 | 17 | MP87-MP104 | Highway | 3.42 | Fair or Better | None | 3.36 | 3.69 | Fair or Better | None | None | 72.00\% | Fair or Better | High | Low |
| 191-4 | 12 | MP104-MP116 | Highway | 3.44 | Fair or Better | None | 3.29 | 3.32 | Fair or Better | Low | None | 42.00\% | Fair or Better | High | Low |
| 191-5 | 5 | MP116-MP121 | Highway | 3.10 | Fair or Better | Low | 3.16 | 3.07 | Fair or Better | Low | Medium | 80.00\% | Fair or Better | High | Medium |
| 70-6 | 9 | MP339-MP330 | Highway | 3.23 | Fair or Better | Low | 3.15 | 3.25 | Fair or Better | Low | Low | 60.00\% | Fair or Better | High | Medium |
| 70-7 | 30 | MP330-MP300 | Highway | 2.83 | Fair or Better | Medium | 2.87 | 3.08 | Fair or Better | Medium | Medium | 87.00\% | Fair or Better | High | High |
| 70-8 | 2 | MP300-MP298 | Highway | 2.59 | Fair or Better | Medium | 3.35 | 3.67 | Fair or Better | None | None | 100.00\% | Fair or Better | High | High |
| 70-9 | 5 | MP298-MP293 | Highway | 2.71 | Fair or Better | Medium | 3.44 | 3.36 | Fair or Better | None | None | 100.00\% | Fair or Better | High | High |
| 70-10 | 19 | MP293-MP274 | Highway | 2.69 | Fair or Better | Medium | 3.10 | 3.35 | Fair or Better | Low | None | 79.00\% | Fair or Better | High | High |
| 70-11 | 4 | MP274-MP270 | Highway | 2.40 | Fair or Better | High | 3.27 | 3.28 | Fair or Better | Low | Low | 88.00\% | Fair or Better | High | High |
| 70-12 | 15 | MP270-MP255 | Highway | 3.57 | Fair or Better | None | 3.28 | 3.53 | Fair or Better | Low | None | 33.00\% | Fair or Better | High | Low |
| 70\|60-13 | 12 | MP255-MP243 | Highway | 3.28 | Fair or Better | Low | 3.13 | 3.28 | Fair or Better | Low | Low | 54.00\% | Fair or Better | High | Medium |
| 60E-14 | 16 | MP243-MP227 | Highway | 3.68 | Fair or Better | None | 3.66 | 3.82 | Fair or Better | None | None | 44.00\% | Fair or Better | High | Low |
| 60E-15 | 2 | MP227-MP225 | Highway | 4.03 | Fair or Better | None | 3.70 | 3.65 | Fair or Better | None | None | 0.00\% | Fair or Better | None | None |
| 60E-16 | 2 | MP225-MP223 | Highway | 4.50 | Fair or Better | None | 4.22 | 4.15 | Fair or Better | None | None | 0.00\% | Fair or Better | None | None |
| 60E-17 | 11 | MP223-MP212 | Highway | 3.51 | Fair or Better | None | 3.93 | 3.99 | Fair or Better | None | None | 76.00\% | Fair or Better | High | Low |
| 60E-18 | 7 | MP212-MP205 | Highway | 3.30 | Fair or Better | Low | 3.62 | 3.83 | Fair or Better | None | None | 93.00\% | Fair or Better | High | Medium |
| 60E-19 | 6 | MP205-MP199 | Highway | 3.57 | Fair or Better | None | 3.57 | 3.65 | Fair or Better | None | None | 33.00\% | Fair or Better | High | Low |
| 60E-20 | 5 | MP199-MP194 | Interstate | 4.17 | Fair or Better | None | 3.87 | 3.83 | Fair or Better | None | None | 0.00\% | Fair or Better | None | None |
| Emphasis Area? | No | Weighted Average |  | 3.18 | Fair or Better | Low |  |  |  |  |  |  |  |  |  |

Pavement Performance Area - Need Analysis Step 2

|  |  |  |  | Need Adjustments |  | Final Need | Comments (may include programmed projects or issues from previous reports) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Segment <br> \# | Segment Length (miles) | Segment Mileposts (MP) | Initial <br> Need | Hot Spots | Previous Projects (which supersede condition data) |  |  |
| 191-1 | 24 | MPO-MP24 | Medium | MP 0-5 Both; MP 5-6 NB; MP 6-7 Both; MP 7-8 NB, MP 9-10 Both; MP 12-15 NB; MP15-20 Both; MP 20-22 NB; MP 22-23 SB; MP24-23 Both | None | Medium |  |
| 191-2 | 43 | MP24-MP67 | High | MP 24-27 Both; MP 27-28 SB; MP28-29 NB; MP 2932 Both; MP 32-33 SB; MP 3335 Both; MP 35-36 SB; MP 42-43 SB MP 45-46 Both; MP 48-49 SB; MP 50-62 Both | None | High |  |
| 191-3 | 17 | MP87-MP104 | Low | MP 88-89 NB; Mp89-092 Both; MP94-95 NB; MP95-101 Both; MP101-104 NB | None | Low |  |
| 191-4 | 12 | MP104-MP116 | Low | MP 104-109 Both | None | Low |  |
| 191-5 | 5 | MP116-MP121 | Medium | MP117-121 Both | None | Medium |  |
| 70-6 | 9 | MP339-MP330 | Medium | MP 330-332 SB; MP332-333 Both; MP 333-335 SB; MP 335-336 NB; MP 336-337 Both; MP 338-339 SB; MP 339-340 Both | None | Medium | Pavement rehabilitation MP 335.8-342.1 FY 2021 F027901D |
| 70-7 | 30 | MP330-MP300 | High | MP 300-314 Both; MP314-315 EB; MP 327-329 EB; MP 329-330 Both | None | High |  |
| 70-8 | 2 | MP300-MP298 | High | MP 298-300 Both | None | High |  |
| 70-9 | 5 | MP298-MP293 | High | MP 293-298 Both | None | High |  |
| 70-10 | 19 | MP293-MP274 | High | MP 274-275 Both; MP279-293 Both | None | High |  |
| 70-11 | 4 | MP274-MP270 | High | MP 270-271 EB; MP 271-274 Both | None | High |  |
| 70-12 | 15 | MP270-MP255 | Low | MP 255-256 EB; MP 256-257 Both; MP 257-258 WB; MP 258-260 Both; MP266268 EB | None | Low |  |
| 70\|60-13 | 12 | MP255-MP243 | Medium | MP 243-244 EB; MP 244-245 Both; MP 245-246 EB; MP 249-251 EB; MP 252-255 Both | None | Medium |  |
| 60E-14 | 16 | MP243-MP227 | Low | MP236-243 Both | None | Low |  |
| 60E-15 | 2 | MP227-MP225 | None | None | None | None |  |
| 60E-16 | 2 | MP225-MP223 | None | None | None | None |  |
| 60E-17 | 11 | MP223-MP212 | Low | MP 212-213 Both; MP 213-215 WB; MP215-219 Both; MP 219-220 EB; MP 220221 Both; MP 221-222 EB | None | Low |  |
| 60E-18 | 7 | MP212-MP205 | Medium | MP 205-206 WB; MP 206-212 Both | None | Medium |  |
| 60E-19 | 6 | MP205-MP199 | Low | MP 199-201 WB; MP 201-202 EB; MP 204-205 WB | None | Low |  |
| 60E-20 | 5 | MP199-MP194 | None | None | None | None |  |

## Pavement History





|  |  |  | Pavement Treatment Reference Numbers |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1. 2001 (NB/SB): 0.5" AR-ACFC <br> 2. 2007 (NB/SB): Remove $0.5^{\prime \prime}, 0.5^{\prime \prime}$ AR-ACFC | $\left.\right\|^{19.2005(20)}$ |  |  |  |  |
|  |  | 33. 2007 (EBWB): Reme |  | 57. 2009 (EEMB): Gind |  |
| 4. 2008 ( NBISB): Fog Coat |  | 40. 2012 (EENB): S | Seal coat |  |  |
|  |  | 4, 41.2013 (EEPB): Reme |  |  |  |
| ${ }^{\text {7. } 2014}$ ( NBESBE): Seal Coavfrog Coat | ${ }^{20} 52.2020$ (NBISBE: |  | Rembe 1.5 S. Now 12.5 AR Na | 61. 2008 (EBWB): Remone Or', New 599' AC, Nee ARACFC |  |
|  |  | 44. 2005 (EBWB): R | Renowe 01.5.5. New $12.55^{\text {a }}$ AR New ARACFC |  |  |
|  |  | 45.2013 EEFMB): | Remoned 0.5. New $0.55^{\text {a }}$ ACF C |  |  |
|  |  |  | Remen |  |  |
|  | 30. 2011 (EEMB): M Mroro Seal | 48. 2004 (EBNB): R | Remoer 3 , New 3'AC, New ARACFC | 66.2018(E8/W8) 0.5 " 7 |  |
|  |  | 49.2012 (EFWB): | Mico Seal |  |  |
|  |  |  | $\xrightarrow{\text { Renow } 2335 \% \text { New } 233^{\text {AR }} \text { AR }}$ |  |  |
| 16.208) (NeISB): Seai Coat | 34.2009(EBPB): Fog Coat | ${ }^{52} 20203$ (EFWB): R |  |  |  |
|  | 36. 2004(EBWB): Fogcosat | 54.2000 (EBNB): Remer | Remen | \|in |  |

Legend
$\square$ New Paving or Reconstruction
Mill and Overlay (Adding Structural Thickness)
Mill and Replace (No Change Structural Thickness)
Fog Coat or Thin Overlay Treatments

## Aロロт



Pavement Historical Investment

| Route | Segment | Pavement History Value (bid projects) | Pavement History (bid projects) | Resulting Historical Investment |
| :---: | :---: | :---: | :---: | :---: |
| 191 | 1 | 4.5 | Low | Low |
| 191 | 2 | 5.3 | Medium | Medium |
| 191 | 3 | 7.2 | High | High |
| 191 | 4 | 1.5 | Low | Low |
| 191 | 5 | 3.9 | Low | Low |
| 70 | 6 | 3.1 | Low | Low |
| 70 | 7 | 3.5 | Low | Low |
| 70 | 8 | 5.0 | Medium | Medium |
| 70 | 9 | 7.5 | High | High |
| 70 | 10 | 3.8 | Low | Low |
| 70 | 11 | 4.5 | Low | Low |
| 70 | 12 | 3.9 | Low | Low |
| $70 \mid 60$ | 13 | 3.0 | Low | Low |
| 60E | 14 | 10.7 | High | Low |
| 60E | 15 | 9.5 | High | Low |
| 60E | 16 | 7.3 | High | Low |
| 60E | 17 | 9.2 | High | Low |
| 60E | 18 | 0 | Low | Low |
| 60E | 19 | 0.5 | Low | Low |
| 60E | 20 | 1 | Low | Low |

Pavement Performance Area - Need Analysis Step 3

| Segment \# | Segment Length (miles) | Segment Mileposts (MP) | Final Need | Bid History Investment | PeCos History Investment | Resulting Historical Investment | Contributing Factors and Comments |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 191-1 | 24 | MPO-MP24 | Medium | Low | Low | Low |  |
| 191-2 | 43 | MP24-MP67 | High | Medium | Medium | Medium |  |
| 191-3 | 17 | MP87-MP104 | Low | High | Low | High |  |
| 191-4 | 12 | MP104-MP116 | Low | Low | High | Medium |  |
| 191-5 | 5 | MP116-MP121 | Medium | Low | High | Medium |  |
| 70-6 | 9 | MP339-MP330 | Medium | Low | Low | Low | Pavement rehabilitation MP 335.8-342.1 FY 2021 F027901D |
| 70-7 | 30 | MP330-MP300 | High | Low | Low | Low |  |
| 70-8 | 2 | MP300-MP298 | High | Medium | Low | Medium |  |
| 70-9 | 5 | MP298-MP293 | High | High | Low | High |  |
| 70-10 | 19 | MP293-MP274 | High | Low | High | Medium |  |
| 70-11 | 4 | MP274-MP270 | High | Low | Low | Low |  |
| 70-12 | 15 | MP270-MP255 | Low | Low | High | Medium |  |
| 70\|60-13 | 12 | MP255-MP243 | Medium | Low | High | Medium |  |
| 60E-14 | 16 | MP243-MP227 | Low | Medium | Low | Medium |  |
| 60E-15 | 2 | MP227-MP225 | None | Medium | Low | Medium |  |
| 60E-16 | 2 | MP225-MP223 | None | Medium | Low | Medium |  |
| 60E-17 | 11 | MP223-MP212 | Low | Medium | Medium | Medium |  |
| 60E-18 | 7 | MP212-MP205 | Medium | Low |  | Low |  |
| 60E-19 | 6 | MP205-MP199 | Low | Low |  | Low |  |
| 60E-20 | 5 | MP199-MP194 | None | Low |  | Low |  |

Bridge Performance Area - Need Analysis Step 1

| Segment \# | Segment Length (miles) | Segment <br> Mileposts <br> (MP) | Number of Bridges in Segment | Bridge Index |  |  | Lowest Bridge Rating |  |  | Sufficiency Rating |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Performance Score | Performance Objective | Level of Need | Performance Score | Performance Objective | Level of Need | Performance Score | Performance Objective | Level of Need |
| 191-1 | 24 | MPO-MP24 | 1 | 6.00 | Fair or Better | None | 6.00 | Fair or Better | None | 87.80 | Fair or Better | None |
| 191-2 | 43 | MP24-MP67 | 2 | 5.36 | Fair or Better | Medium | 5.00 | Fair or Better | Low | 69.23 | Fair or Better | Low |
| 191-3 | 17 | MP87-MP104 | 2 | 5.51 | Fair or Better | Low | 5.00 | Fair or Better | Low | 93.81 | Fair or Better | None |
| 191-4 | 12 | MP104-MP116 | 1 | 6.00 | Fair or Better | None | 6.00 | Fair or Better | None | 69.50 | Fair or Better | Low |
| 191-5 | 5 | MP116-MP121 | 0 | No Bridges | Fair or Better | N/A | No Bridges | Fair or Better | N/A | No Bridges | Fair or Better | N/A |
| 70-6 | 9 | MP339-MP330 | 1 | 6.00 | Fair or Better | None | 6.00 | Fair or Better | None | 68.10 | Fair or Better | Low |
| 70-7 | 30 | MP330-MP300 | 8 | 5.74 | Fair or Better | Low | 5.00 | Fair or Better | Low | 70.25 | Fair or Better | None |
| 70-8 | 2 | MP300-MP298 | 1 | 6.00 | Fair or Better | None | 6.00 | Fair or Better | None | 73.00 | Fair or Better | None |
| 70-9 | 5 | MP298-MP293 | 0 | No Bridges | Fair or Better | N/A | No Bridges | Fair or Better | N/A | No Bridges | Fair or Better | N/A |
| 70-10 | 19 | MP293-MP274 | 1 | 7.00 | Fair or Better | None | 7.00 | Fair or Better | None | 80.00 | Fair or Better | None |
| 70-11 | 4 | MP274-MP270 | 2 | 6.69 | Fair or Better | None | 5.00 | Fair or Better | Low | 82.02 | Fair or Better | None |
| 70-12 | 15 | MP270-MP255 | 1 | 6.00 | Fair or Better | None | 6.00 | Fair or Better | None | 52.90 | Fair or Better | Medium |
| 70\|60-13 | 12 | MP255-MP243 | 11 | 5.16 | Fair or Better | Medium | 4.00 | Fair or Better | Medium | 78.01 | Fair or Better | None |
| 60E-14 | 16 | MP243-MP227 | 6 | 5.52 | Fair or Better | Low | 3.00 | Fair or Better | High | 68.13 | Fair or Better | Low |
| 60E-15 | 2 | MP227-MP225 | 3 | 6.32 | Fair or Better | None | 6.00 | Fair or Better | None | 84.08 | Fair or Better | None |
| 60E-16 | 2 | MP225-MP223 | 2 | 5.00 | Fair or Better | Medium | 5.00 | Fair or Better | Low | 86.43 | Fair or Better | None |
| 60E-17 | 11 | MP223-MP212 | 7 | 6.64 | Fair or Better | None | 5.00 | Fair or Better | Low | 95.57 | Fair or Better | None |
| 60E-18 | 7 | MP212-MP205 | 8 | 5.89 | Fair or Better | Low | 5.00 | Fair or Better | Low | 90.24 | Fair or Better | None |
| 60E-19 | 6 | MP205-MP199 | 6 | 5.93 | Fair or Better | Low | 5.00 | Fair or Better | Low | 91.43 | Fair or Better | None |
| 60E-20 | 5 | MP199-MP194 | 4 | 6.00 | Fair or Better | None | 6.00 | Fair or Better | None | 93.95 | Fair or Better | None |

Bridge Performance Area - Need Analysis Step 2

| Segment\# | Segment Length (miles) | Segment Mileposts (MP) | Number of Bridges in Segment | Initial Need | Need Adjustments |  | Final Need | Historical Review | Comments |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Hot Spots <br> (Rating of 4 or multiple 5's) | Previous Projects (which supersede condition data) |  |  |  |
| 191-1 | 24 | MPO-MP24 | 1 | None | None | None | None | None |  |
| 191-2 | 43 | MP24-MP67 | 2 | Medium | Cochise UPRR OP (\#157) (MP 62.88) | None | Medium | None |  |
| 191-3 | 17 | MP87-MP104 | 2 | Low | None | None | Low | None |  |
| 191-4 | 12 | MP104-MP116 | 1 | Low | None | None | Low | None |  |
| 191-5 | 5 | MP116-MP121 | 0 | N/A | No Bridges within Segment | None | N/A | None |  |
| 70-6 | 9 | MP339-MP330 | 1 | Low | None | None | Low | None |  |
| 70-7 | 30 | MP330-MP300 | 8 | Low | Holyoak Wash Bridge (\#514) (MP 302.53) | None | Low | None |  |
| 70-8 | 2 | MP300-MP298 | 1 | None | None | None | None | None |  |
| 70-9 | 5 | MP298-MP293 | 0 | N/A | No Bridges within Segment | None | N/A | None |  |
| 70-10 | 19 | MP293-MP274 | 1 | None | None | None | None | None |  |
| 70-11 | 4 | MP274-MP270 | 2 | Low | None | None | Low | None |  |
| 70-12 | 15 | MP270-MP255 | 1 | Low | None | None | Low | None |  |
| 70\|60-13 | 12 | MP255-MP243 | 11 | Medium | Bloody Tanks Bridge (\#173) (MP 243.71) <br> Pinal Creek Bridge (\#266) (MP249.64) <br> Pinal Creek Bridge (\#36) (MP249.80) <br> Pinal Creek Bridge (\#549) (MP 250.37) <br> McMillen Wash Bridge (\#1028) (MP251.75) | None | Medium | None |  |
| 60E-14 | 16 | MP243-MP227 | 6 | Medium | Queen Creek Bridge (\#436) (MP 226.14) Waterfall Canyon Bridge (\#328) (MP229.50) | None | Medium | None |  |
| 60E-15 | 2 | MP227-MP225 | 3 | None | None | None | None | None |  |
| 60E-16 | 2 | MP225-MP223 | 2 | Medium | None | None | Medium | None |  |
| 60E-17 | 11 | MP223-MP212 | 7 | Low | None | None | Low | None |  |
| 60E-18 | 7 | MP212-MP205 | 8 | Low | Sand Tanks Wash Bridge EB (\#435) (MP208.75) Bridge WB <br> (\# 857) (MP 207.98) | None | Low | None |  |
| 60E-19 | 6 | MP205-MP199 | 6 | Low | None | None | Low | None |  |
| 60E-20 | 5 | MP199-MP194 | 4 | None | None | None | None | None |  |

## ADOT

Bridge Performance Area - Need Analysis Step 3

| $\underset{\#}{\text { Segment }}$ | Segment Length (Miles) | Segment Mileposts (MP) | Number of Bridges in Segment | Final Need | Contributing Factors |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Bridge | Current Ratings | Historical Review |
| 191-1 | 24 | $\begin{aligned} & \hline \text { MPO- } \\ & \text { MP24 } \end{aligned}$ | 1 | None | None | None |  |
| 191-2 | 43 | $\begin{aligned} & \text { MP24- } \\ & \text { MP67 } \end{aligned}$ | 2 | Medium | Cochise UPRR OP (\#157) (MP 62.88) | Deck and Superstructure of 5 | Cochise UPRR OP (No. 157 MP 62.88) Deck=5 |
| 191-3 | 17 | MP87MP104 | 2 | Low | None | None | Monk Draw Bridge SB (No. 292 MP 89.28) Eval=5 |
| 191-4 | 12 | MP104MP116 | 1 | Low | None | None |  |
| 191-5 | 5 | MP116 <br> MP121 | 0 | N/A | No Bridges within Segment | No Bridges within Segment |  |
| 70-6 | 9 | $\begin{aligned} & \text { MP339- } \\ & \text { MP330 } \end{aligned}$ | 1 | Low | None | None |  |
| 70-7 | 30 | MP330MP300 | 8 | Low | $\begin{aligned} & \hline \text { Holyoak Wash Bridge (\#514) (MP 302.53) } \\ & \text { Boack Rock Wask Br (\#545) (MP306.76) } \\ & \text { Hunzinger Wash Br (\#561) (MP313.62) } \\ & \text { Patterson Wash Br (\#1421) (MP 327.72) } \\ & \hline \end{aligned}$ | Deck, Sub- and Superstructure of 5 Superstructure of 5 Superstructure of 5 Superstructure of 5 | Hunzinger Wash Bridge (No. 561 MP 313.62) Super=5; Eval=5 Black Rock Wash Bridge (No. 515 MP 306.76)Super=5; Eval=5 Holyoak Wash Bridge (No. 514 MP 302.53) Deck, Sub, Super, Eval=5 |
| 70-8 | 2 | MP300- <br> MP298 | 1 | None | None | None |  |
| 70-9 | 5 | $\begin{aligned} & \text { MP298- } \\ & \text { MP293 } \end{aligned}$ | 0 | N/A | No Bridges within Segment | No Bridges within Segment |  |
| 70-10 | 19 | MP293- <br> MP274 | 1 | None | None | None |  |

Bridge Performance Area - Need Analysis Step 3 (continued)

| $\begin{aligned} & \text { Segment } \\ & \# \end{aligned}$ | Segment Length (Miles) | Segment Mileposts (MP) | Number of Bridges in Segment | Final Need | Contributing Factors |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Bridge | Current Ratings | Historical Review |
| 70-11 | 4 | $\begin{aligned} & \text { MP274- } \\ & \text { MP270 } \end{aligned}$ | 2 | Low | None | None | Peridot RR OP (No. 477 MP 271.27) Deck=5 |
| 70-12 | 15 | $\begin{aligned} & \text { MP270- } \\ & \text { MP255 } \end{aligned}$ | 1 | Low | None | None |  |
| 70\|60-13 | 12 | $\begin{aligned} & \text { MP255- } \\ & \text { MP243 } \end{aligned}$ | 11 | Medium | Bloody Tanks Bridge (\#173) (MP 243.71) Pinal Creek Bridge (\#266) (MP249.64) Pinal Creek Bridge (\#36) (MP249.80) Pinal Creek Bridge (\#549) (MP 250.37) McMillen Wash Bridge (\#1028) (MP251.75) | Deck and Substructure of 5 Superstructure of 5 <br> Deck and Substructure of 5 <br> Deck and Substructure of 5 <br> Deck, Sub- and Superstructure of 5 | Bloody Tanks Bridge (No. 173 MP 243.71) Super, Eval =5 Globe Viaduct (No. 1787 MP 250.9) Deck=5 <br> Pinal Creek Bridge (No. 266 MP249.64) Deck, Sub and Eval=5 <br> Pinal Creek Bridge (No. 36 MP249.80) Deck, Sub and Eval $=5$ <br> Pinal Creek Bridge (No. 549 MP 250.37) Deck, Sub, Super and Eval=5 <br> McMillen Wash Bridge (No. 1028 MP251.75) Super and Eval =5 |
| 60E-14 | 16 | $\begin{aligned} & \text { MP243- } \\ & \text { MP227 } \end{aligned}$ | 6 | Medium | Queen Creek Bridge (\#436) (MP 226.14) Waterfall Canyon Bridge (\# 328) (MP229.50) | Deck and Superstructure of 4 , Substructure of 3 Substructure of 5 |  |
| 60E-15 | 2 | $\begin{aligned} & \text { MP227- } \\ & \text { MP225 } \end{aligned}$ | 3 | None | None | None |  |
| 60E-16 | 2 | $\begin{aligned} & \text { MP225- } \\ & \text { MP223 } \end{aligned}$ | 2 | Medium | None | None | Wash Bridge (No.319 MP224.64) Eval=5 Silver King Wash Br (No. 318 MP223.7) Eval=5 |
| 60E-17 | 11 | $\begin{aligned} & \text { MP223- } \\ & \text { MP212 } \end{aligned}$ | 7 | Low | None | None | Wash Bridge (No.288 MP224.87) Eval=5 Queen Creek Bridge WB(No. 296 MP222.45) Super and Eval=5 |
| 60E-18 | 7 | $\begin{aligned} & \text { MP212- } \\ & \text { MP205 } \end{aligned}$ | 8 | Low | Sand Tanks Wash Bridge EB (\#435) (MP208.75) Bridge WB <br> (\# 857) (MP 207.98) | Deck and Substructure of 5 <br> Deck and Substructrure of 5 |  |
| 60E-19 | 6 | MP205- <br> MP199 | 6 | Low | None | None |  |
| 60E-20 | 5 | MP199- <br> MP194 | 4 | None | None | None |  |

Bridge Ratings History

identifies the bridge indicated is of concern from a historical ratings perspective
 performance of the bridge)

Maximum \# of Increases: Maximum number of times that the Deck Rating, Substructure Rating, or Superstructure Rating increased from 1997 to 2014. (Higher number could indicate a higher level of investment) Change in Sufficiency Rating: Cumulative change in Sufficiency Rating from 1997 to 2014. (Bigger negative number could indicate a more dramatic decline in the performance of the bridge)

Bridge Ratings History (continued)


## O

identifies the bridge indicated is of concern from a historical ratings perspective
Maximum \# of Decreases: Maximum number of times that the Deck Rating, Substructure Rating, or Superstructure Rating decreased from 1997 to 2014. (Higher number could indicate a more dramatic decline in the performance of the bridge)
Maximum \# of Increases: Maximum number of times that the Deck Rating, Substructure Rating, or Superstructure Rating increased from 1997 to 2014. (Higher number could indicate a higher level of investment) Change in Sufficiency Rating: Cumulative change in Sufficiency Rating from 1997 to 2014. (Bigger negative number could indicate a more dramatic decline in the performance of the bridge)

Mobility Performance Area - Needs Analysis Step 1

| Segment \# | Segm ent Milep osts | Segment Length (miles) | Environment Type | Facility Operation | Mobility Index |  |  | Future Daily V/C |  |  | Existing Peak Hour V/C |  |  |  |  | Closure Extent (occurrences/year/mile) |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Performance Score | Performa nce Objective | Level of Need | Performance Score | Performanc e Objective | Level of Need | Performance Score |  | Performan ce Objective | Level of Need |  | Performance Score |  | Performanc e Objective | Level of Need |  |
|  |  |  |  |  |  |  |  |  |  |  | NB | SB |  | NB | SB | NB | SB |  | NB | SB |
| 191-1 | 0-24 | 24 | Rural | Interrupte d | 0.16 | Fair or Better | None | 0.18 | Fair or Better | None | 0.13 | 0.13 | Fair or Better | None | None | 0.04 | 0.02 | Fair or Better | None | None |
| 191-2 | $\begin{gathered} 24- \\ 67 \\ \hline \end{gathered}$ | 43 | Rural | Interrupte d | 0.13 | Fair or Better | None | 0.17 | Fair or Better | None | 0.08 | 0.11 | Fair or Better | None | None | 0.03 | 0.01 | Fair or Better | None | None |
| 191-3 | $\begin{aligned} & 87- \\ & 104 \\ & \hline \end{aligned}$ | 17 | Rural | Uninterrup ted | 0.05 | Fair or Better | None | 0.05 | Fair or Better | None | 0.03 | 0.03 | Fair or Better | None | None | 0.02 | 0.00 | Fair or Better | None | None |
| 191-4 | $\begin{gathered} 104- \\ 116 \end{gathered}$ | 12 | Rural | Uninterrup ted | 0.17 | Fair or Better | None | 0.19 | Fair or Better | None | 0.11 | 0.11 | Fair or Better | None | None | 0.08 | 0.07 | Fair or Better | None | None |
| 191-5 | $\begin{gathered} 116- \\ 121 \end{gathered}$ | 5 | Urban | Interrupte d | 0.27 | Fair or Better | None | 0.30 | Fair or Better | None | 0.15 | 0.16 | Fair or Better | None | None | 0.20 | 0.20 | Fair or Better | None | None |
| 70-6 | $\begin{gathered} 330- \\ 339 \end{gathered}$ | 9 | Urban | Interrupte d | 0.41 | Fair or Better | None | 0.45 | Fair or Better | None | 0.31 | 0.29 | Fair or Better | None | None | 0.02 | 0.04 | Fair or Better | None | None |
| 70-7 | $\begin{gathered} 300- \\ 330 \end{gathered}$ | 30 | Rural | Uninterrup ted | 0.18 | Fair or Better | None | 0.20 | Fair or Better | None | 0.11 | 0.10 | Fair or Better | None | None | 0.04 | 0.01 | Fair or Better | None | None |
| 70-8 | $\begin{gathered} 298- \\ 300 \end{gathered}$ | 2 | Rural | Uninterrup ted | 0.11 | Fair or Better | None | 0.12 | Fair or Better | None | 0.08 | 0.05 | Fair or Better | None | None | 0.10 | 0.00 | Fair or Better | None | None |
| 70-9 | $\begin{gathered} 293- \\ 298 \end{gathered}$ | 5 | Rural | Uninterrup ted | 0.24 | Fair or Better | None | 0.26 | Fair or Better | None | 0.16 | 0.12 | Fair or Better | None | None | 0.04 | 0.04 | Fair or Better | None | None |
| 70-10 | $\begin{gathered} 274- \\ 293 \end{gathered}$ | 19 | Rural | Uninterrup ted | 0.15 | Fair or Better | None | 0.17 | Fair or Better | None | 0.11 | 0.08 | Fair or Better | None | None | 0.07 | 0.05 | Fair or Better | None | None |
| 70-11 | $\begin{gathered} 270- \\ 274 \end{gathered}$ | 4 | Rural | Uninterrup ted | 0.18 | Fair or Better | None | 0.20 | Fair or Better | None | 0.13 | 0.10 | Fair or Better | None | None | 0.00 | 0.00 | Fair or Better | None | None |
| 70-12 | $\begin{gathered} 255- \\ 270 \end{gathered}$ | 15 | Rural | Uninterrup ted | 0.24 | Fair or Better | None | 0.27 | Fair or Better | None | 0.16 | 0.17 | Fair or Better | None | None | 0.17 | 0.00 | Fair or Better | None | None |
| 70/60E-13 | $\begin{gathered} 243- \\ 255 \end{gathered}$ | 12 | Urban | Interrupte d | 0.40 | Fair or Better | None | 0.45 | Fair or Better | None | 0.26 | 0.25 | Fair or Better | None | None | 0.22 | 0.35 | Fair or Better | None | None |
| 60E-14 | $\begin{gathered} 227- \\ 243 \end{gathered}$ | 16 | Rural | Uninterrup ted | 1.42 | Fair or Better | High | 1.71 | Fair or Better | High | 0.79 | 1.14 | Fair or Better | Mediu m | High | 0.67 | 1.84 | Fair or Better | Medi um | High |
| 60E-15 | $\begin{gathered} 225- \\ 227 \end{gathered}$ | 2 | Rural | Uninterrup ted | 0.27 | Fair or Better | None | 0.37 | Fair or Better | None | 0.11 | 0.11 | Fair or Better | None | None | 0.00 | 0.90 | Fair or Better | None | High |
| 60E-16 | $\begin{gathered} 223- \\ 225 \end{gathered}$ | 2 | Rural | Uninterrup ted | 0.27 | Fair or Better | None | 0.38 | Fair or Better | None | 0.16 | 0.16 | Fair or Better | None | None | 0.60 | 0.15 | Fair or Better | Medi um | None |
| 60E-17 | $\begin{gathered} \hline 212- \\ 223 \end{gathered}$ | 11 | Rural | Uninterrup ted | 0.26 | Fair or Better | None | 0.37 | Fair or Better | None | 0.15 | 0.14 | Fair or Better | None | None | 0.04 | 0.23 | Fair or Better | None | None |
| 60E-18 | $\begin{gathered} 205- \\ 212 \end{gathered}$ | 7 | Rural | Uninterrup ted | 0.53 | Fair or Better | None | 0.66 | Fair or Better | Low | 0.30 | 0.32 | Fair or Better | None | None | 0.00 | 0.23 | Fair or Better | None | None |
| 60E-19 | $\begin{gathered} 199- \\ 205 \end{gathered}$ | 6 | Urban | Interrupte d | 1.01 | Fair or Better | High | 0.86 | Fair or Better | Medi um | 0.86 | 0.91 | Fair or Better | Mediu m | Medi um | 0.10 | 0.30 | Fair or Better | None | None |
| 60E-20 | $\begin{gathered} 194.3 \\ -199 \\ \hline \end{gathered}$ | 4.7 | Urban | Uninterrup ted | 1.31 | Fair or Better | High | 1.45 | Fair or Better | High | 0.84 | 0.88 | Fair or Better | Mediu <br> m | Medi um | 0.68 | 0.09 | Fair or Better | Medi um | None |

Mobility Performance Area - Needs Analysis Step 1 (continued)

| $\begin{aligned} & \text { Segment } \\ & \# \end{aligned}$ | Segment <br> Mileposts | Segment Length(miles) (miles) | Environment Type | Facility Operation | Directional LOTTR (all vehicles) |  |  |  |  | Bicycle Accommodation |  |  | Initial Need |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Performance Score |  | Performance Objective | Level of Need |  | Performance Score | Performance Objective | Level of Need |  |
|  |  |  |  |  | NB | SB |  | NB | SB |  |  |  |  |
| 191-1 | 0-24 | 24 | Rural | Interrupted | 1.40 | 1.39 | Fair or Better | Medium | Medium | 66\% | Fair or Better | Medium | Low |
| 191-2 | 24-67 | 43 | Rural | Interrupted | No Data | No Data | Fair or Better | N/A | N/A | 100\% | Fair or Better | None | None |
| 191-3 | 87-104 | 17 | Rural | Uninterrupted | No Data | No Data | Fair or Better | N/A | N/A | 49\% | Fair or Better | High | Low |
| 191-4 | 104-116 | 12 | Rural | Uninterrupted | No Data | No Data | Fair or Better | N/A | N/A | 97\% | Fair or Better | None | None |
| 191-5 | 116-121 | 5 | Urban | Interrupted | No Data | No Data | Fair or Better | N/A | N/A | 27\% | Fair or Better | High | Low |
| 70-6 | 330-339 | 9 | Urban | Interrupted | No Data | No Data | Fair or Better | N/A | N/A | 46\% | Fair or Better | High | Low |
| 70-7 | 300-330 | 30 | Rural | Uninterrupted | No Data | No Data | Fair or Better | N/A | N/A | 73\% | Fair or Better | Low | Low |
| 70-8 | 298-300 | 2 | Rural | Uninterrupted | No Data | No Data | Fair or Better | N/A | N/A | 0\% | Fair or Better | High | Low |
| 70-9 | 293-298 | 5 | Rural | Uninterrupted | No Data | No Data | Fair or Better | N/A | N/A | 26\% | Fair or Better | High | Low |
| 70-10 | 274-293 | 19 | Rural | Uninterrupted | No Data | No Data | Fair or Better | N/A | N/A | 4\% | Fair or Better | High | Low |
| 70-11 | 270-274 | 4 | Rural | Uninterrupted | No Data | No Data | Fair or Better | N/A | N/A | 4\% | Fair or Better | High | Low |
| 70-12 | 255-270 | 15 | Rural | Uninterrupted | No Data | No Data | Fair or Better | N/A | N/A | 23\% | Fair or Better | High | Low |
| 70/60E-13 | 243-255 | 12 | Urban | Interrupted | 1.16 | 1.15 | Fair or Better | None | None | 54\% | Fair or Better | Medium | Low |
| 60E-14 | 227-243 | 16 | Rural | Uninterrupted | 1.12 | 1.17 | Fair or Better | None | None | 49\% | Fair or Better | High | High |
| 60E-15 | 225-227 | 2 | Rural | Uninterrupted | 1.18 | 1.14 | Fair or Better | None | None | 95\% | Fair or Better | None | Low |
| 60E-16 | 223-225 | 2 | Rural | Uninterrupted | 1.05 | 1.12 | Fair or Better | None | None | 87\% | Fair or Better | None | Low |
| 60E-17 | 212-223 | 11 | Rural | Uninterrupted | 1.05 | 1.09 | Fair or Better | None | None | 96\% | Fair or Better | None | None |
| 60E-18 | 205-212 | 7 | Rural | Uninterrupted | 1.12 | 1.05 | Fair or Better | None | None | 100\% | Fair or Better | None | Low |
| 60E-19 | 199-205 | 6 | Urban | Interrupted | 1.20 | 1.14 | Fair or Better | None | None | 42\% | Fair or Better | High | High |
| 60E-20 | 194.3-199 | 4.7 | Urban | Uninterrupted | 1.06 | 1.06 | Fair or Better | None | None | 100\% | Fair or Better | None | High |

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## Mobility Performance Area - Needs Analysis Step 2

| Segment | Segment Mileposts (MP) | SegmentLength (miles) | Initial Need | Need Adjustments | Final Need | Planned and Programmed Future Projects |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Recent Projects Since 2020 |  |  |
| 191-1 | 0-24 | 24 | Low | None | Low | Programmed: None Planned: None |
| 191-2 | 24-67 | 43 | None | None | None | Programmed: None Planned: None |
| 191-3 | 87-104 | 17 | Low | None | Low | Programmed: None <br> Planned: None |
| 191-4 | 104-116 | 12 | None | None | None | Programmed: None <br> Planned: Restripe to 5 lanes between Atresia Road and Lebanon Road (to 2023) MP 110.9-116 |
| 191-5 | 116-121 | 5 | Low | None | Low | Programmed: None <br> Planned: Restripe to 5 lanes between 11th Street and US 70 (to 2023) MP 120-121 |
| 70-6 | 330-339 | 9 | Low | None | Low | Programmed: None Planned: None |
| 70-7 | 300-330 | 30 | Low | None | Low | Programmed: None Planned: None |
| 70-8 | 298-300 | 2 | Low | None | Low | Programmed: None Planned: None |
| 70-9 | 293-298 | 5 | Low | None | Low | Programmed: None <br> Planned: None |
| 70-10 | 274-293 | 19 | Low | None | Low | Programmed: None <br> Planned: None |
| 70-11 | 270-274 | 4 | Low | None | Low | Programmed: None <br> Planned: None |
| 70-12 | 255-270 | 15 | Low | None | Low | Programmed: None <br> Planned: None |

## Mobility Performance Area - Needs Analysis Step 2 (continued)

|  | Segment | Segment | Initial | Need Adjustments | Final Need | Planned and Programmed Future Projects |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Mileposts (MP) | Length (miles) | Need | Recent Projects Since 2020 |  |  |
| 70/60E-13 | 243-255 | 12 | Low | FY19 MP 247 New DMS Sign Eastbound (Arizona Statewide DMS Plan) and Construct new sidewalks on northside from MP 243-252 (Cobre Valley Comprehensive Transportation Study) | Low | Programmed: Construct alternative alignment/Widen to 4 lanes (2030) MP243-252; Implement access management through Miami (2030) 243245.5 <br> Planned: None |
| 60E-14 | 227-243 | 16 | High | None | High | Programmed: Construct alternative alignment/Widen to 4 lanes (2030) MP227-243; Realign intersection (2030) MP 242 <br> Planned: None |
| 60E-15 | 225-227 | 2 | Low | None | Low | Programmed: Construct alternative alignment/Widen to 4 lanes (2030) <br> MP225-227 <br> Planned: None |
| 60E-16 | 223-225 | 2 | Low | None | Low | Programmed: Construct alternative alignment/Widen to 4 lanes (2030) MP223-225 <br> Planned: None |
| 60E-17 | 212-223 | 11 | None | Picket Post- Construct new EB lanes parallel to existing, between Reymert Wash and Queen Creek and Gonzales Pass- Construct new EB lanes west of the summit, construct new WB lanes east of the summit | None | Programmed: Construct alternative alignment/Widen to 4 lanes (2030) <br> MP212-223 <br> Planned: None |
| 60E-18 | 205-212 | 7 | Low |  | Low | Programmed: None <br> Planned: None |
| 60E-19 | 199-205 | 6 | High |  | High | Programmed: None <br> Planned: None |
| 60E-20 | 194.3-199 | 4.7 | High |  | High | Programmed: None <br> Planned: None |

Mobility Performance Area - Needs Analysis Step 3

|  |  |  |  | Roadway Variables |  |  |  |  |  |  |  | Traffic Variables |  |  | Relevant Mobility Related Existing Infrastructure |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Segment | Segment Mileposts (MP) | Segment Length (miles) | Final Need | Functional Classification | Environmental Type (Urban/Rural) | Terrain | \# of <br> Lanes/ <br> Direction | Weighted Average Speed Limit | Aux Lanes | Divided/ NonDivided | \% No Passing | Existing LOS | $\begin{gathered} \text { Future } \\ 2040 \\ \text { LOS } \end{gathered}$ | \% Trucks |  |
| 191-1 | 0-24 | 24 | Low | State Highway | Rural | Level | 2 | 55 | No | Non-Divided | 12\% | A/B | A/B | 15\% | This segment includes one rest area |
| 191-2 | 24-67 | 43 | None | State Highway | Rural | Level | 2 | 55 | No | Non-Divided | 26\% | A/B | A/B | 27\% | This segment includes a Border patrol Check Point effecting NB traffic |
| 191-3 | 87-104 | 17 | Low | State Highway | Rural | Level | 4 | 55 | No | Divided | 3\% | A/B | A/B | 21\% |  |
| 191-4 | 104-116 | 12 | None | State Highway | Rural | Level | 4 | 65 | No | Non-Divided | 30\% | A/B | A/B | 16\% |  |
| 191-5 | 116-121 | 5 | Low | State Highway | Urban | Level | 4 | 40 | No | Non-Divided | 13\% | A-C | A-C | 10\% |  |
| 70-6 | 330-339 | 9 | Low | State Highway | Urban | Level | 4 | 40 | No | Non-Divided | 0\% | A-C | A-C | 13\% | This segment includes one rest area |
| 70-7 | 300-330 | 30 | Low | State Highway | Rural | Level | 2 | 55 | No | Non-Divided | 13\% | A/B | A/B | 27\% |  |
| 70-8 | 298-300 | 2 | Low | State Highway | Rural | Level | 2 | 65 | No | Non-Divided | 6\% | A/B | A/B | 25\% |  |
| 70-9 | 293-298 | 5 | Low | State Highway | Rural | Level | 2 | 50 | No | Non-Divided | 53\% | A/B | A/B | 25\% | This segment includes one rest area |
| 70-10 | 274-293 | 19 | Low | State Highway | Rural | Level | 2 | 55 | No | Non-Divided | 37\% | A/B | A/B | 25\% |  |
| 70-11 | 270-274 | 4 | Low | State Highway | Rural | Level | 2 | 55 | No | Non-Divided | 77\% | A/B | A/B | 25\% |  |
| 70-12 | 255-270 | 15 | Low | State Highway | Rural | Level | 2 | 60 | No | Non-Divided | 10\% | A/B | A/B | 17\% | This segment includes a climbing/passing lane |
| $\begin{gathered} \hline 70 / 60 \mathrm{E}- \\ 13 \\ \hline \end{gathered}$ | 243-255 | 12 | Low | State Highway | Urban | Level | 4 | 45 | No | Non-Divided | 0\% | A-C | A-C | 11\% | This segment includes one eastbound DMS. |
| 60E-14 | 227-243 | 16 | High | State Highway | Rural | Mountainous | 2 | 50 | No | Non-Divided | 68\% | D-F | D-F | 11\% |  |
| 60E-15 | 225-227 | 2 | High | State Highway | Rural | Rolling | 2 | 45 | No | Non-Divided | 98\% | D-F | D-F | 18\% | This segment includes one rest area |
| 60E-16 | 223-225 | 2 | High | State Highway | Rural | Level | 2 | 55 | No | Non-Divided | 55\% | A/B | D-F | 13\% |  |
| 60E-17 | 212-223 | 11 | None | State Highway | Rural | Level | 4 | 65 | No | Divided | 0\% | A/B | A/B | 13\% |  |
| 60E-18 | 205-212 | 7 | Low | State Highway | Rural | Level | 2 | 65 | No | Divided | 0\% | A/B | C | 13\% | This segment includes one eastbound DMS. |
| 60E-19 | 199-205 | 6 | High | State Highway | Urban | Level | 2 | 55 | No | Divided | 0\% | E/F | D | 11\% |  |
| 60E-20 | $\begin{gathered} \hline 194.3- \\ 199 \\ \hline \end{gathered}$ | 4.7 | High | State Highway | Urban | Level | 2 | 65 | No | Divided | 0\% | A-C | A-C | 9\% | This segment includes one westboound DMS. |

Safety Performance Area - Needs Analysis Step 1

| Segment | Operating Environment | Offset | Segment Length (miles) | Segment Mileposts (MP) | Safety Index |  |  | Directional Safety Index |  |  |  | \% of Fatal + Suspected Serious Injury Crashes at Intersections |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Performance Score | Performance Objective | Level of Need | NB/EB <br> Performance Score | SB/WB <br> Performance Score | NB/EB Level of Need | SB/WB Level of Need | Performance Score | Performance Objective | Level of Need |
| 191-1 | 2 or 3 Lane Undivided Highway | 0 | 24 | 0-24 | 0.39 | Average or Better | None | 0.04 | 0.73 | None | None | Insufficient Data | Average or Better | N/A |
| 191-2 | 2 or 3 Lane Undivided Highway | 0 | 43 | 24-67 | 0.49 | Average or Better | None | 0.54 | 0.44 | None | None | Insufficient Data | Average or Better | N/A |
| 191-3 | 2 or 3 or 4 Lane Divided Highway | 1 | 17 | 87-104 | 0.59 | Average or Better | None | 0.00 | 1.18 | None | Medium | Insufficient Data | Average or Better | N/A |
| 191-4 | 2 or 3 Lane Undivided Highway | 0 | 12 | 104-116 | 0.58 | Average or Better | None | 1.06 | 0.11 | Medium | None | Insufficient Data | Average or Better | N/A |
| 191-5 | 4 or 5 Lane Undivided Highway | 2 | 5 | 116-121 | 0.06 | Average or Better | None | 0.12 | 0.00 | None | None | Insufficient Data | Average or Better | N/A |
| 70-6 | 4 or 5 Lane Undivided Highway | 2 | 9 | 330-339 | 0.38 | Average or Better | None | 0.67 | 0.08 | None | None | Insufficient Data | Average or Better | N/A |
| 70-7 | 2 or 3 Lane Undivided Highway | 0 | 30 | 300-300 | 0.68 | Average or Better | None | 0.89 | 0.48 | None | None | Insufficient Data | Average or Better | N/A |
| 70-8 | 2 or 3 Lane Undivided Highway | 0 | 2 | 298-300 | Insufficient Data | Average or Better | N/A | Insufficient Data | Insufficient Data | N/A | N/A | Insufficient Data | Average or Better | N/A |
| 70-9 | 2 or 3 Lane Undivided Highway | 0 | 5 | 293-298 | Insufficient Data | Average or Better | N/A | Insufficient Data | Insufficient Data | N/A | N/A | Insufficient Data | Average or Better | N/A |
| 70-10 | 2 or 3 Lane Undivided Highway | 0 | 19 | 274-293 | 1.63 | Average or Better | High | 0.76 | 2.50 | None | High | Insufficient Data | Average or Better | N/A |
| 70-11 | 2 or 3 Lane Undivided Highway | 0 | 4 | 270-274 | 3.37 | Average or Better | High | 6.74 | 0.00 | High | None | Insufficient Data | Average or Better | N/A |
| 70-12 | 2 or 3 Lane Undivided Highway | 0 | 15 | 255-270 | 2.63 | Average or Better | High | 2.97 | 2.28 | High | High | Insufficient Data | Average or Better | N/A |
| 60\|70-13 | 2 or 3 Lane Undivided Highway | 0 | 12 | 243-255 | 2.97 | Average or Better | High | 3.36 | 2.57 | High | High | Insufficient Data | Average or Better | N/A |
| 60-14 | 2 or 3 Lane Undivided Highway | 0 | 16 | 227-243 | 1.78 | Average or Better | High | 1.498123938 | 2.068352263 | High | High | Insufficient Data | Average or Better | N/A |
| 60-15 | 2 or 3 Lane Undivided Highway | 0 | 2 | 225-227 | Insufficient Data | Average or Better | N/A | Insufficient Data | Insufficient Data | N/A | N/A | Insufficient Data | Average or Better | N/A |
| 60-16 | 2 or 3 Lane Undivided Highway | 0 | 2 | 223-225 | Insufficient Data | Average or Better | N/A | Insufficient Data | Insufficient Data | N/A | N/A | Insufficient Data | Average or Better | N/A |
| 60-17 | 2 or 3 or 4 Lane Divided Highway | 1 | 11 | 212-223 | 1.23 | Average or Better | Medium | 1.816933945 | 0.645038045 | High | None | Insufficient Data | Average or Better | N/A |
| 60-18 | 2 or 3 or 4 Lane Divided Highway | 1 | 7 | 205-212 | 0.50 | Average or Better | None | 0.906787638 | 0.089862971 | None | None | Insufficient Data | Average or Better | N/A |
| 60-19 | 2 or 3 or 4 Lane Divided Highway | 1 | 6 | 199-205 | 0.95 | Average or Better | Low | 1.623453821 | 0.268706189 | High | None | Insufficient Data | Average or Better | N/A |
| 60-20 | Urban 4 Lane Freeway | 6 | 4.7 | 194.3-199 | 1.29 | Average or Better | Medium | 1.89364123 | 0.692238644 | High | None | Insufficient Data | Average or Better | N/A |

Safety Performance Area - Needs Analysis Step 1 (continued)

| Segment | Operating Environment | Segment Length (miles) | Segment <br> Mileposts <br> (MP) | \% of Fatal + Suspected Serious Injury Crashes Involving Lane Departures |  |  | \% of Fatal + Suspected Serious Injury Crashes Involving Pedestrians |  | \% of Fatal + Suspected Serious Injury Crashes Involving Trucks |  |  | Initial <br> Need |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Performance Score | Performance Objective | Level of Need | Performance Score | Performance Objective | Performance Score | Performance Objective | Level of Need |  |
| 191-1 | 2 or 3 Lane Undivided Highway | 24 | 0-24 | Insufficient Data | Average or Better | N/A | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | None |
| 191-2 | 2 or 3 Lane Undivided Highway | 43 | 24-67 | Insufficient Data | Average or Better | N/A | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | None |
| 191-3 | 2 or 3 or 4 Lane Divided Highway | 17 | 87-104 | Insufficient Data | Average or Better | N/A | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | Low |
| 191-4 | 2 or 3 Lane Undivided Highway | 12 | 104-116 | Insufficient Data | Average or Better | N/A | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | Low |
| 191-5 | 4 or 5 Lane Undivided Highway | 5 | 116-121 | Insufficient Data | Average or Better | N/A | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | None |
| 70-6 | 4 or 5 Lane Undivided Highway | 9 | 330-339 | 25\% | Average or Better | None | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | None |
| 70-7 | 2 or 3 Lane Undivided Highway | 30 | 300-300 | Insufficient Data | Average or Better | N/A | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | None |
| 70-8 | 2 or 3 Lane Undivided Highway | 2 | 298-300 | Insufficient Data | Average or Better | N/A | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | N/A |
| 70-9 | 2 or 3 Lane Undivided Highway | 5 | 293-298 | Insufficient Data | Average or Better | N/A | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | N/A |
| 70-10 | 2 or 3 Lane Undivided Highway | 19 | 274-293 | Insufficient Data | Average or Better | N/A | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | High |
| 70-11 | 2 or 3 Lane Undivided Highway | 4 | 270-274 | Insufficient Data | Average or Better | N/A | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | High |
| 70-12 | 2 or 3 Lane Undivided Highway | 15 | 255-270 | 22\% | Average or Better | None | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | High |
| 60\|70-13 | 2 or 3 Lane Undivided Highway | 12 | 243-255 | 21\% | Average or Better | None | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | High |
| 60-14 | 2 or 3 Lane Undivided Highway | 16 | 227-243 | 81\% | Average or Better | High | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | High |
| 60-15 | 2 or 3 Lane Undivided Highway | 2 | 225-227 | Insufficient Data | Average or Better | N/A | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | N/A |
| 60-16 | 2 or 3 Lane Undivided Highway | 2 | 223-225 | Insufficient Data | Average or Better | N/A | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | N/A |
| 60-17 | 2 or 3 or 4 Lane Divided Highway | 11 | 212-223 | 78\% | Average or Better | High | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | High |
| 60-18 | 2 or 3 or 4 Lane Divided Highway | 7 | 205-212 | 17\% | Average or Better | None | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | None |
| 60-19 | 2 or 3 or 4 Lane Divided Highway | 6 | 199-205 | 60\% | Average or Better | Low | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | Medium |
| 60-20 | Urban 4 Lane Freeway | 4.7 | 194.3-199 | 50\% | Average or Better | None | Insufficient Data | Average or Better | Insufficient Data | Average or Better | N/A | Medium |

Safety Performance Area - Needs Analysis Step 2

| Segment | Segment Length (miles) | Segment Mileposts (MP) | Initial Need | Hot Spots | Relevant Recently Completed or Under Construction Projects (which supersede performance data)* | Final Need | Comments (may include tentatively programmed projects with potential to address need or other relevant issues identified in previous reports) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 191-1 | 24 | 0-24 | None |  | None | None | None |
| 191-2 | 43 | 24-67 | None |  | None | None | None |
| 191-3 | 17 | 87-104 | Low |  | None | Low | None |
| 191-4 | 12 | 104-116 | Low |  | None | Low | Road Rehabilitation SR-266 to SR-366 milepost 104.5 to 113.5 (9 mile) |
| 191-5 | 5 | 116-121 | None |  | Restripe to 5 lanes between 11th Street and US 70 (MP 120-121) | None | None |
| 70-6 | 9 | 330-339 | None | MP 336.5 - JCT US191 | Traffic Signal or Roundabout Construction (MP 335.5) <br> Construct Pedestrian Bridge Extension (MP 299-300) | Low | None |
| 70-7 | 30 | 300-300 | None |  | None | None | None |
| 70-8 | 2 | 298-300 | N/A |  | None | N/A | None |
| 70-9 | 5 | 293-298 | N/A |  | Reduced Speed from 50 MPH to 40 MPH (MP 294-298) <br> Eliminate passing zone through Bylas (MP 294.6-295.5) <br> Pedestrian Safety improvements - Pedestrian crossings, warning signs/flashing lights, ADA compliant pedestrian gates (MP 294-298) Curb installation on north side of US 70 (MP 296.5) | N/A | None |
| 70-10 | 19 | 274-293 | High |  | None | High | None |
| 70-11 | 4 | 270-274 | High |  | None | High | None |
| 70-12 | 15 | 255-270 | High |  | Install Lighting and Center Turn Lane at US 70 \& 177 intersection (US 70 Cutter Safety Improvements previous round Prioritized solution) | High | None |
| 60\|70-13 | 12 | 243-255 | High | MP 247-253.4 | Construct new sidewalks on north side (MP 243-252) <br> DMS Sign Eastbound Installed (MP 247) Lighting MP 247.6-247.9 FY 2022 F035201D, F035201C | High | Lighting installation from N Cherry Ave to Radanovich Blvd (MP 247.6-247.9) Planned for FY22 |
| 60-14 | 16 | 227-243 | High | $\begin{gathered} \hline \text { MP 241-242.6, MP } 227 \\ -232.3 \\ \hline \end{gathered}$ | None | High | None |
| 60-15 | 2 | 225-227 | N/A |  | None | N/A | None |
| 60-16 |  | 223-225 | N/A |  | None | N/A | None |
| 60-17 | 11 | 212-223 | High | MP 214.3-216.7 | Picket Post- Construct new EB lanes parallel to existing, between Reymert Wash and Queen Creek (MP 219.9-222.3) <br> Gonzales Pass- Construct new EB lanes west of the summit, construct new WB lanes east of the summit (MP 216.3-219.9) | High | None |
| 60-18 | 7 | 205-212 | None | MP 206-208 | None | Low | None |
| 60-19 | 6 | 199-205 | Medium | MP 200.4-203.5 | None | Medium | None |
| 60-20 | 4.7 | 194.3-199 | Medium | MP 195-197 | None | Medium | None |

## ADOT

Safety Performance Area - Needs Analysis Step 3

| Seament Number | 19.1 | ${ }_{192}$ | 19.3 | 19.4 | 19.5 | ${ }^{0.6}$ | ${ }_{0} 7$ | 70.8 | 0.9 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Serementestest (mines) | ${ }^{24}$ | ${ }^{43}$ | 17 | 12 | S | , | ${ }^{30}$ | 2 | 5 |
|  |  | ${ }_{\text {2. }}^{\text {24.6 }}$ | ${ }_{\text {81/ } 104}^{\text {low }}$ | ${ }_{\text {cow }}^{\text {Low } 126}$ | ${ }_{\text {Nome }}^{\text {Nom }}$ | ${ }_{\text {cow }}^{\text {low }}$ | come | N/a | 208.288 |
| micaso | 1 Crashes were fatal <br> 2 Crashes had suspected serious injuries <br> $0 \%$ Crashes at intersections <br> 0 Crashes involve lane departures <br> 0 Crashes involve pedestrians <br> 0 Crashes involve trucks <br> 0 Crashes involve bicycles | 2 Crashes were fatal <br> 2 Crashes had suspected serious injuries <br> 0 Crashes at intersections <br> 0 Crashes involve lane departures <br> 0 Crashes involve pedestrians <br> 0 Crashes involve trucks <br> 0 Crashes involve bicycles |  |  |  |  | 3 Crashes were fatal <br> 2 Crashes had suspected serious injuries <br> 0 Crashes at intersections <br> 0 Crashes involve lane departures <br> 0 Crashes involve pedestrians <br> 0 Crashes involve trucks <br> 0 Crashes involve bicycles |  |  |
| nistramut teent Tpe | Smone | Ssmoseste osomin | Smoleseceososman | 60\% Involve Collision with Motor Vehicle <br> 20\% Involve Collision with Fixed Object <br> 20\% Involve Overturning | NA. Smane ses eos mil | 42\% Involve Collision with Motor Vehicle <br> 25\% Involve Collision with Fixed Object <br> 8\% Involve Collision With Animal | $80 \%$ Involve Collision with Motor Vehicle $20 \%$ Involve Collision with Pedestrian | NAA Smompesiecoos osmal |  |
| Collion Tpe | NA. Smonesese cosomal | NA Sampese sie os smal | NA Sampesere oosmal |  |  |  |  | Na. Smme | NA, Ssmosesiecoosman |
|  | NA. Smone sese cos smal | NA Sampese se ocosmal |  |  |  |  |  | man | NA. Sampese seo oosman |
|  | NA. Smposese coso min |  | NA, Smome Steseos man | 40\% Occur in Daylight Conditions <br> 20\% Occur in Dawn Conditions <br> 20\% Occur in Dusk Conditions | NA, Smomestere oos man | $50 \%$ Occur in Daylight Conditions <br> $33 \%$ Occur in Dark-Lighted Conditions <br> $8 \%$ Occur in Dusk Conditions <br> 8\% Occur in Dusk Conditions | 80\% Occur in Dark-Unlighted Conditions 20\% Occur in Daylight Conditions | .Smone sie eos man |  |
| Sutrae Conditions | NA. Smanosese ososmal | NA. Smomesese ocosman |  |  | NA. Sampesese ososmal |  |  | NA Sampese sie oos smal |  |
|  | NA Sampese seososmal |  | NA. Smomesese coosmin | 40\% Involve a first unit event of Motor Vehicle in Transport <br> 20\% <br> Involve a first unit event of Ran Off the Road (Right) <br> 0\% Involve a first unit event of Crossed Centerline | mobse osoman | $50 \%$ Involve a first unit event of Motor Vehicle in <br>  <br> Transport <br> $25 \%$ Involve a first unit event of Ran Off the Road <br> (Right) <br> 17\% Involve a first unit event of Collision with <br> Pedestrian |  | Smpe | N/A. Smmesesme |
| 5 | NA. Smonesese cosman | NA. Smmeseste osoman |  |  | NAA Smmelesie oos man | $75 \%$ No Apparent Influence <br> $17 \%$ Illness <br> $8 \%$ Under the Influence of Drugs or Alcohol | 60\% Under the Influence of Drugs or Alcohol $20 \%$ No Apparent Influence $20 \%$ Unknown | N/A.Smple size eosmal | NA. Smpese sie cososmal |
| Statevomicu vege | NAA Smonosese cososman | NA. Smonesesie cos smal | NAA Smomesese coosman | Sen |  | $42 \%$ Shoulder And Lap Belt Used <br> $25 \%$ None Used <br>   <br> $8 \%$ Air Bag Deployed | 60\% Shoulder And Lap Belt Used 20\% Not Applicable 20\% None Used | NAA Smmelese cososman | NA. Smme |
| Horsoot Cants sumates |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  | Tin |  |  |  |

Safety Performance Area－Needs Analysis Step 3 （continued）

|  | ${ }^{298}$ | ${ }_{\text {\％} 0,10}^{19}$ | ${ }^{0.14}$ | 20.15 | ${ }^{\text {col0 } 13}$ | 60.18 | 6.15 | 60．16 | 6017 | 60．18 | 60.19 | 6820 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | ${ }_{\text {2 }}^{24}$ | 19 <br> 24.23 <br> 18 | ${ }^{20.274}$ | ${ }_{\text {25 } 230}$ | ${ }_{\text {23，} 25}^{25}$ | ${ }_{\substack{16 \\ 27.23 \\ \hline 23}}$ | ${ }_{\text {coser }}$ | $\substack{\text { Noer } \\ \text { Hed }}$ |  | 。 | 。 | $\bigcirc$ |  |
| fran weed | Nome | mos | mon | mos | mon | mos | N／A | N／ | Hom | ，ow | meamm | Neatim |  |
| micasom | \％ $\begin{gathered}1 \\ 2 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0\end{gathered}$ |  |  | 7 Crashes were fatal <br> 2 Crashes had suspected serious injuries <br> 0 Crashes at intersections <br> 0 Crashes involve lane departures <br> 1 Crashes involve pedestrians <br> 0 Crashes involve trucks <br> 0 Crashes involve bicycles |  |  | 0 Crashes were fatal <br> 0 Crashes had suspected <br> 0 serious injuries <br> 0 Crashes at intersections <br> 0 Crashes involve lane <br> departures <br> 0 Crashes involve pedestrians <br> 0 Crashes involve trucks <br> 0 Crashes involve bicycles |  | 4 Crashes were fatal <br> 5 Crashes had suspected serious injuries <br> 0 Crashes at intersections <br> 1 Crashes involve lane departures <br> 0 Crashes involve pedestrians <br> 0 Crashes involve trucks <br> 0 Crashes involve bicycles |  | 2 Crashes were fatal <br> 8 Crashes had suspected serious injuries <br> 1 Crashes at intersections <br> 1 Crashes involve lane departures <br> 0 Crashes involve pedestrians <br> 0 Crashes involve trucks <br> 0 Crashes involve bicycles |  | 47 Crashes were fatal <br> 91 Crashes had suspected serious <br> injuries  <br> 4 Crashes at intersections <br> 11 Crashes involve lane departures <br> 2 Crashes involve pedestrians <br> 0 Crashes involve trucks <br> 1 Crashes involve bicycles |
| rextumut vent tre |  | $43 \%$ Involve Collision with Fixed Object $14 \%$ Involve Collision with Pedalcyclist <br> 14\％Involve Other Non－Collision | N／ASmote iecto osman | $44 \%$ Involve Collision with Motor Vehicle $33 \%$ Involve Collision with Pedestrian $22 \%$ Involve Collision with Fixed Object | $8 \%$ Involve Overturning <br> $4 \%$ Involve Collision with Pedestrian | 54\％Involve Collision with Fixed Object 35\％Involve Collision with Motor Vehicle $4 \%$ Involve Collision with |  | Na／Smome sec oos man | $44 \%$ Involve Overturning $33 \%$ Involve Collision with Motor Vehicle $11 \%$ Involve Collision with Pedestrian | 67\％Involve Collision with Motor Vehicle <br> 17\％Involve Collision with Non－Fixed Object <br> 17\％Involve Overturning | $40 \%$ Involve Overturning $40 \%$ Involve Collision with Motor Vehicle 20\％Involve Collision with Fixed Object | $33 \%$ Involve Overturning <br> 17\％Involve Collision with Fixed Object |  |
| Colles |  |  | man | $\begin{aligned} & \hline 44 \% \text { Involve Other } \\ & 22 \% \text { Involve Rear End } \\ & 22 \% \text { Involve Single Vehicle } \end{aligned}$ | $\begin{aligned} & \hline 29 \% \text { Involve Angle } \\ & 21 \% \text { Involve Rear End } \\ & 17 \% \text { Involve Single Vehicle } \end{aligned}$ | 54\％Involve Single Vehicle <br> 19\％Involve Head 15\％Involve Other |  | NA．Smamesese osoman | $56 \%$ Involve Single Vehicle <br> $11 \%$ Involve Head On <br> $11 \%$ Involve Sideswipe（same） | 50\％Involve Rear End $33 \%$ Involve Single Vehicle $17 \%$ Involve Angle | 40\％Involve Single Vehicle $30 \%$ Involve Rear End |  | $\begin{array}{\|ll\|} \hline 40 \% & \text { Involve Single Vehicle } \\ 25 \% & \text { Involve Rear End } \\ 12 \% & \text { Involve Other } \end{array}$ |
|  |  | $\begin{aligned} & 29 \% \text { Involve No Improper Action } \\ & 29 \% \text { Involve Unknown } \\ & 14 \% \text { Involve Failure to Yield Right-of-Way } \end{aligned}$ |  | $44 \%$ Involve Unknown $33 \%$ Involve Speed too Fast for Conditions |  |  | NA．Smonosese orosmul | NA．Smome sese osoman | 22\％Involve Unknown $11 \%$ Involve Drove in Opposing Lane | ${ }^{\text {B3x }}$ | $100 \%$ Involve No Improper Action $60 \%$ Involve Exceeded Lawful Speed $10 \%$ Involve Unknown | 100\％Involve No Improper Action 67\％Involve Exceeded Lawful Speed 17\％Involve Other Unsafe Passing |  |
| ugtura comations |  | 43\％Occur in Daylight Conditions <br> 29\％Occur in Dark－Unknown Lighting <br> 29\％Occur in Dark－Unlighted Conditions | W．Smene sectoosman |  |  |  |  | VA．Smpeseseseosman | 44\％Occur in Dark－Unlighted Conditions 44\％Occur in Daylight Conditions 11\％Occur in Dawn Conditions | $50 \%$ Occur in Daylight Conditions $33 \%$ Occur in Dark－Unlighted Conditions $17 \%$ Occur in Dawn Conditions | 60\％Occur in Daylight Conditions 30\％Occur in Dark－Unlighted Conditions <br> 10\％Occur in Dark－Lighted Conditions |  | $62 \%$ Occur in Daylight Conditions <br> $16 \%$ Occur in Dark－Lighted Conditions <br> $14 \%$ Occur in Dark－Unlighted |
| Sutase conditions |  |  | NA．Smone siesoosman | ${ }^{\text {aseme }}$ |  | $\underbrace{\text { ans }}$ | NA Smpmeseses oos sman | Vat Smpeneseceos osmal | Lex move onconstios |  |  | \％moneoncoome |  |
| Untevent |  |  |  | $22 \%$ Involve a first unit event of Overturn <br> 11\％Collision with Pedestrian |  |  |  |  |  |  |  |  | \％\％ |
| ampmialcontion |  | $$ | WA．Smpereseseosomman | $67 \%$ Unknown $11 \%$ Fatigued／Fell Asleep $11 \%$ Under the Influence of Drugs or Alcohol | $\begin{aligned} & \text { 54\% No Apparent Influence } \\ & 25 \% \text { Unknown } \\ & \text { 17\% Under the Influence of Drugs or Alcohol } \end{aligned}$ | $$ |  | VA．Smoseseseocosman | Centerline 33\％Unknown 33\％Under the Influence of Drugs or Alcohol 22\％No Apparent Influence |  | （Left） 40\％No Apparent Influence $30 \%$ Under the Influence of Drugs or Alcohol 20\％Fatigued／Fell Asleep | Fixed Object 40\％No Apparent Influence $30 \%$ Under the Influence of Drugs or Alcohol 20\％Fatigued／Fell Asleep |  |
| State oemie usere |  |  |  |  |  |  | Na．Smomesiesoosman | Na Sampesese oos man |  | （en | （tas | $\begin{aligned} & \text { 67\% Shoulder And Lap Belt Used } \\ & 17 \% \text { Unknown } \\ & 17 \% \text { None Used } \end{aligned}$ |  |
| notsooc casas summese |  |  |  |  | M1227． 23.4 |  |  |  | we2213－21267 | ${ }_{\text {w2206 } 288}$ | wn2004 2035 | mer 15 －197 |  |
|  |  |  |  | Men |  |  |  |  |  |  |  |  |  |

Freight Performance Area - Needs Analysis Step 1

| Segment | Facility Operations | Segment Mileposts (MP) | Segment Length (miles) | Freight Index |  |  | Directional TTTR (trucks only) |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Performance Score | Performance Objective | Level of Need | Performance Score |  | Performance Objective | Level of Need |  |
|  |  |  |  |  |  |  | NB/EB | SB/WB |  | NB/EB | SB/WB |
| 191-1* | Interrupted | 1-24 | 24 | 2.26 | Fair or Better | High | 1.40 | 1.39 | Fair or Better | None | None |
| 191-2* | Interrupted | 24-67 | 43 | No Data | Fair or Better | N/A | No Data | No Data | Fair or Better | N/A | N/A |
| 191-3^ | Uninterrupted | 87-104 | 17 | No Data | Fair or Better | N/A | No Data | No Data | Fair or Better | N/A | N/A |
| 191-4^ | Uninterrupted | 104-116 | 12 | No Data | Fair or Better | N/A | No Data | No Data | Fair or Better | N/A | N/A |
| 191-5* | Interrupted | 116-121 | 5 | No Data | Fair or Better | N/A | No Data | No Data | Fair or Better | N/A | N/A |
| 70-6* | Interrupted | 339-330 | 9 | No Data | Fair or Better | N/A | No Data | No Data | Fair or Better | N/A | N/A |
| 70-7^ | Uninterrupted | 330-300 | 30 | No Data | Fair or Better | N/A | No Data | No Data | Fair or Better | N/A | N/A |
| 70-8^ | Uninterrupted | 300-298 | 2 | No Data | Fair or Better | N/A | No Data | No Data | Fair or Better | N/A | N/A |
| 70-9^ | Uninterrupted | 298-293 | 5 | No Data | Fair or Better | N/A | No Data | No Data | Fair or Better | N/A | N/A |
| 70-10^ | Uninterrupted | 293-274 | 19 | No Data | Fair or Better | N/A | No Data | No Data | Fair or Better | N/A | N/A |
| 70-11^ | Uninterrupted | 274-270 | 4 | No Data | Fair or Better | N/A | No Data | No Data | Fair or Better | N/A | N/A |
| 70-12^ | Uninterrupted | 270-255 | 15 | No Data | Fair or Better | N/A | No Data | No Data | Fair or Better | N/A | N/A |
| 70/60E-13* | Interrupted | 255-243 | 12 | 1.58 | Fair or Better | None | 1.16 | 1.15 | Fair or Better | None | None |
| 60E-14^ | Uninterrupted | 243-227 | 16 | 1.49 | Fair or Better | High | 1.12 | 1.17 | Fair or Better | None | None |
| 60E-15^ | Uninterrupted | 227-225 | 2 | 1.32 | Fair or Better | Medium | 1.18 | 1.14 | Fair or Better | None | None |
| 60E-16^ | Uninterrupted | 225-223 | 2 | 1.28 | Fair or Better | Low | 1.05 | 1.12 | Fair or Better | None | None |
| 60E-17^ | Uninterrupted | 223-212 | 11 | 1.18 | Fair or Better | None | 1.05 | 1.09 | Fair or Better | None | None |
| 60E-18^ | Uninterrupted | 212-205 | 7 | 1.22 | Fair or Better | Low | 1.12 | 1.05 | Fair or Better | None | None |
| 60E-19* | Interrupted | 205-199 | 6 | 1.63 | Fair or Better | Low | 1.20 | 1.14 | Fair or Better | None | None |
| 60E-20^ | Uninterrupted | 199-194.3 | 4.7 | 1.20 | Fair or Better | None | 1.06 | 1.06 | Fair or Better | None | None |
| Emphasis Area? | Yes | Weighted Corridor Average |  | 1.64 | Good | High |  |  |  |  |  |

Freight Performance Area - Needs Analysis Step 1 (continued)

| Segment | Facility Operations | Segment Mileposts (MP) | Segment Length (miles) | Closure Duration (minutes/mile/year) |  |  |  |  | Bridge Clearance (feet) |  |  | Initial Need |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Performance Score |  | Performance Objective | Level of Need |  | Performance Score | Performance Objective | Level of Need |  |
|  |  |  |  | NB/EB | SB/WB |  | NB/EB | SB/WB |  |  |  |  |
| 191-1* | Interrupted | 1-24 | 24 | 3.02 | 1.00 | Fair or Better | None | None | No UP | Fair or Better | None | High |
| 191-2* | Interrupted | 24-67 | 43 | 2.67 | 1.78 | Fair or Better | None | None | 22.04 | Fair or Better | None | N/A |
| 191-3^ | Uninterrupted | 87-104 | 17 | 2.47 | 0.00 | Fair or Better | None | None | No UP | Fair or Better | None | N/A |
| 191-4^ | Uninterrupted | 104-116 | 12 | 12.23 | 5.00 | Fair or Better | None | None | No UP | Fair or Better | None | N/A |
| 191-5* | Interrupted | 116-121 | 5 | 26.08 | 16.96 | Fair or Better | None | None | None | Fair or Better | None | N/A |
| 70-6* | Interrupted | 339-330 | 9 | 1.33 | 4.67 | Fair or Better | None | None | No UP | Fair or Better | None | N/A |
| 70-7^ | Uninterrupted | 330-300 | 30 | 4.55 | 5.40 | Fair or Better | None | None | 17.03 | Fair or Better | None | N/A |
| 70-8^ | Uninterrupted | 300-298 | 2 | 14.30 | 0.00 | Fair or Better | None | None | No UP | Fair or Better | None | N/A |
| 70-9^ | Uninterrupted | 298-293 | 5 | 2.40 | 3.00 | Fair or Better | None | None | None | Fair or Better | None | N/A |
| 70-10^ | Uninterrupted | 293-274 | 19 | 8.63 | 2.51 | Fair or Better | None | None | No UP | Fair or Better | None | N/A |
| 70-11^ | Uninterrupted | 274-270 | 4 | 0.00 | 0.00 | Fair or Better | None | None | No UP | Fair or Better | None | N/A |
| 70-12^ | Uninterrupted | 270-255 | 15 | 17.39 | 0.00 | Fair or Better | None | None | No UP | Fair or Better | None | N/A |
| 70/60E-13* | Interrupted | 255-243 | 12 | 22.75 | 26.52 | Fair or Better | None | None | 15.84 | Fair or Better | Medium | Low |
| 60E-14^ | Uninterrupted | 243-227 | 16 | 63.60 | 344.95 | Fair or Better | None | High | 13.03 | Fair or Better | High | High |
| 60E-15^ | Uninterrupted | 227-225 | 2 | 0.00 | 90.50 | Fair or Better | None | Low | 16.79 | Fair or Better | None | Medium |
| 60E-16^ | Uninterrupted | 225-223 | 2 | 52.20 | 12.25 | Fair or Better | None | None | No UP | Fair or Better | None | Low |
| 60E-17^ | Uninterrupted | 223-212 | 11 | 3.27 | 61.40 | Fair or Better | None | None | No UP | Fair or Better | None | None |
| 60E-18^ | Uninterrupted | 212-205 | 7 | 0.00 | 22.29 | Fair or Better | None | None | No UP | Fair or Better | None | Low |
| 60E-19* | Interrupted | 205-199 | 6 | 14.00 | 20.30 | Fair or Better | None | None | No UP | Fair or Better | None | Low |
| 60E-20^ | Uninterrupted | 199-194.3 | 4.7 | 74.94 | 7.11 | Fair or Better | Low | None | No UP | Fair or Better | None | Low |

Freight Performance Area - Needs Analysis Step 2

| Segment | Segment Length (miles) | Segment Mileposts (MP) | Initial Need | Truck Height Restriction Hot Spots (Clearance < 16.25') | Relevant Recently Completed or Under Construction Projects (which supersede performance data)* | Final Need | Comments (may include tentatively programmed projects with potential to address needs or other relevant issues identified in previous reports) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 191-1* | 24 | 1-24 | High |  |  | High |  |
| 191-2* | 43 | 24-67 | N/A |  |  | N/A |  |
| 191-3^ | 17 | 87-104 | N/A |  |  | N/A |  |
| 191-4^ | 12 | 104-116 | N/A |  |  | N/A | Restripe to 5 lanes between Atresia Road and Lebanon Road (to 2023) MP 110.9-116 |
| 191-5* | 5 | 116-121 | N/A |  |  | N/A | One mile of planned restriping to 5 lanes between 11th Street and US 70 |
| 70-6* | 9 | 339-330 | N/A |  |  | N/A |  |
| 70-7^ | 30 | 330-300 | N/A |  |  | N/A |  |
| 70-8^ | 2 | 300-298 | N/A |  |  | N/A |  |
| 70-9^ | 5 | 298-293 | N/A |  |  | N/A |  |
| 70-10^ | 19 | 293-274 | N/A |  |  | N/A |  |
| 70-11^ | 4 | 274-270 | N/A |  |  | N/A |  |
| 70-12^ | 15 | 270-255 | N/A |  |  | N/A |  |
| $\begin{gathered} \text { 70/60E- } \\ 13^{*} \end{gathered}$ | 12 | 255-243 | Low | Pinal SPRR UP | FY19 MP 247 New DMS Sign EastBound (Arizona Statewide DMS Plan) and Construct new sidewalks on northside from MP 243-252 (Cobre Valley Comprehensive Transportation Study) | Low | Projects planned: to Construct alternative alignment/Widen to 4 lanes (2030) MP243-252; to implement access management through Miami (2030) 243-245.5 |
| 60E-14^ | 16 | 243-227 | High | Queen Creek <br> Tunnel |  | High | Projects Planned: to construct alternative alignment/Widen to 4 lanes (2030) MP243-252; to implement access management through Miami (2030) throughout the segment |
| 60E-15^ | 2 | 227-225 | Medium |  |  | Medium | Projects Planned: to construct alternative alignment/Widen to 4 lanes (2030) MP243-252; Implement access management through Miami (2030) throughout the segment |
| 60E-16^ | 2 | 225-223 | Low |  |  | Low | Projects Planned: to construct alternative alignment/Widen to 4 lanes (2030) MP243-252; Implement access management through Miami (2030) throughout the segment |
| 60E-17^ | 11 | 223-212 | None |  | Picket Post- Construct new EB lanes parallel to existing, between Reymert Wash and Queen Creek and Gonzales Pass- Construct new EB lanes west of the summit, construct new WB lanes east of the summit | None | Projects Planned: to construct alternative alignment/Widen to 4 lanes (2030) MP243-252; Implement access management through Miami (2030) throughout the segment |
| 60E-18^ | 7 | 212-205 | Low |  |  | Low |  |
| 60E-19* | 6 | 205-199 | Low |  |  | Low |  |
| 60E-20^ | 4.7 | 199-194.3 | Low |  |  | Low |  |

Freight Performance Area - Needs Analysis Step 3

|  |  |  |  | Roadway Variables |  |  |  |  |  |  |  | Traffic Variables |  |  | Relevant Freight Related Existing Infrastructure |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Segment | Mileposts (MP) | Length (miles) | Final Need | Functional Classification | Environmental Type (Urban/Rural) | Terrain | \# of Lanes/ Direction | Weighted Average Speed Limit | Aux <br> Lanes | Divided/ NonDivided | \% No Passing | Existing LOS | Future 2040 LOS | \% Trucks |  |
| 191-1* | 1-24 | 24 | High | State Highway | Rural | Level | 2 | 55 | No | Undivided | 12\% | A/B | A/B | 15\% | This segment includes one rest area |
| 191-2* | 24-67 | 43 | N/A | State Highway | Rural | Level | 2 | 55 | No | Undivided | 26\% | A/B | A/B | 27\% | This segment includes a Border patrol Check Point effecting NB traffic |
| 191-3^ | 87-104 | 17 | N/A | State Highway | Rural | Level | 4 | 55 | No | Divided | 3\% | A/B | A/B | 21\% |  |
| 191-4^ | 104-116 | 12 | N/A | State Highway | Rural | Level | 4 | 65 | No | Undivided | 30\% | A/B | A/B | 16\% |  |
| 191-5* | 116-121 | 5 | N/A | State Highway | Urban | Level | 4 | 40 | No | Undivided | 13\% | A-C | A-C | 10\% |  |
| 70-6* | 339-330 | 9 | N/A | State Highway | Urban | Level | 4 | 40 | No | Undivided | 0\% | A-C | A-C | 13\% | This segment includes one rest area |
| 70-7^ | 330-300 | 30 | N/A | State Highway | Rural | Level | 2 | 55 | No | Undivided | 13\% | A/B | A/B | 27\% |  |
| 70-8^ | 300-298 | 2 | N/A | State Highway | Rural | Level | 2 | 65 | No | Undivided | 6\% | A/B | A/B | 25\% |  |
| 70-9^ | 298-293 | 5 | N/A | State Highway | Rural | Level | 2 | 50 | No | Undivided | 53\% | A/B | A/B | 25\% | This segment includes one rest area |
| 70-10^ | 293-274 | 19 | N/A | State Highway | Rural | Level | 2 | 55 | No | Undivided | 37\% | A/B | A/B | 25\% |  |
| 70-11^ | 274-270 | 4 | N/A | State Highway | Rural | Level | 2 | 55 | No | Undivided | 77\% | A/B | A/B | 25\% |  |
| 70-12^ | 270-255 | 15 | N/A | State Highway | Rural | Level | 2 | 60 | No | Undivided | 10\% | A/B | A/B | 17\% | This segment includes a climbing/passing lane |
| $\begin{gathered} \text { 70/60E- } \\ 13^{*} \end{gathered}$ | 255-243 | 12 | Low | State Highway | Urban | Level | 4 | 45 | No | Undivided | 0\% | A-C | A-C | 11\% | This segment includes one eastbound DMS. |
| 60E-14^ | 243-227 | 16 | High | State Highway | Rural | Mountainous | 2 | 50 | No | Undivided | 68\% | D-F | D-F | 11\% |  |
| 60E-15^ | 227-225 | 2 | Medium | State Highway | Rural | Rolling | 2 | 45 | No | Undivided | 98\% | D-F | D-F | 18\% | This segment includes one rest area |
| 60E-16^ | 225-223 | 2 | Low | State Highway | Rural | Level | 2 | 55 | No | Undivided | 55\% | A/B | D-F | 13\% |  |
| 60E-17^ | 223-212 | 11 | None | State Highway | Rural | Level | 4 | 65 | No | Divided | 0\% | A/B | A/B | 13\% |  |
| 60E-18^ | 212-205 | 7 | Low | State Highway | Rural | Level | 2 | 65 | No | Divided | 0\% | A/B | C | 13\% | This segment includes one eastbound DMS. |
| 60E-19* | 205-199 | 6 | Low | State Highway | Urban | Level | 2 | 55 | No | Divided | 0\% | E/F | D | 11\% |  |
| 60E-20^ | 199-194.3 | 4.7 | Low | State Highway | Urban | Level | 2 | 65 | No | Divided | 0\% | A-C | A-C | 9\% | This segment includes one eastbound DMS. |

## Needs Summary Table

| Segment Number and Mileposts (MP) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Performanc e Area | 191-1 | 191-2 | 191-3 | 191-4 | 191-5 | 70-6 | 70-7 | 70-8 | 70-9 | 70-10 | 70-11 | 70-12 | $\begin{gathered} 70 / 60 \mathrm{E}- \\ 13 \end{gathered}$ | 60E-14 | 60E-15 | 60E-16 | 60E-17 | 60E-18 | 60E-19 | 60E-20 |
|  | $\begin{gathered} \text { MP } \\ 0-24 \end{gathered}$ | $\begin{gathered} \text { MP } \\ 24-67 \end{gathered}$ | $\begin{aligned} & \hline \text { MP } \\ & 87- \\ & 104 \\ & \hline \end{aligned}$ | MP <br> $104-$ <br> 116 | $\begin{gathered} \hline \text { MP } \\ 116- \\ 121 \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { MP } \\ 339- \\ 330 \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { MP } \\ 330- \\ 300 \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { MP } \\ 300- \\ 298 \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { MP } \\ 298- \\ 293 \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { MP } \\ 293- \\ 274 \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { MP } \\ 274- \\ 270 \\ \hline \end{gathered}$ | $\begin{aligned} & \hline \text { MP } \\ & 270- \\ & 255 \\ & \hline \end{aligned}$ | $\begin{gathered} \text { MP } \\ 255-243 \end{gathered}$ | $\begin{gathered} \hline \text { MP } \\ 243- \\ 227 \\ \hline \end{gathered}$ | $\begin{aligned} & \hline \text { MP } \\ & 227- \\ & 225 \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline \text { MP } \\ & 225- \\ & 223 \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline \text { MP } \\ & 223- \\ & 212 \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline \text { MP } \\ & 223- \\ & 213 \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline \text { MP } \\ & 223- \\ & 214 \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline \text { MP } \\ & 223- \\ & 215 \\ & \hline \end{aligned}$ |
| Pavement | $\begin{gathered} \hline \text { Mediu } \\ \mathrm{m} \end{gathered}$ | High | Low | Low | Medium | Medium | High | High | High | High | High | Low | Medium | Low | None | None | Low | Medium | Low | None |
| Bridge | None | Mediu m | Low | Low | N/A | Low | Low | None | N/A | None | Low | Low | Medium | Medium | None | Medium | Low | Low | Low | None |
| Mobility* | Low | None | Low | None | Low | Low | Low | Low | Low | Low | Low | Low | Low | High | Low | Low | None | Low | High | High |
| Safety* | None | None | Low | Low | None | Low | None | N/A | N/A | High | High | High | High | High | N/A | N/A | High | Low | Medium | Medium |
| Freight* | High | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | N/A | Low | High | Medium | Low | None | Low | Low | Low |
| Average Need | 1.23 | 0.77 | 0.77 | 0.54 | 0.54 | 0.92 | 0.85 | 0.69 | 0.69 | 1.38 | 1.54 | 1.23 | 1.77 | 2.54 | 0.69 | 0.77 | 1.00 | 1.15 | 1.69 | 1.38 |


| Level of Need | Average Need Range |
| :---: | :---: |
| None* | $<0.1$ |
| Low | $0.1-1.0$ |
| Medium | $1.0-2.0$ |
| High | $>2.0$ |

Identified as Emphasis Areas for US60|US 70|US 191 Corridor
N/A indicates insufficient or no data available to determine level of need


## Aロロт

## Appendix E: Life-Cycle Cost Analysis

Pavement Life-Cycle Cost Analysis Worksheet


Pavement Improvement Project History

## US191 Pavement Preservations South of Safford MP 87-MP 104

| Year | Project Number | Tracs No. | Direction of Improvement | Treatment Type | Improvement Description | Thickness (inches) | Beg. MP | End MP | Length <br> (miles) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2005 | STP-191-B(003)B | H503703C | NB/SB | Asphalt Reconstruction | New AB | 8 | 87 | 91 | 4 |
|  |  |  |  | Asphalt Reconstruction | New AC | 5 | 87 | 91 | 4 |
|  |  |  |  | Asphalt Reconstruction | New ACFC | 0.5 | 87 | 91 | 4 |
| 2005 | STP-191-B(004)B | H503706C | NB/SB | Asphalt Reconstruction | AB | 8 | 97.5 | 99.5 | 2 |
|  |  |  |  | Asphalt Reconstruction | AC | 5 | 97.5 | 99.5 | 2 |
|  |  |  |  | Asphalt Reconstruction | AR-ACFC | 0.5 | 97.5 | 99.5 | 2 |
| 2008 | - | H735401C | NB/SB | Asphalt Light Rehab | Seal Coat | 0.3 | 92 | 93.5 | 1.5 |
| 2008 | - | H736101C | NB/SB | Asphalt Light Rehab | Seal Coat | 0.3 | 96 | 104 | 8 |
| 2009 | STP -191-B(006)B | H503705C | NB/SB | Asphalt Reconstruction | $A B$ | 4 | 93.5 | 98 | 4.5 |
|  |  |  |  | Asphalt Reconstruction | AC | 5 | 93.5 | 98 | 4.5 |
|  |  |  |  | Asphalt Reconstruction | AR-ACFC | 0.5 | 93.5 | 98 | 4.5 |
| 2011 | ARRA-191-B(200)A | H503704C | NB/SB | Asphalt Reconstruction | AB | 8 | 91.5 | 95 | 3.5 |
|  |  |  |  | Asphalt Reconstruction | Remove | 0.5 | 91.5 | 95 | 3.5 |
|  |  |  |  | Asphalt Reconstruction | AR-ACFC | 0.5 | 91.5 | 95 | 3.5 |
| 2012 | STP-191-B(201)A | H818501C | NB/SB | Asphalt Medium Rehab | AC | 2.5 | 100 | 103.5 | 3.5 |
|  |  |  |  | Asphalt Medium Rehab | AR-ACFC | 0.5 | 100 | 103.5 | 3.5 |
| 2018 | STP-191-Y(200)T | F007701C | NB/SB | Asphalt Medium Rehab | Remove | 2.5 | 87 | 91 | 4 |
|  |  |  |  | Asphalt Medium Rehab | AC | 2.5 | 87 | 91 | 4 |
|  |  |  |  | Asphalt Medium Rehab | DC | 0.6 | 87 | 91 | 4 |
|  |  |  |  |  |  |  |  |  |  |
| Interval between Improvements in Years |  |  |  | Treatment Type Options | Estimated Historical Interval Value between Improvements in Years |  |  |  |  |
| After Asphalt Reconstruction: 4 <br> After Asphalt Reconstruction: 2 <br> After Asphalt Medium Rehab: 6 |  |  |  | Concrete Reconstruction | 4 |  |  |  |  |
|  |  |  |  | Asphalt Reconstruction |  |  |  |  |  |
|  |  |  |  | Concrete Medium Rehab |  |  |  |  |  |
|  |  |  |  | Concrete Light Rehab |  |  |  |  |  |
|  |  |  |  | Asphalt Medium Rehab | 6 |  |  |  |  |
|  |  |  |  | Asphalt Light Rehab |  |  |  |  |  |

Design Alternative \# 1 - Concrete Reconstruction
US191 Pavement Preservations South of Safford MP 87 - MP 104


Design Alternative \# 3 - Asphalt Medium Rehab
US191 Pavement Preservations South of Safford MP 87 - MP 104


## Summary of LCCA Results

US191 Pavement Preservations South of Safford MP 87 - MP 104

|  | Concrete Reconstruction | Asphalt Reconstruction | Asphalt Medium Rehab Focus | Asphalt Light Rehab Focus |
| :--- | ---: | ---: | ---: | ---: |
| Net Present Value - 3\% | $\$ 262,287,676$ | $\$ 240,087,944$ | $\$ 221,616,023$ | $\$ 209,767,952$ |
| Net Present Value - $7 \%$ | $\$ 209,609,168$ | $\$ 180,939,969$ | $\$ 131,013,653$ | $\$ 132,066,536$ |
| Agency Cost | $\$ 328,651,059$ | $\$ 320,186,021$ | $\$ 352,562,679$ | $\$ 310,274,799$ |

Cost Ratio at 3\% Discount Rate
1.25 Ratio of Concrete Reconstruction to Lowest Cost Rehab
1.14 Ratio of Asphalt Reconstruction to Lowest Cost Rehab

Cost Ratio at 7\% Discount Rate
1.60 Ratio of Concrete Reconstruction to Lowest Cost Rehab
1.38 Ratio of Asphalt Reconstruction to Lowest Cost Rehab

Note: A cost ratio < 1.15 means the Net Present Value (NPV) of reconstruction is within $15 \%$ of the NPV of the lowest cost rehab so reconstruction should likely be the initial improvement solution. A cost ratio > 1.15 means the NPV of reconstruction is more than $15 \%$ of the NPV of the lowest cost rehab so rehab should likely be the initial improvement solution.


Pavement Life-Cycle Cost Analysis Workshee


## ADOT



| US70 MP 293 - MP 298 |  | Design Alternative \# 1 - Concrete Reconstruction |  |  |  | Design Alternative \# 2 - Asphalt Reconstruction |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Name of Design Alternativer |  |  |  |  |  | er Name of Design Alternative |  |  |  |
| Number of Years | Year | Concrete Reconstruction | Agency Cost (\$) | Net Present Value @ 3\% | Net Present Value @ $¢ 7$ | Number of Years | Year | Asphalt Reconstruction | Agency Cost (s) | Net Present Value @ 3\% | Net Present Value @ 7\% |
| 0 | 2022 | None | 50 | \$0 | so | 0 | 2022 | None | 50 | \$0 | 50 |
| 1 | 2023 | None | \$0 | \$0 | \$0 | , | 2023 | None | \$0 | \$0 | \$0 |
| 2 | 2024 | None | \$0 | \$0 | \$0 | 2 | 2024 | None | \$0 | \$0 | \$0 |
| 3 | 2025 | None | \$0 | \$0 | \$0 | 3 | 2025 | None | \$0 | \$0 | \$0 |
| 4 | 2026 | None | \$0 | \$0 | \$0 | 4 | 2026 | None | \$0 | \$0 |  |
| 5 | ${ }_{2}^{2027}$ | Concrete Reconstruction | \$35,365, 848 | \$31,422,098 | \$26,980,436 | 5 | 2027 | Asphalt Reconstruction | \$28,281,064 | \$25,127,359 | \$21,57, ${ }^{\text {a }}$ 88 |
| ${ }^{6}$ | 2028 | None | \$0 | \$0 |  | 6 | 2028 | None | \$0 | \$0 | \$0 |
| 7 | 2029 2030 | None | \$0 | $\$ 0$ 50 | \$0 | 7 | 2029 | None | 50 | so | \$0 |
| 8 | 2030 | None | \$0 | \$0 | \$0 | 8 | 2030 | None | s0 | \$0 | \$0 |
| 9 | 2031 | None | \$0 | \$0 | \$0 | 9 | 2031 | None | 50 | \$0 |  |
| 10 | ${ }_{2032}^{2033}$ | None | \$0 | \$0 so | \$0 | 10 | 2032 | None | 50 | \$0 | \$0 |
| $\begin{aligned} & 11 \\ & 12 \end{aligned}$ | 2033 2034 | None None | \$0 | \$0 | \$00 | 11 | 2033 | None | \$0 | \$0 |  |
| 13 | 2035 | None | \$0 | \$0 | 50 | 12 | 2034 | None | 50 | 50 |  |
| 14 | 2036 | None | \$0 | \$0 | \$0 | 14 | 2036 | None | 50 | so | 50 |
| 15 | 2037 | None | so | \$0 | \$0 | 15 | 2037 | None | so | \$0 | so |
| 16 | 2038 | None | \$0 | \$0 | \$0 | 16 | 2038 | None | \$0 | \$0 | \$0 |
| 17 | 2039 | None | \$0 | \$0 | \$0 | 17 | 2039 | None | 50 | \$0 | so |
| 18 | 2040 2041 | None None | \$0 So | \$0 so |  | 18 | 2040 | $\xrightarrow{\text { None }}$ | 50 $\$ 531588$ | ${ }^{50}$ |  |
| $\begin{aligned} & 19 \\ & 20 \end{aligned}$ | ${ }_{2042}^{2041}$ | None None | \$0 | \$0 \$0 | \$0 $\$ 0$ | ${ }_{20}^{19}$ | 2041 2042 | Asphalt Light Rehab None cen | \$5,313,588 ${ }_{\text {So }}$ | \$3,121,173 \$0 | \$1,572,099 ${ }_{\text {sol }}$ |
| 21 | 2043 | Concrete Light Rehab | \$3,789,198 | \$2,097,987 | \$979,201 | 21 | 2043 | None | so | \$0 | \$0 |
| 22 | 2044 | None | \$0 | \$0 | \$0 | 22 | 2044 | None | \$0 | \$0 | \$0 |
| 23 | 2045 | None | 50 | \$0 | \$0 | 23 | 2045 | None | \$0 | \$0 | \$0 |
| 24 | 2046 | None | \$0 | \$0 | \$0 | 24 | 2046 | None | \$0 | \$0 | \$0 |
| 25 | 2047 | None | so | \$0 | \$0 | 25 | 2047 | None | 50 | \$0 | so |
| 26 | 2048 | None | 50 | \$0 | 50 | 26 | 2048 | None | \$0 | \$0 | \$0 |
| 27 | 2049 2050 | None | \$0 | \$0 | \$0 | 27 | 2049 | Asphat Medium Rehab | \$7,970,382 | \$3,695,824 | \$1,372,464 |
| 28 | 2050 | None None | \$0 | \$0 | \$0 | 28 | 2050 | None | 50 | \$0 | so |
| 29 30 | 2051 2052 | None None | \$0 so | \$00 | [ $\begin{array}{r}\text { \$0 } \\ 50 \\ 50\end{array}$ | 29 30 | 2051 2052 | None | \$0 | \$0 | S0 |
| $\begin{aligned} & 30 \\ & 31 \end{aligned}$ | ${ }_{2053}^{2052}$ | None Concrete Medium Rehab | \$5,705,574 | ( $\begin{array}{r}\text { \$0 } \\ \$ 2,50,621\end{array}$ | [ $\begin{array}{r}\text { \$0 } \\ \text { \$79,525 }\end{array}$ | 30 31 | 2052 2053 | None None | \$0 | \$0 | \$00 |
| 32 | 2054 | None | \$0 | \$0 | \$0 | 32 | 2054 | None | 50 | \$0 | S0 |
| 33 | 2055 | None | \$0 | \$0 | \$0 | 33 | 2055 | None | \$0 | so | So |
| 34 | 2056 | None | \$0 | \$0 | \$0 | 34 | 2056 | None | \$0 | \$0 | so |
| 35 | 2057 | None | \$0 | \$0 | 50 | 35 | 2057 | None | 50 | \$0 | so |
| ${ }^{36}$ | 2058 | None | \$0 | \$0 | \$0 | ${ }_{37} 36$ | 2058 | None | \$0 | \$0 | ${ }_{50}$ |
| 37 | 2059 | None | \$0 | \$0 | 50 | 37 | 2059 | None | \$0 | so | so |
| 38 | 2060 | None | \$0 | \$0 | \$0 | 38 | 2060 2061 | Asphalt Light Rehab | \$5,313,588 | \$1,779,961 |  |
| 39 | 2061 | None None | S0 50 | \$0 | \$0 | 39 40 | 2061 2062 | None | S0 s0 | \$0 | S0 |
| 40 | 2062 | ${ }_{\text {None }}$ | \$00 | 50 | 50 50 | ${ }_{41}^{40}$ | ${ }_{2063}^{2062}$ | None None | 50 | \$50 |  |
| ${ }_{42}^{41}$ | 2063 2064 | None | \$0 | \$0 | \$0 | ${ }_{42}^{41}$ | ${ }_{2064}^{2063}$ | None | S0 | so | S0 |
| ${ }^{43}$ | 2065 | None | \$0 | \$0 |  | ${ }^{43}$ | 2065 | None | 50 | \$0 | so |
| 44 | 2066 | Concreet Light Rehab | \$3,789,198 | \$1,063,033 | \$206,559 | 44 | 2066 | None None | \$0 | \$0 s0 | (50 |
| Pick Last Used DA treatment type to calculate Remaining Service Life ) |  | None | 50 | 50 |  | ${ }_{\text {Pick Last Used D }}$ | 2067 | None | \$0 | so |  |
|  |  | Concrete Light Rehab | \$3,59, 738 | \$980,467 | \$183,394 | Pick Last Used DA treatment type to calculateRemaining Service Life » |  | Asphalt Light Rehab | \$2,988,983 | \$814,090 | \$152,273 |
| Enter Year of Last Used DA Improvement " |  | 2066 | Remaining Service Life Cost^^ |  |  | Enter Year of Last Used DA Improvement \#) |  | 2060 | Remaining Service Life Cost^^ |  |  |
|  |  |  |  | $\begin{array}{\|c\|} \hline \text { Net Present Value (\$) @ } \\ 3 \% \end{array}$ | $\begin{gathered} \hline \text { Net Present Value (\$) @ } \\ 7 \% \\ \hline \end{gathered}$ |  |  |  |  | $\begin{gathered} \hline \text { Net Present Value (\$) @ } \\ 3 \% \\ \hline \end{gathered}$ | $\begin{array}{\|c} \hline \text { Net Present Value (\$) @ } \\ 7 \% \\ \hline \end{array}$ |
|  |  |  | NET PRESENT VALUE | \$35,953,272 | \$28,732,327 |  |  |  | NET PRESENT VALUE | \$32,910,227 | \$24,802,476 |
|  |  |  | AGENCY COST | \$45,050,080 |  |  |  |  | AGENCY COST | \$43,889,729 |  |



## Summary of LCCA Results

## US70 MP 293 - MP 298

|  | Concrete Reconstruction | Asphalt Reconstruction | Asphalt Medium Rehab Focus | Asphalt Light Rehab Focus |
| :--- | ---: | ---: | ---: | ---: |
| Net Present Value $-3 \%$ | $\$ 35,953,272$ | $\$ 32,910,227$ | $\$ 28,425,286$ | $\$ 32,545,584$ |
| Net Present Value $-7 \%$ | $\$ 28,732,327$ | $\$ 24,802,476$ | $\$ 15,943,097$ | $\$ 21,609,233$ |
| Agency Cost | $\$ 45,050,080$ | $\$ 43,889,729$ | $\$ 47,603,202$ | $\$ 47,542,821$ |

Cost Ratio at 3\% Discount Rate
1.26 Ratio of Concrete Reconstruction to Lowest Cost Rehab
1.16 Ratio of Asphalt Reconstruction to Lowest Cost Rehab

Cost Ratio at 7\% Discount Rate
1.80 Ratio of Concrete Reconstruction to Lowest Cost Rehab
1.56 Ratio of Asphalt Reconstruction to Lowest Cost Rehab

Note: A cost ratio < 1.15 means the Net Present Value (NPV) of reconstruction is within 15\% of the NPV of the lowest cost rehab so reconstruction should likely be the initial improvement solution. A cost ratio >1.15 means the NPV of reconstruction is more than $15 \%$ of the NPV of the lowest cost rehab so rehab should likely be the initial improvement solution.


Appendix F: Crash Modification Factors and Factored Unit Construction Costs

| SOLUTION | 2016 CONSTRUCTION UNIT COST | INFLATION FACTOR 2016-2022 | 2022 CONSTRUCTION UNIT COST | UNIT | FACTOR^ | 2016 FACTORED CONSTRUCTION UNIT COST | 2022 <br> FACTORED CONSTRUCTION UNIT COST | DESCRIPTION | 2022 CMF FOR CORRIDOR PROFILE STUDIES | CMF NOTES |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| REHABILITATION |  |  |  |  |  |  |  |  |  |  |
| Rehabilitate Pavement (AC) | \$276,500 | 1.74 | \$481,110 | Mile | 2.20 | \$610,000 | \$1,060,000 | Mill and replace 1"-3" AC pavement; accounts for ${ }^{\prime \prime}$ width; for one direction of travel on two-lane roadway; includes pavement, striping, delineators, RPMs, rumble strips | 0.68 | Updated to include 2 additional values (in addition to 3 previous values) from CMF Clearinghouse and revised combination of rehabilitate pavement (0.88), striping, delineators, RPMs ( 0.77 for combination), and rumble strips ( 0.89 ) $=0.68$ |
| Rehabilitate Bridge | \$65 | 1.74 | \$113 | SF | 2.20 | \$140 | \$250 | Based on deck area; bridge only - no other costs included | 0.95 | Assumed - should have a minor effect on crashes at the bridge |
|  |  |  |  |  |  |  |  |  |  |  |
| GEOMETRIC IMPROVEMENT |  |  |  |  |  |  |  |  |  |  |
| Re-profile Roadway | \$974,500 | 1.74 | \$1,695,630 | Mile | 2.20 | \$2,140,000 | \$3,730,000 | Includes excavation of approximately 3 ", pavement replacement (AC), striping, delineators, RPMs, rumble strips, for one direction of travel on two-lane roadway (38' width) | 0.70 | Assumed - this is similar to rehab pavement. This solution is intended to address vertical clearance at bridge, not profile issue; factor the cost as a ratio of needed depth to $3^{\prime \prime}$. |
| Realign Roadway | \$2,960,000 | 1.74 | \$5,150,400 | Mile | 2.20 | \$6,510,000 | \$11,330,000 | All costs per direction except bridges; applicable to areas with small or moderate fills and cuts, minimal retaining walls | 0.50 | Based on Caltrans and NCDOT |
| Improve Skid Resistance | \$675,000 | 1.74 | \$1,174,500 | Mile | 2.20 | \$1,490,000 | \$2,580,000 | Average cost of pavement replacement and variable depth paving to increase super-elevation; for one direction of travel on two-lane roadway; includes pavement, striping, delineators, RPMs, rumble strips | 0.65 | Updated to include 6 additional values (in addition to 6 previous values) from CMF Clearinghouse (0.71) and calculated composite CMF value using that 0.71 value, the HSM value (0.87) for skid resistance; striping, delineators, RPMs ( 0.77 for combination), and rumble strips ( 0.89 ) $=0.65$ |


| SOLUTION | 2016 CONSTRUCTION UNIT COST | INFLATION FACTOR 2016-2022 | 2022 CONSTRUCTION UNIT COST | UNIT | FACTOR^ | 2016 FACTORED CONSTRUCTION UNIT COST | 2022 <br> FACTORED CONSTRUCTION UNIT COST | DESCRIPTION | 2022 CMF FOR CORRIDOR PROFILE STUDIES | CMF NOTES |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| INFRASTRUCTURE IMPROVEMENT |  |  |  |  |  |  |  |  |  |  |
| Reconstruct to Urban Section | \$1,000,000 | 1.74 | \$1,740,000 | Mile | 2.20 | \$2,200,000 | \$3,828,000 | Includes widening by 16 ' total (AC = $12^{\prime}+2^{\prime}+2^{\prime}$ ) to provide median, curb \& gutter along both side of roadway, single curb for median, striping (doesn't include widening for additional travel lane). | 0.88 | From HSM |
| Construct Auxiliary Lanes (AC) | \$914,000 | 1.74 | \$1,590,360 | Mile | 2.20 | \$2,011,000 | \$3,499,000 | For addition of aux lane (AC) in one direction of travel; includes all costs except bridges; for generally at-grade facility with minimal walls and no major drainage improvements | 0.78 | Average of 4 values from clearinghouse |
| Construct Climbing Lane (High) | \$3,000,000 | 1.74 | \$5,220,000 | Mile | 2.20 | \$6,600,000 | \$11,484,000 | In one direction; all costs except bridges; applicable to areas with large fills and cuts, retaining walls, rock blasting, steep slopes on both sides of road | 0.75 | From HSM |
| Construct Climbing Lane (Medium) | \$2,250,000 | 1.74 | \$3,915,000 | Mile | 2.20 | \$4,950,000 | \$8,613,000 | In one direction; all costs except bridges; applicable to areas with medium or large fills and cuts, retaining walls, rock blasting, steep slopes on one side of road | 0.75 | From HSM |
| Construct Climbing Lane (Low) | \$1,500,000 | 1.74 | \$2,610,000 | Mile | 2.20 | \$3,300,000 | \$5,742,000 | In one direction; all costs except bridges; | 0.75 | From HSM |


| SOLUTION | 2016 CONSTRUCTION UNIT COST | INFLATION FACTOR 2016-2022 | 2022 CONSTRUCTION UNIT COST | UNIT | FACTOR^ | 2016 FACTORED CONSTRUCTION UNIT COST | 2022 <br> FACTORED CONSTRUCTION UNIT COST | DESCRIPTION | 2022 CMF FOR CORRIDOR PROFILE STUDIES | CMF NOTES |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  | applicable to areas with small or moderate fills and cuts, minimal retaining walls |  |  |
| Construct Reversible Lane (Low) | \$2,400,000 | 1.74 | \$4,176,000 | Lane-Mile | 2.20 | \$5,280,000 | \$9,190,000 | All costs except bridges; applicable to areas with small or moderate fills and cuts, minimal retaining walls | 0.73 for uphill and <br> 0.88 for downhill | Based on proposed conditions on I-17 with 2 reversible lanes and a concrete barrier |
| Construct Reversible Lane (High) | \$4,800,000 | 1.74 | \$8,352,000 | Lane-Mile | 2.20 | \$10,560,000 | \$18,370,000 | All costs except bridges; applicable to areas with large fills and cuts, retaining walls, rock blasting, mountainous terrain | 0.73 for uphill and <br> 0.88 for downhill | Based on proposed conditions on I-17 with 2 reversible lanes and a concrete barrier |
| Construct Passing Lane | \$1,500,000 | 1.74 | \$2,610,000 | Mile | 2.20 | \$3,300,000 | \$5,742,000 | In one direction; all costs except bridges; applicable to areas with small or moderate fills and cuts, minimal retaining walls | 0.63 | Average of 3 values from clearinghouse |
| Construct Entry/Exit Ramp | \$730,000 | 1.74 | \$1,270,200 | Each | 2.20 | \$1,610,000 | \$2,790,000 | Cost per ramp; includes pavement, striping, signing, RPMs, lighting, typical earthwork \& drainage; does not include any major structures or improvements on crossroad | 1.09 | Average of 16 values on clearinghouse; for adding a ramp not reconstructing. CMF applied to crashes 0.25 miles upstream/downstream from the gore. |
| Relocate Entry/Exit Ramp | \$765,000 | 1.74 | \$1,331,100 | Each | 2.20 | \$1,680,000 | \$2,930,000 | Cost per ramp; includes pavement, striping, signing, RPMs, lighting, typical earthwork, drainage and demolition of | 1.00 | Assumed to not add any crashes since the ramp is simply moving and not being added. CMF applied to crashes 0.25 miles upstream/downstream from the gore. |


| SOLUTION | 2016 CONSTRUCTION UNIT COST | INFLATION <br> FACTOR <br> 2016-2022 | 2022 CONSTRUCTION UNIT COST | UNIT | FACTOR^ | 2016 FACTORED CONSTRUCTION UNIT COST | 2022 <br> FACTORED CONSTRUCTION UNIT COST | DESCRIPTION | 2022 CMF FOR CORRIDOR PROFILE STUDIES | CMF NOTES |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  | existing ramp; does not include any major structures or improvements on crossroad |  |  |
| Construct Turn Lanes | \$42,500 | 1.74 | \$73,950 | Each | 2.20 | \$93,500 | \$163,000 | Includes 14' roadway widening (AC) for one additional turn lane (250' long) on one leg of an intersection; includes AC pavement, curb \& gutter, sidewalk, ramps, striping, and minor signal modifications | 0.81 | Average of 7 values from HSM; CMF applied to intersection-related crashes; this solution also applies when installing a deceleration lane |
| Modify Entry/Exit Ramp | \$445,000 | 1.74 | \$774,300 | Each | 2.20 | \$979,000 | \$1,703,000 | Cost per ramp; includes pavement, striping, signing, RPMs, lighting, minor earthwork, \& drainage; For converting existing ramp to parallel-type configuration | 0.21 | Average of 4 values from clearinghouse (for exit ramps) and equation from HSM (for entrance ramp). CMF applied to crashes within $1 / 8$ mile upstream/downstream from the gore. |
| Widen \& Modify Entry/Exit Ramp | \$619,000 | 1.74 | \$1,077,060 | Each | 2.20 | \$1,361,800 | \$2,370,000 | Cost per ramp; includes pavement, striping, signing, RPMs, lighting, minor earthwork, \& drainage; For converting 1 -lane ramp to 2-lane ramp and converting to parallel-type ramp | 0.21 | Will be same as "Modify Ramp" |
| Replace Pavement (AC) (with overexcavation) | \$1,446,500 | 1.74 | \$2,516,910 | Mile | 2.20 | \$3,180,000 | \$5,540,000 | Accounts for 38' width; for one direction of travel on two-lane roadway; includes pavement, overexcavation, striping, delineators, RPMs, rumble strips | 0.70 | Same as rehab |


| SOLUTION | 2016 CONSTRUCTION UNIT COST | INFLATION FACTOR 2016-2022 | 2022 CONSTRUCTION UNIT COST | UNIT | FACTOR^ | 2016 <br> FACTORED CONSTRUCTION UNIT COST | 2022 <br> FACTORED CONSTRUCTION UNIT COST | DESCRIPTION | 2022 CMF FOR CORRIDOR PROFILE STUDIES | CMF NOTES |
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| Replace Pavement (PCCP) (with overexcavation) | \$1,736,500 | 1.74 | \$3,021,510 | Mile | 2.20 | \$3,820,000 | \$6,650,000 | Accounts for 38' width; for one direction of travel on two-lane roadway; includes pavement, overexcavation, striping, delineators, RPMs, rumble strips | 0.70 | Same as rehab |
| Replace Bridge (Short) | \$125 | 1.74 | \$218 | SF | 2.20 | \$280 | \$480 | Based on deck area; bridge only - no other costs included; cost developed generally applies to bridges crossing small washes | 0.95 | Assumed - should have a minor effect on crashes at the bridge |
| Replace Bridge (Medium) | \$160 | 1.74 | \$278 | SF | 2.20 | \$350 | \$610 | Based on deck area; bridge only - no other costs included; cost developed generally applies to bridges crossing over the mainline freeway, crossroads, or large washes | 0.95 | Assumed - should have a minor effect on crashes at the bridge |
| Replace Bridge (Long) | \$180 | 1.74 | \$313 | SF | 2.20 | \$400 | \$690 | Based on deck area; bridge only - no other costs included; cost developed generally applies to bridges crossing large rivers or canyons | 0.95 | Assumed - should have a minor effect on crashes at the bridge |
| Widen Bridge | \$175 | 1.74 | \$305 | SF | 2.20 | \$390 | \$670 | Based on deck area; bridge only - no other costs included | 0.90 | Assumed - should have a minor effect on crashes at the bridge |
| Install Pedestrian Bridge | \$135 | 1.74 | \$235 | SF | 2.20 | \$300 | \$520 | Includes cost to construct bridge based on linear feet of the bridge. This cost includes and assumes ramps and sidewalks | 0.1 (pedestrian only) | Assumed direct access on both sides of structure |


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|  |  |  |  |  |  |  |  | leading to the structure. |  |  |
| Implement Automated Bridge Deicing | \$115 | 1.74 | \$200 | SF | 2.20 | \$250 | \$440 | Includes cost to replace bridge deck and install system | 0.72 (snow/ice) | Average of 3 values on clearinghouse for snow/ice |
| Install Wildlife Crossing Under Roadway | \$650,000 | 1.74 | \$1,131,000 | Each | 2.20 | \$1,430,000 | \$2,488,000 | Includes cost of structure for wildlife crossing under roadway and 1 mile of fencing in each direction that is centered on the wildlife crossing | $\begin{gathered} 0.25 \\ \text { (wildlife) } \end{gathered}$ | Assumed; CMF applies to wildliferelated crashes within 0.5 miles both upstream and downstream of the wildlife crossing in both directions |
| Install Wildlife Crossing Over Roadway | \$1,140,000 | 1.74 | \$1,983,600 | Each | 2.20 | \$2,508,000 | \$4,364,000 | Includes cost of structure for wildlife crossing over roadway and 1 mile of fencing in each direction that is centered on the wildlife crossing | $\begin{gathered} 0.25 \\ \text { (wildlife) } \end{gathered}$ | Assumed; CMF applies to wildliferelated crashes within 0.5 miles both upstream and downstream of the wildlife crossing in both directions |
| Construct Drainage Structure Minor | \$280,000 | 1.74 | \$487,200 | Each | 2.20 | \$616,000 | \$1,072,000 | Includes 3-36" pipes and roadway reconstruction (approx. 1,000 ft) to install pipes | 0.70 | Same as rehab; CMF applied to crashes 1/8 mile upstream/downstream of the structure |
| Construct Drainage Structure Intermediate | \$540,000 | 1.74 | \$939,600 | Each | 2.20 | \$1,188,000 | \$2,067,000 | Includes 5 barrel 8'x6' RCBC and roadway reconstruction (approx. 1,000 ft) to install RCBC | 0.70 | Same as rehab; CMF applied to crashes $1 / 8$ mile upstream/downstream of the structure |
| Construct Drainage Structure Major | \$8,000 | 1.74 | \$13,920 | LF | 2.20 | \$17,600 | \$30,600 | Includes bridge that is 40 wide and reconstruction of approx. 500' on each approach | 0.70 | Same as rehab; CMF applied to crashes 1/8 mile upstream/downstream of the structure |


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| Install Acceleration Lane | \$127,500 | 1.74 | \$221,850 | Each | 2.20 | \$280,500 | \$488,000 | For addition of an acceleration lane (AC) on one leg of an intersection that is 1,000' long plus a taper; includes all costs except bridges; for generally at-grade facility with minimal walls and no major drainage improvements | 0.85 | Average of 6 values from the FHWA Desktop Reference for Crash Reduction Factors |
| Install Curb and Gutter | \$211,200 | 1.74 | \$367,488 | Mile | 2.20 | \$465,000 | \$808,000 | In both directions; curb and gutter | 0.89 | From CMF Clearinghouse |
| Install Sidewalks, Curb, and Gutter | \$475,200 | 1.74 | \$826,848 | Mile | 2.20 | \$1,045,000 | \$1,819,000 | In both directions; $5^{\prime}$ sidewalks, curb, and gutter | 0.89 installing sidewalk 0.24 (pedestrian crashes only) | From CMF Clearinghouse <br> Avg of 6 values from FHWA Desktop Reference |
| Install Sidewalks | \$264,000 | 1.74 | \$459,360 | Mile | 2.20 | \$581,000 | \$1,011,000 | In both directions; 5' sidewalks | 0.24 (pedestrian crashes only) | Avg of 6 values from FHWA Desktop Reference |
| OPERATIONAL IMPROVEMENT |  |  |  |  |  |  |  |  |  |  |
| Implement Variable Speed Limits (Wireless, Overhead) | \$718,900 | 1.25 | \$898,625 | Mile | 2.20 | \$1,580,000 | \$1,980,000 | In one direction; includes 1 sign assembly per mile (foundation and structure), wireless communication, detectors | 0.91 (all crashes) 0.69 (weatherrelated) | Originally only 1 value from CMF Clearinghouse. Updated to include 1 value for all crashes and 2 additional values for weather-related crashes |
| Implement Variable Speed Limits (Wireless, Ground-mount) | \$169,700 | 1.25 | \$212,125 | Mile | 2.20 | \$373,300 | \$467,000 | In one direction; includes 2 signs per mile (foundations and posts), wireless communication, detectors | 0.91 (all crashes) 0.69 (weatherrelated) | Originally only 1 value from CMF Clearinghouse. Updated to include 1 value for all crashes and 2 additional values for weather-related crashes |


| SOLUTION | $\qquad$ | INFLATION FACTOR 2016-2022 | 2022 CONSTRUCTION UNIT COST | UNIT | FACTOR^ | 2016 FACTORED CONSTRUCTION UNIT COST | 2022 <br> FACTORED CONSTRUCTION UNIT COST | DESCRIPTION | 2022 CMF FOR CORRIDOR PROFILE STUDIES | CMF NOTES |
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| Implement Variable Speed Limits (Wireless, Solar, Overhead) | \$502,300 | 1.25 | \$627,875 | Mile | 2.20 | \$1,110,000 | \$1,380,000 | In one direction; includes 1 sign assembly per mile (foundation and structure), wireless communication, detectors, solar power | 0.91 (all crashes) 0.69 (weatherrelated) | Originally only 1 value from CMF Clearinghouse. Updated to include 1 value for all crashes and 2 additional values for weather-related crashes |
| Implement Variable Speed Limits (Wireless, Solar, Ground-mount) | \$88,400 | 1.25 | \$110,500 | Mile | 2.20 | \$194,500 | \$243,000 | In one direction; includes 2 signs per mile (foundations and posts), wireless communication, detectors, solar power | 0.91 (all crashes) 0.69 (weatherrelated) | Originally only 1 value from CMF Clearinghouse. Updated to include 1 value for all crashes and 2 additional values for weather-related crashes |
| Implement Ramp Metering (Low) | \$25,000 | 1.25 | \$31,250 | Each | 2.20 | \$55,000 | \$68,800 | For each entry ramp location; urban area with existing ITS backbone infrastructure; includes signals, poles, timer, pull boxes, etc. | 0.64 | From 1 value from clearinghouse; CMF applied to crashes 0.25 miles after gore |
| Implement Ramp Metering (High) | \$150,000 | 1.25 | \$187,500 | Mile | 2.20 | \$330,000 | \$413,000 | Area without existing ITS backbone infrastructure; in addition to ramp meters, also includes conduit, fiber optic lines, and power | 0.64 | From 1 value from clearinghouse |
| Implement Signal Coordination | \$140,000 | 1.25 | \$175,000 | Mile | 2.20 | \$308,000 | \$385,000 | Includes conduit, conductors, and controllers for 4 intersections that span a total of approximately 2 miles | 0.90 | Assumed |
| Implement Left-Turn Phasing | \$7,500 | 1.25 | \$9,375 | Each | 2.20 | \$16,500 | \$20,600 | Includes four new signal heads (two in each direction) and associated conductors for one intersection | $\begin{gathered} 0.88 \text { (protected) } \\ 0.98 \\ \text { (permitted/protected } \\ \text { or } \\ \text { protected/permitted) } \end{gathered}$ | From HSM; CMF $=0.94$ for each protected approach and 0.99 for each permitted/protected or protected/permitted approach. CMFs of different approaches should be |


| SOLUTION | 2016 CONSTRUCTION UNIT COST | INFLATION FACTOR 2016-2022 | 2022 CONSTRUCTION UNIT COST | UNIT | FACTOR^ | 2016 <br> FACTORED CONSTRUCTION UNIT COST | 2022 <br> FACTORED CONSTRUCTION UNIT COST | DESCRIPTION | 2022 CMF FOR CORRIDOR PROFILE STUDIES | CMF NOTES |
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|  |  |  |  |  |  |  |  |  |  | multiplied together. CMF applied to crashes within intersection |
| Install Adaptive Signal Control and Signal Coordination | \$363,500 | 1.25 | \$454,375 | mile | 2.20 | \$800,000 | \$1,000,000 | Controller upgrades, advanced detection, software configuration, cameras; includes conduit, conductors, and controllers for 4 intersections that span a total of approximately 2 miles for coordination | 0.78 (adaptive control) 0.90 (signal coordination) | Updated to include 15 additional values (in addition to 2 previous values) for adaptive control from CMF Clearinghouse |
| ROADSIDE DESIGN |  |  |  |  |  |  |  |  |  |  |
| Install Guardrail | \$130,000 | 1.74 | \$226,200 | Mile | 2.20 | \$286,000 | \$498,000 | One side of road | 0.62 (ROR) | 0.62 is average of 2 values from clearinghouse |
| Install Cable Barrier | \$80,000 | 1.74 | \$139,200 | Mile | 2.20 | \$176,000 | \$306,000 | In median | 0.65 | Updated to include 5 additional values (in addition to 5 previous values) from CMF Clearinghouse |
| Widen Shoulder (AC) | \$256,000 | 1.74 | \$445,440 | Mile | 2.20 | \$563,000 | \$980,000 | Assumes 10' of existing shoulder (combined left and right), includes widening shoulder by a total of 4'; new pavement for 4' width and mill and replace existing 10' width; includes pavement, minor earthwork, striping edge lines, RPMs, high-visibility delineators, safety edge, and rumble strips | $\begin{gathered} 0.68\left(1-4^{\prime}\right) \\ 0.64 \text { (>= 4') } \end{gathered}$ | 0.86 is average of 5 values from clearing house for widening shoulder 1-4'. 0.76 is calculated from HSM for widening shoulder >=4'. (Cost needs to be updated if dimension of existing and widened shoulder differ from Description.) |


| SOLUTION | 2016 CONSTRUCTION UNIT COST | INFLATION <br> FACTOR <br> 2016-2022 | 2022 CONSTRUCTION UNIT COST | UNIT | FACTOR^ | 2016 <br> FACTORED CONSTRUCTION UNIT COST | 2022 <br> FACTORED CONSTRUCTION UNIT COST | DESCRIPTION | 2022 CMF FOR CORRIDOR PROFILE STUDIES | CMF NOTES |
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| Rehabilitate Shoulder (AC) | \$113,000 | 1.74 | \$196,620 | Mile | 2.20 | \$249,000 | \$433,000 | One direction of travel (14' total shoulder width-4' left and 10 ' right); includes paving (mill and replace), striping, high-visibility delineators, RPMs, safety edge, and rumble strips for both shoulders | 0.72 | 0.98 is average of 34 values on clearinghouse for shoulder rehab/replace; include striping, delineators, RPMs ( 0.77 combined CMF), and rumble strips (0.89). (Cost needs to be updated if dimension of existing shoulder differs from Description.) |
| Replace Shoulder (AC) | \$364,000 | 1.74 | \$633,360 | Mile | 2.20 | \$801,000 | \$1,393,000 | One direction of travel (14' total shoulder width-4' left and 10 right); includes paving (full reconstruction), striping, high-visibility delineators, RPMs, safety edge, and rumble strips for both shoulders | 0.72 | 0.98 is average of 34 values on clearinghouse for shoulder rehab/replace; include striping, delineators, RPMs ( 0.77 combined CMF), and rumble strips (0.89). (Cost needs to be updated if dimension of existing shoulder differs from Description.) |
| Install Rumble Strip | \$5,500 | 1.74 | \$9,570 | Mile | 2.20 | \$12,000 | \$21,000 | Both edges - one direction of travel; includes only rumble strip; no shoulder rehab or paving or striping | 0.89 | Average of 75 values on clearinghouse and consistent with HSM |
| Install Centerline Rumble Strip | \$2,800 | 1.74 | \$4,872 | Mile | 2.20 | \$6,000 | \$11,000 | Includes rumble strip only; no pavement rehab or striping | 0.85 | From HSM |
| Install Wildlife Fencing | \$340,000 | 1.74 | \$591,600 | Mile | 2.20 | \$748,000 | \$1,302,000 | Fencing only plus jump outs for 1 mile (both directions) | $\begin{gathered} 0.50 \\ \text { (wildlife) } \end{gathered}$ | Assumed |
| Remove Tree/Vegetation | \$200,000 | 1.74 | \$348,000 | Mile | 2.20 | \$440,000 | \$766,000 | Intended for removing trees that shade the roadway to allow sunlight to help melt snow and ice (see Increase Clear Zone CMF for general tree/vegetation removal in clear zone) | 0.72 (snow/ice) | Average of 3 values on clearinghouse for snow/ice |


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| Increase Clear Zone | \$59,000 | 1.74 | \$102,660 | Mile | 2.20 | \$130,000 | \$226,000 | In one direction; includes widening the clear zone by 10 ' to a depth of $3^{\prime}$ | 0.71 | Median of 14 values from FHWA Desktop Reference for Crash Reduction Values |
| Install Access Barrier Fence | \$15 | 1.74 | \$26 | LF | 2.20 | \$33 | \$60 | $8^{\prime}$ fencing along residential section of roadway | $\begin{gathered} 0.10 \\ \text { (pedestrian only) } \end{gathered}$ | Equal to pedestrian overpass |
| Install Rock-Fall Mitigation - Wire Mesh | \$1,320,000 | 1.74 | \$2,296,800 | Mile | 2.20 | \$2,904,000 | \$5,053,000 | Includes wire mesh and rock stabilization (one direction) | 0.75 (debris) | Assumed |
| Install Rock-Fall Mitigation Containment Fence \& Barrier | \$2,112,000 | 1.74 | \$3,674,880 | Mile | 2.20 | \$4,646,000 | \$8,085,000 | Includes containment fencing, concrete barrier, and rock stabilization (one direction) | 0.75 (debris) | Assumed |
| Install Raised Concrete Barrier in Median | \$650,000 | 1.74 | \$1,131,000 | Mile | 2.20 | \$1,430,000 | \$2,488,000 | Includes concrete barrier with associated striping and reflective markings; excludes lighting in barrier (one direction) | 0.90 (Cross-median and head on crashes eliminated completely) | All cross median and head-on fatal or incapacitating injury crashes are eliminated completely; all remaining crashes have 0.90 applied |
| Formalize Pullout (Small) | \$7,500 | 1.74 | \$13,050 | Each | 2.20 | \$17,000 | \$29,000 | Includes paving and signage (signs, posts, and foundations) approximately 4,200 sf | 0.97 | Assumed - similar to Install Other General Warning Signs; CMF applied to crashes within 0.25 miles after sign |
| Formalize Pullout (Medium) | \$27,500 | 1.74 | \$47,850 | Each | 2.20 | \$61,000 | \$105,000 | Includes paving and signage (signs, posts, and foundations) approximately 22,500 sf | 0.97 | Assumed - similar to Install Other General Warning Signs; CMF applied to crashes within 0.25 miles after sign |
| Formalize Pullout (Large) | \$80,500 | 1.74 | \$140,070 | Each | 2.20 | \$177,100 | \$308,000 | Includes paving and signage (signs, posts, and foundations) approximately 70,000 sf | 0.97 | Assumed - similar to Install Other General Warning Signs; CMF applied to crashes within 0.25 miles after sign |
| INTERSECTION IMPROVEMENTS |  |  |  |  |  |  |  |  |  |  |


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| Construct Traffic Signal | \$150,000 | 1.74 | \$261,000 | Each | 2.20 | \$330,000 | \$574,000 | 4-legged intersection; includes poles, foundations, conduit, controller, heads, luminaires, mast arms, etc. | 0.95 | From HSM; CMF applied to crashes within intersection only |
| Improve Signal Visibility | \$35,000 | 1.74 | \$60,900 | Each | 2.20 | \$77,000 | \$134,000 | 4-legged intersection; signal head size upgrade, installation of new back-plates, and installation of additional signal heads on new poles. | 0.85 | Average of 7 values from clearinghouse; CMF applied to crashes within intersection only |
| Install Raised Median | \$360,000 | 1.74 | \$626,400 | Mile | 2.20 | \$792,000 | \$1,378,000 | Includes removal of 14' wide pavement and construction of curb \& gutter; does not include cost to widen roadway to accommodate the median; if the roadway needs to be widened, include cost from New General Purpose Lane | 0.83 | Average from HSM |
| Install Transverse Rumble Strip/Pavement Markings | \$3,000 | 1.74 | \$5,220 | Each | 2.20 | \$7,000 | \$11,000 | Includes pedestrian markings and rumble strips only across a $30^{\prime}$ wide travelway; no pavement rehab or other striping | 0.95 | Average of 17 values from clearinghouse; CMF applied to crashes within 0.5 miles after the rumble strips and markings |
| Construct Single-Lane Roundabout | \$1,500,000 | 1.74 | \$2,610,000 | Each | 2.20 | \$3,300,000 | \$5,742,000 | Removal of signal at 4legged intersection; realignment of each leg for approx. 800 feet including paving, curbs, sidewalk, striping, lighting, signing | 0.22 | From HSM; CMF applied to crashes within intersection only |


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| Construct Double-Lane Roundabout | \$1,800,000 | 1.74 | \$3,132,000 | Each | 2.20 | \$3,960,000 | \$6,890,000 | Removal of signal at 4-legged intersection; realignment of each leg for approx. 800 feet including paving, curbs, sidewalk, striping, lighting, signing | 0.40 | From HSM; CMF applied to crashes within intersection only |
| Install Indirect Left Turn Intersection | \$1,140,000 | 1.74 | \$1,983,600 | each | 2.20 | \$2,500,000 | \$4,364,000 | Raised concrete median improvements; intersection improvements; turn lanes | 0.76 | Updated to include 2 additional values (in addition to 1 previous value) from CMF Clearinghouse |
| Convert Standard Diamond Interchange to Diverging Diamond Interchange | \$2,272,700 | 1.74 | \$3,954,498 | each | 2.20 | \$5,000,000 | \$8,700,000 | Convert traditional diamond interchange into diverging diamond interchange; assumes re-use of existing bridges | 0.56 | Updated to include 2 additional values (in addition to 1 previous value) from CMF Clearinghouse |
| Left-in Only Center Raised Median Improvements | \$84,100 | 1.74 | \$146,334 | each | 2.20 | \$185,000 | \$322,000 | Left-in only center raised median improvements | 0.87 | CMF Clearinghouse |
| ROADWAY DELINEATION |  |  |  |  |  |  |  |  |  |  |
| Install High-Visibility Edge Line Striping | \$10,800 | 1.25 | \$13,500 | Mile | 2.20 | \$23,800 | \$29,700 | 2 edge lines and lane line - one direction of travel |  | Average of 3 values from clearinghouse. Assumes package of striping, delineators, and RPMs. (If implemented separately, CMF will be higher.) |
| Install High-Visibility Delineators | \$6,500 | 1.25 | \$8,125 | Mile | 2.20 | \$14,300 | \$17,900 | Both edges - one direction of travel | 0.77 | Average of 3 values from clearinghouse. Assumes package of striping, delineators, and RPMs. (If implemented separately, CMF will be higher.) |
| Install Raised Pavement Markers | \$2,000 | 1.25 | \$2,500 | Mile | 2.20 | \$4,400 | \$5,500 | Both edges - one direction of travel |  | Average of 3 values from clearinghouse. Assumes package of striping, delineators, and RPMs. (If implemented separately, CMF will be higher.) |


| SOLUTION | 2016 CONSTRUCTION UNIT COST | INFLATION FACTOR 2016-2022 | 2022 <br> CONSTRUCTION UNIT COST | UNIT | FACTOR^ | 2016 FACTORED CONSTRUCTION UNIT COST | 2022 <br> FACTORED CONSTRUCTION UNIT COST | DESCRIPTION | 2022 CMF FOR CORRIDOR PROFILE STUDIES | CMF NOTES |
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| Install In-Lane Route Markings | \$6,000 | 1.25 | \$7,500 | Each | 2.20 | \$13,200 | \$16,500 | Installation of a series of three in-lane route markings in one lane | 0.95 | Assumed; CMF applied to crashes within 1.0 mile before the gore |
| IMPROVED VISIBILITY |  |  |  |  |  |  |  |  |  |  |
| Cut Side Slopes | \$80 | 1.74 | \$139 | LF | 2.20 | \$200 | \$300 | For small grading to correct sight distance issues; not major grading | 0.85 | Intent of this solution is to improve sight distance. Most CMF's are associated with vehicles traveling on slope. Recommended CMF is based on FDOT and NCDOT but is more conservative. |
| Install Lighting (connect to existing power) | \$270,000 | 1.74 | \$469,800 | Mile | 2.20 | \$594,000 | \$1,034,000 | One side of road only; offset lighting, not high-mast; does not include power supply; includes poles, luminaire, pull boxes, conduit, conductor | 0.75 (night) | Average of 3 values on clearinghouse \& consistent with HSM |
| Install Lighting (solar powered LED) | \$10,000 | 1.74 | \$17,400 | Pole | 2.20 | \$22,000 | \$38,300 | Offset lighting, not high-mast; solar power LED; includes poles, luminaire, solar panel | 0.75 (night) | Average of 3 values on clearinghouse \& consistent with HSM |
| DRIVER INFORMATION/WARNING |  |  |  |  |  |  |  |  |  |  |
| Install Dynamic Message Sign (DMS) | \$250,000 | 1.25 | \$312,500 | Each | 2.20 | \$550,000 | \$688,000 | Includes sign, overhead structure, and foundations; wireless communication; does not include power supply | 1.00 | Not expected to reduce crashes |
| Install Dynamic Weather Warning Beacons | \$40,000 | 1.25 | \$50,000 | Each | 2.20 | \$88,000 | \$110,000 | Assumes solar operation and wireless communication or connection to existing power and communication; ground mounted; includes posts, foundations, solar panel, and dynamic sign | 0.80 (weatherrelated) | Average of 3 values from FHWA Desktop Reference for Crash Reduction Factors; CMF applies to crashes within 0.25 miles after a sign |


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| Install Dynamic Speed Feedback Signs | \$25,000 | 1.25 | \$31,250 | Each | 2.20 | \$55,000 | \$68,800 | Assumes solar operation and no communication; ground mounted; includes regulatory sign, posts, foundations, solar panel, and dynamic sign | 0.94 | Average of 2 clearinghouse values; CMF applies to crashes within 0.50 miles after a sign |
| Install Chevrons | \$18,400 | 1.25 | \$23,000 | Mile | 2.20 | \$40,500 | \$50,600 | On one side of road includes signs, posts, and foundations | 0.79 | Average of 11 clearinghouse values |
| Install Curve Warning Signs | \$2,500 | 1.25 | \$3,125 | Each | 2.20 | \$5,500 | \$6,900 | Includes 2 signs, posts, and foundations | 0.83 | Average of 4 clearinghouse values; CMF applies to crashes within 0.25 miles after a sign |
| Install Traffic Control Device Warning Signs (e.g., stop sign ahead, signal ahead, etc.) | \$2,500 | 1.25 | \$3,125 | Each | 2.20 | \$5,500 | \$6,900 | Includes 2 signs, posts, and foundations | 0.85 | FHWA Desktop Reference for Crash Reduction Factors; CMF applies to crashes within 0.25 miles after a sign |
| Install Other General Warning Signs (e.g., intersection ahead, wildlife in area, slow vehicles, etc.) | \$2,500 | 1.25 | \$3,125 | Each | 2.20 | \$5,500 | \$6,900 | Includes 2 signs, posts, and foundations | 0.97 | Assumed; CMF applies to crashes within 0.25 miles after a sign |
| Install Wildlife Warning System | \$162,000 | 1.25 | \$202,500 | Each | 2.20 | \$356,400 | \$446,000 | Includes wildlife detection system at a designated wildlife crossing, flashing warning signs (assumes solar power), advance signing, CCTV (solar and wireless), game fencing for approximately 0.25 miles in each direction - centered on the wildlife crossing, and regular fencing for 1.0 mile in each direction - centered on the wildlife crossing. | $\begin{gathered} 0.50 \\ \text { (wildlife) } \end{gathered}$ | Assumed; CMF applies to wildliferelated crashes within 0.5 miles both upstream and downstream of the wildlife crossing in both directions |


| SOLUTION | 2016 CONSTRUCTION UNIT COST | INFLATION FACTOR 2016-2022 | 2022 CONSTRUCTION UNIT COST | UNIT | FACTOR^ | 2016 FACTORED CONSTRUCTION UNIT COST | 2022 <br> FACTORED CONSTRUCTION UNIT COST | DESCRIPTION | 2022 CMF FOR CORRIDOR PROFILE STUDIES | CMF NOTES |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Install Warning Sign with Beacons | \$15,000 | 1.25 | \$18,750 | Each | 2.20 | \$33,000 | \$41,300 | In both directions; includes warning sign, post, and foundation, and flashing beacons (assumes solar power) at one location | 0.75 | FHWA Desktop Reference for Crash Reduction Factors for Installing Flashing Beacons as Advance Warning; CMF applies to crashes within 0.25 miles after a sign |
| Install Rectangular Rapid Flashing Beacon (RRFB) | \$15,000 | 1.25 | \$18,750 | Each | 2.20 | \$33,000 | \$41,300 | In both directions; includes warning sign, post, and foundation, and flashing beacons (assumes solar power) at one location | 0.53 | CMF Clearinghouse |
| Install Larger Stop Sign with Beacons | \$10,000 | 1.25 | \$12,500 | Each | 2.20 | \$22,000 | \$27,500 | In one direction; includes large stop sign, post, and foundation, and flashing beacons (assumes solar power) at one location | 0.85/0.81 | Use 0.85 for adding beacons to an existing sign; 0.81 for installing a larger sign with flashing beacons; CMF applies to intersection-related crashes |
| Install Advanced Warning Signal System | \$108,000 | 1.25 | \$135,000 | each | 2.20 | \$238,000 | \$297,000 | Overhead static sign with flashing beacons, detectors, and radar system. Signs for each mainline approach of the intersection (2) | 0.61 | FHWA Desktop Reference for CRF |
| DATA COLLECTION |  |  |  |  |  |  |  |  |  |  |
| Install Roadside Weather Information System (RWIS) | \$60,000 | 1.25 | \$75,000 | Each | 2.20 | \$132,000 | \$165,000 | Assumes wireless communication and solar power, or connection to existing power and communications | 1.00 | Not expected to reduce crashes |
| Install Closed Circuit Television (CCTV) Camera | \$25,000 | 1.25 | \$31,250 | Each | 2.20 | \$55,000 | \$68,800 | Assumes connection to existing ITS backbone or wireless communication; does not include fiber-optic backbone infrastructure; includes pole, camera, etc. | 1.00 | Not expected to reduce crashes |


| SOLUTION | 2016 CONSTRUCTION UNIT COST | INFLATION FACTOR 2016-2022 | 2022 CONSTRUCTION UNIT COST | UNIT | FACTOR^ | 2016 FACTORED CONSTRUCTION UNIT COST | 2022 <br> FACTORED CONSTRUCTION UNIT COST | DESCRIPTION | 2022 CMF FOR CORRIDOR PROFILE STUDIES | CMF NOTES |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Install Vehicle Detection Stations | \$15,000 | 1.25 | \$18,750 | Each | 2.20 | \$33,000 | \$41,300 | Assumes wireless communication and solar power, or connection to existing power and communications | 1.00 | Not expected to reduce crashes |
| Install Flood Sensors (Activation) | \$15,000 | 1.25 | \$18,750 | Each | 2.20 | \$33,000 | \$41,300 | Sensors with activation cabinet to alert through texting (agency) | 1.00 | Not expected to reduce crashes |
| Install Flood Sensors (Gates) | \$100,000 | 1.25 | \$125,000 | Each | 2.20 | \$220,000 | \$275,000 | Sensors with activation cabinet to alert through texting (agency) and beacons (public) plus gates | 1.00 | Not expected to reduce crashes |
| WIDEN CORRIDOR |  |  |  |  |  |  |  |  |  |  |
| Construct New General Purpose Lane (PCCP) | \$1,740,000 | 1.74 | \$3,027,600 | Mile | 2.20 | \$3,830,000 | \$6,660,000 | For addition of 1 GP lane (PCCP) in one direction; includes all costs except bridges; for generally at-grade facility with minimal walls and no major drainage improvements | 0.90 | North Carolina DOT uses 0.90 and Florida DOT uses 0.87 |
| Construct New General Purpose Lane (AC) | \$1,200,000 | 1.74 | \$2,088,000 | Mile | 2.20 | \$2,640,000 | \$4,590,000 | For addition of 1 GP lane ( AC ) in one direction; includes all costs except bridges; for generally at-grade facility with minimal walls and no major drainage improvements | 0.90 | North Carolina DOT uses 0.90 and Florida DOT uses 0.88 |
| Convert a 2-Lane undivided highway to a 5-Lane highway | \$1,576,000 | 1.74 | \$2,742,240 | Mile | 2.20 | \$3,467,200 | \$6,030,000 | For expanding a 2-lane undivided highway to a 5-lane highway (4 through lanes with TWLTL), includes standard shoulder widths but no curb, gutter, or sidewalks | 0.60 | Assumed to be slightly lower than converting from a 4 -lane to a 5 -lane highway |


| SOLUTION | 2016 CONSTRUCTION UNIT COST | INFLATION FACTOR 2016-2022 | 2022 CONSTRUCTION UNIT COST | UNIT | FACTOR^ | 2016 FACTORED CONSTRUCTION UNIT COST | 2022 <br> FACTORED CONSTRUCTION UNIT COST | DESCRIPTION | 2022 CMF FOR CORRIDOR PROFILE STUDIES | CMF NOTES |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Install Center Turn Lane | \$1,053,000 | 1.74 | \$1,832,220 | Mile | 2.20 | \$2,316,600 | \$4,030,000 | For adding a center turn lane (i.e., TWLTL); assumes symmetrical widening on both sides of the road; includes standard shoulder widths but no curb, gutter, or sidewalk | 0.75 | From FHWA Desktop Reference for Crash Reduction Factors, CMF Clearinghouse, and SR 87 CPS comparison |
| Construct 4-Lane Divided Highway (Using Existing 2-Lane Road for one direction) | \$3,000,000 | 1.74 | \$5,220,000 | Mile | 2.20 | \$6,600,000 | \$11,484,000 | In both directions; one direction uses existing 2 -lane road; other direction assumes addition of 2 new lanes (AC) with standard shoulders; includes all costs except bridges | 0.67 | Assumed |
| Construct 4-Lane Divided Highway (No Use of Existing Roads) | \$6,000,000 | 1.74 | \$10,440,000 | Mile | 2.20 | \$13,200,000 | \$22,968,000 | In both directions; assumes addition of 2 new lanes (AC) with standard shoulders in each direction; includes all costs except bridges | 0.67 | Assumed |
| Construct Bridge over At-Grade Railroad Crossing | \$10,000,000 | 1.74 | \$17,400,000 | Each | 2.20 | \$22,000,000 | \$38,280,000 | Assumes bridge width of 4 lanes (AC) with standard shoulders; includes abutments and bridge approaches; assumes vertical clearance of 23'4" + 6'8" superstructure | 0.72 (All train-related crashes eliminated) | Removes all train-related crashes at atgrade crossing; all other crashes CMF = 0.72 |
| Construct Underpass at At-Grade Railroad Crossing | \$15,000,000 | 1.74 | \$26,100,000 | Each | 2.20 | \$33,000,000 | \$57,420,000 | Assumes underpass width of 4 lanes (AC) with standard shoulders; includes railroad bridge with abutments and underpass approaches; assumes vertical clearance of 16'6" + 6'6" superstructure | 0.72 (All train-related crashes eliminated) | Removes all train-related crashes at atgrade crossing; all other crashes CMF = 0.72 |


| SOLUTION | 2016 CONSTRUCTION UNIT COST | INFLATION FACTOR 2016-2022 | 2022 CONSTRUCTION UNIT COST | UNIT | FACTOR^ | 2016 <br> FACTORED CONSTRUCTION UNIT COST | 2022 <br> FACTORED CONSTRUCTION UNIT COST | DESCRIPTION | 2022 CMF FOR CORRIDOR PROFILE STUDIES | CMF NOTES |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Construct High-Occupancy Vehicle (HOV) Lane | \$900,000 | 1.74 | \$1,566,000 | Mile | 2.20 | \$1,980,000 | \$3,445,000 | For addition of 1 HOV lane (AC) in one direction with associated signage and markings; includes all costs except bridges; for generally at-grade facility with minimal walls and no major drainage improvements | 0.95 | Similar to general purpose lane |
| ALTERNATE ROUTE |  |  |  |  |  |  |  |  |  |  |
| Construct Frontage Roads | \$2,400,000 | 1.74 | \$4,176,000 | Mile | 2.20 | \$5,280,000 | \$9,190,000 | For 2-lane AC frontage road; includes all costs except bridges; for generally at-grade facility with minimal walls | 0.90 | Assumed - similar to new general purpose lane |
| Construct 2-Lane Undivided Highway | \$3,000,000 | 1.74 | \$5,220,000 | Mile | 2.20 | \$6,600,000 | \$11,484,000 | In both directions; assumes addition of 2 new lanes (AC) with standard shoulders in each direction; includes all costs except bridges | 0.90 | Assuming new alignment for a bypass |

## Aロロт

## Appendix G: Performance Area Risk Factors

## Pavement Performance Area

- Elevation
- Mainline Daily Traffic Volume
- Mainline Daily Truck Volume


## Elevation <br> Variance above 4000' divided by 1000; (Elev- <br> 4000)/1000 <br> Score Condition <br> $0<4000^{\prime}$ <br> 0-5 4000'-9000 <br> $5>9000^{\prime}$

## Mainline Daily Traffic Volume

Exponential equation; score $\left.=5-\left(5^{*} e^{(A D T *}-.000039\right)\right)$
Score Condition
$0<6,000$
0-5 6,000-160,000
$5>160,000$

## Mainline Daily Truck Volume

Exponential equation; score $=5-\left(5^{*} e^{\left(A D T^{*}-0.00025\right)}\right)$
Score Condition
$0<900$
0-5 900-25,000
$5>25,000$

Bridge Performance Area

- Mainline Daily Traffic Volume
- Elevation
- Carries Mainline Traffic

Mainline Daily Traffic Volume
Exponential equation; score $=5-\left(5^{*} e^{\left(A D T^{*}-0.000039\right)}\right)$ Score Condition
$0<6,000$
0-5 6,000-160,000
$5>160,000$
Elevation
Variance above 4000' divided by 1000; (Elev-4000)/1000
Score Condition
$0<4000^{\prime}$
0-5 4000'- $9000^{\prime}$
$5>9000^{\prime}$
Carries Mainline Traffic
Score Condition
0 Does not carry mainline traffic
5 Carries mainline traffic
Detour Length
Divides detour length by 10 and multiplies by 2.5 Score Condition
$0 \quad 0$ miles
0-5 $0-20$ miles
$5>20$ miles
Scour Critical Ratin
Variance below 8
Score Condition
$0 \quad$ Rating > 8
0-5 Rating 8-3
5 Rating < 3
Vertical Clearance
Variance below 16' $\times 2.5$; (16 -Clearance) $\times 2.5$
Score Condition
$0>16$ '
0-5 16'-14'
$5<14$

## Mobility Performance Area

- Mainline VMT
- Buffer Index (PTI-TTI)
- Detour Length
- Outside Shoulder Width


## Mainline VMT

Exponential equation; score $=5-\left(5^{*} e\left(A D T^{\star}-0.0000139\right)\right)$

| Score | Condition |
| :---: | :--- |
| 0 | $<16,000$ |
| $0-5$ | $16,000-400,000$ |
| 5 | $>400,000$ |

## Buffer Index

Buffer Index x 10
Score Condition
$0 \quad$ Buffer Index $=0.00$
0-5 Buffer Index 0.00-0.50
5 Buffer Index $>0.50$

| Detour Length |  |
| :---: | :--- |
| Score | Condition |
| 0 | Detour $<10$ miles |
| 5 | Detour $>10$ miles |

Outside Shoulder Width
Variance below 10', if only 1 lane in each direction
Score Condition
$10^{\prime}$ or above or >1 lane in each direction
0-5 10'-5' and 1 lane in each direction $5 \quad 5$ ' or less and 1 lane in each direction

## Safety Performance Area

- Mainline Daily Traffic Volume
- Interrupted Flow
- Elevation
- Outside Shoulder Width
- Vertical Grade


## Mainline Daily Traffic Volume

| Exponential equation; score $=5-\left(5^{*} \mathrm{e}^{\left(A D T^{*}-0.000039\right)}\right)$ |  |
| :---: | :---: |
| Score | Condition |
| 0 | <6,000 |
| 0-5 | 6,000-160,000 |
| 5 | >160,000 |
| Interrupted Flow |  |
| Score | Condition |
| 0 | Not interrupted flow |
| 5 | Interrupted Flow |
| Elevation |  |
| Variance above 4000' divided by 1000; (Elev-4000)/1000 |  |
| Score | Condition |
| 0 | < 4000' |
| 0-5 | 4000'-9000' |
| 5 | > 9000' |

Outside Shoulder Width
Variance below 10'
Score Condition
0 10' or above
0-5 10' - 5'
5 5' or less
Grade
Variance above 3\% x 1.5
Score Condition
$0<3 \%$
0-5 3\%-6.33\%
$5>6.33 \%$

Freight Performance Area

- Mainline Daily Truck Volume
- Detour Length
- Truck Buffer Index (TPTI-TTTI)
- Outside Shoulder Width

Mainline Daily Truck Volume
Exponential equation; score $=5-\left(5^{*} e^{\left(A D T^{*}-0.00025\right)}\right)$
Score Condition
$0<900$
0-5 900-25,000
$5>25,000$

Detour Length
Score Condition
0 Detour < 10 miles
5 Detour > 10 miles
Truck Buffer Index
Truck Buffer Index x 10
Score Condition
$0 \quad$ Buffer Index $=0.00$
0-5 Buffer Index 0.00-0.50
5 Buffer Index $>0.50$
Outside Shoulder Width
Variance below $10^{\prime}$, if only 1 lane in each direction Score Condition
$0 \quad 10^{\prime}$ or above or $>1$ lane in each direction
0-5 10'-5' and 1 lane in each direction
5 5' or less and 1 lane in each direction

| Solution Number | Mainline Traffic Vol (vpd) (2-way) | Solution Length (miles) | Bridge Detour Length (miles) (N19) | Elevation (ft) | Scour <br> Critical <br> Rating <br> (0-9) | Carries Mainline Traffic (Y/N) | Bridge Vert. Clear (ft) | Mainline Truck Vol (vpd) (2-way) | Detour Length > 10 miles (Y/N) | Grade (\%) | Interrupted <br> Flow (Y/N) | Outside/ <br> Right <br> Shoulder <br> Width <br> (ft) | $\begin{aligned} & \text { 1-lane } \\ & \text { each } \end{aligned}$ direction | Segment | Bridge | Pavement | Mobility | Safety | Freight |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CS191.1A | 2,617 | 16 |  | 4,047 |  |  |  | 393 |  | 3 | N | 10.29 | Y | 191.3 | N | N | N | Y | N |
| CS191.1B | 2,617 | 16 |  | 4,047 |  |  |  | 393 |  | 3 | N | 10.29 | Y | 191.3 | N | N | N | Y | N |
| CS70.2 | 11,553 | 0.21 |  | 2,895 |  |  |  | 1,502 |  | 1 | Y | 2.85 | N | 70.6 | N | N | N | Y | N |
| CS70.3A | 2,749 | 5 |  | 2,626 |  |  |  | 687 |  | 3 | N | 11.74 | Y | 70.9 | N | N | N | N | N |
| CS70.3B | 2,749 | 5 |  | 2,626 |  |  |  | 687 |  | 3 | N | 11.74 | Y | 70.9 | N | N | N | N | N |
| CS70.4 | 2,749 | 18 |  | 2,000 |  |  |  | 687 | $Y$ | 3 | N | 5.07 | $Y$ | 70.10 | N | N | Y | Y | N |
| CS70.5 | 5,790 | 15 |  | 2,000 |  |  |  | 984 | Y | 3 | N | 10.76 | Y | 70.12 | N | N | Y | Y | N |
| CS70\|60.6 | 11,143 | 12 |  | 3,500 |  |  |  | 1,226 | Y | 3 | Y | 9.18 | N | 70\|60.13 | N | N | Y | Y | Y |
| CS70\|60.7A | 11,143 | 0.5 |  | 3,500 | 8 | Y | 15.84 | 1,226 | Y | 3 | Y | 9.18 | N | 70\|60.13 | N | N | N | Y | Y |
| CS70\|60.7B | 11,143 | 0.5 |  | 3,500 | 8 | Y | 15.58 | 1,226 | Y | 3 | Y | 9.18 | N | 70\|60.13 | N | N | Y | Y | Y |
| CS60.8 | 10,128 | 16 |  | 3,500 |  |  |  | 1,114 | Y | 5 | N | 4.82 | Y | 60.14 | N | N | Y | Y | Y |
| CS60.9A | 10,128 | 0.23 | 37 | 3,500 | 8 | Y | 15.70 | 1,114 | Y | 5 | N | 4.82 | Y | 60.14 | N | N | N | $Y$ | Y |
| CS60.9B | 10,128 | 0.23 | 37 | 3,500 | 8 | Y | 15.70 | 1,114 | Y | 5 | N | 4.82 | Y | 60.14 | N | N | Y | Y | Y |
| CS60.10 | 11,048 | 7 |  | 2,156 |  |  |  | 1,436 | Y | 3 | N | 10.06 | N | 60.17 | N | N | Y | Y | Y |
| CS60.11 | 14,003 | 3 |  | 1,844 |  |  |  | 1,820 | Y | 1 | N | 8.08 | Y | 60.18 | N | N | $Y$ | Y | Y |
| CS60.12 | 18,653 | 7 |  | 1,755 |  |  |  | 2,052 | Y | 2 | Y | 15.05 | Y | 60.19 | N | N | Y | Y | Y |
| CS60.13 | 48,507 | 4.7 |  | 1,643 |  |  |  | 4,366 | N | 1 | N | 10 | Y | 60.20 | N | N | $Y$ | Y | Y |
| CS60.14 | 48,507 | 2.7 |  | 1,643 |  |  |  | 4,366 | N | 1 | N | 10 | Y | 60.20 | N | N | Y | Y | Y |


| Solution Number | Bridge | Pavement | Mobility | Safety | Freight | Risk Score (0 to 10) |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  | Bridge | Pavement | Mobility | Safety | Freight |
| CS191.1B | N | N | N | Y | N | 0.00 | 0.00 | 0.00 | 0.21 | 0.00 |
| CS70.2 | N | N | N | Y | N | 0.00 | 0.00 | 0.00 | 4.72 | 0.00 |
| CS70.4 | N | N | Y | Y | N | 0.00 | 0.00 | 8.28 | 2.17 | 0.00 |
| CS70.5 | N | N | Y | Y | N | 0.00 | 0.00 | 5.67 | 0.40 | 0.00 |
| CS70\|60.6 | N | N | Y | Y | Y | 0.00 | 0.00 | 6.15 | 3.03 | 4.22 |
| CS70\|60.7A | N | N | N | Y | Y | 0.00 | 0.00 | 0.00 | 3.03 | 4.22 |
| CS70\|60.7B | N | N | Y | Y | Y | 0.00 | 0.00 | 3.58 | 3.03 | 4.22 |
| CS60.8 | N | N | Y | Y | Y | 0.00 | 0.00 | 9.65 | 3.85 | 7.48 |
| CS60.9A | N | N | N | Y | Y | 0.00 | 0.00 | 0.00 | 3.85 | 7.48 |
| CS60.9B | N | N | Y | Y | Y | 0.00 | 0.00 | 6.77 | 3.85 | 7.48 |
| CS60.10 | N | N | Y | Y | Y | 0.00 | 0.00 | 5.53 | 0.69 | 4.34 |
| CS60.11 | N | N | Y | Y | Y | 0.00 | 0.00 | 6.09 | 1.60 | 5.84 |
| CS60.12 | N | N | Y | Y | Y | 0.00 | 0.00 | 6.12 | 3.03 | 4.68 |
| CS60.13 | N | N | Y | Y | Y | 0.00 | 0.00 | 3.19 | 1.69 | 2.22 |
| CS60.14 | N | N | Y | Y | Y | 0.00 | 0.00 | 2.79 | 1.69 | 2.22 |

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Appendix H: Candidate Solution Cost Estimates

| Candidate Solution \# | Location \# | Candidate Solution Name | Investment Category (Preservation [P], Modernization [M], Expansion <br> [E]) | Scope | BMP | EMP | Unit | Quantity | Factored Construction Unit Cost | Preliminary Engineering Cost | Design Cost | Right-of- <br> Way <br> Cost <br> $\$ \$ 6 /$ SF <br> for rural <br> areas <br> and <br> $\$ 12 / \mathbf{/ s F}$ <br> for <br> urban <br> areas) | Construction Cost | Total Cost | Notes |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CS191.A | L5 | US191 <br> Pavement Preservation South of Safford | P | Rehabilitate Pavement (AC) | 88.0 | 104.0 | mi | 32.0 | \$1,060,000 | \$1,018,000 | \$3,392,000 | \$0 | \$33,920,000 | \$38,330,000 | Cost only accounts for one direction of travel so quantity is doubled |
|  |  |  |  | Solution Total |  |  |  |  |  | \$1,018,000 | \$3,392,000 | \$0 | \$33,920,000 | \$38,330,000 |  |
| CS191.1B |  |  | M | Replace <br> Pavement (AC) <br> (with <br> overexcavation) | 88.0 | 104.0 | mi | 32.0 | \$5,540,000 | \$5,318,000 | \$17,728,000 | \$0 | \$177,280,000 | \$200,326,000 | Cost only accounts for one direction of travel so quantity is doubled |
|  |  |  |  |  |  |  |  |  | Solution Total | \$5,318,000 | \$17,728,000 | \$0 | \$177,280,000 | \$200,326,000 |  |
| CS70.2 | เ9 | East Safford Safety Improvements | M | Provide flashing traffic signal warning signs at Milepost 337.82 and Milepost 338.03 | 337.8 | 338.0 | ea | 2.0 | \$41,300 | \$2,000 | \$8,000 | \$0 | \$82,600 | \$92,600 | In both directions; includes warning sign, post, and foundation, and flashing beacons (assumes solar power) at one location |
|  |  |  |  | Consider installing speed zone signs in both directions at 20th Avenue |  |  | ea | 1.0 | \$6,900 | \$0 | \$1,000 | \$0 | \$6,900 | \$7,900 |  |
|  |  |  |  | (Rehabilitate |  |  |  |  |  | \$2,000 | \$9,000 | \$0 | \$90,000 | \$101,000 |  |
| Cs70.3A | L13 | Bylas Area <br> Pavement <br> Preservation | P | Rehabilitate Pavement (AC) | 293.0 | 298.0 | mi | 10.0 | \$1,060,000 | \$318,000 | \$1,060,000 | \$0 | \$10,600,000 | \$11,978,000 | Cost only accounts for one direction of travel so quantity is doubled |
|  |  |  |  | Solution Total |  |  |  |  |  | \$318,000 | \$1,060,000 | \$0 | \$10,600,000 | \$11,978,000 |  |
| C570.3B |  |  | M | Replace <br> Pavement (AC) <br> (with <br> overexcavation) | 293.0 | 298.0 | mi | 10.0 | \$5,540,000 | \$1,662,000 | \$5,540,000 | \$0 | \$55,400,000 | \$62,602,000 | Uses "Replace Bridge (Short)" costs from appendix F |
|  |  |  |  |  |  |  |  |  | Solution Total | \$1,662,000 | \$5,540,000 | \$0 | \$55,400,000 | \$62,602,000 |  |
| CS70.4 | L15 | Bylas to Peridot Safety Improvements | M | Widen Shoulders | 274.0 | 278.0 | mi | 4.0 | \$980,000 | \$118,000 | \$392,000 | \$0 | \$3,920,000 | \$4,430,000 | Assumes 10' of existing shoulder (combined left and right), includes widening shoulder by a total of 4 '; new pavement for 4 ' width and mill and replace existing 10 width; includes pavement, minor earthwork, striping edge lines, RPMs, high-visibility delineators, safety edge, and rumble strips |
|  |  |  |  | $\begin{array}{\|l\|} \hline \text { Install shoulder } \\ \text { rumble strips MP } \\ \text { 275.5-276.5,MP } \\ \text { 279.5-287.5 } \\ \hline \end{array}$ | 275.5 | 287.5 | mi | 18.0 | \$0 | \$0 | \$0 | \$0 | \$0 | \$0 | Cost included in widen shoulders solution |
|  |  |  |  | Install centerline rumble strips MP 275.5-276.5,MP 279.5-287.5 | 275.5 | 287.5 | mi | 9.0 | \$11,000 | \$3,000 | \$10,000 | \$1 | \$99,000 | \$112,001 |  |
|  |  |  |  | Install high visibility striping MP 274-278 | 274.0 | 278.0 | mi | 8.0 | \$29,700 | \$7,000 | \$24,000 | \$0 | \$237,600 | \$268,600 | 2 edge lines and lane line - one direction of travel |


|  |  |  |  | Improve existing pedestrian / speed warning signs to also include flashing beacons and speed feedback signs | 278.5 | 292.0 | ea | 3.0 | \$110,100 | \$10,000 | \$33,000 | \$0 | \$330,300 | \$373,300 | $\$ 68800$ FC Unit Cost for Dynamic Speed Feedback sign, $\$ 41,300$ for flashing beacons |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Construct passing lanes WB | 288.2 | 289.6 | mi | 1.4 | \$5,742,000 | \$241,000 | \$804,000 | \$0 | \$8,038,800 | \$9,083,800 | In one direction; all costs except bridges; applicable to areas with small or moderate fills and cuts, minimal retaining walls |
|  |  |  |  | Formalize pullouts <br> (signage, ROW for pullouts) (WB MP $274.5=9,200$ SF, EB MP 279 = 11,900 SF, EB MP $289=40,600$, WB $292=$ 34,000 ) | 274.5 | 292.0 | ea | 1.0 | \$750,000 | \$23,000 | \$75,000 | \$0 | \$750,000 | \$848,000 | $4 \times$ pullout costs summed to determine FCU cost. MP $274.5=\$ 29,000 ;$ MP $279=\$ 105,000 ;$ MP $289=\$ 308,000 ;$ MP $292=\$ 308,000$ (using proportions of small, medium, and large formalized pullouts from Appendix F. |
|  |  |  |  |  |  |  |  |  | olution Total | \$402,000 | \$1,338,000 | \$1 | \$13,376,000 | \$15,116,000 |  |
| CS70.5 | L19 | East of Globe Safety Improvements | M | Widen Shoulders | 255.0 | 270.0 | mi | 15.0 | \$980,000 | \$441,000 | \$1,470,000 | \$0 | \$14,700,000 | \$16,611,000 |  |
|  |  |  |  | Install centerline rumble strips MP 255-270 | 255.0 | 270.0 | mi | 15.0 | \$11,000 | \$5,000 | \$17,000 | \$1 | \$165,000 | \$187,001 |  |
|  |  |  |  | Install improved lighting | 269.0 | 270.0 | mi | 1.0 | \$1,034,000 | \$31,000 | \$103,000 | \$0 | \$1,034,000 | \$1,168,000 |  |
|  |  |  |  | Construct passing lane in each direction | 255.0 | 256.0 | mi | 2.0 | \$5,742,000 | \$345,000 | \$1,148,000 | \$1 | \$11,484,000 | \$12,977,001 | In one direction; all costs except bridges; applicable to areas with small or moderate fills and cuts, minimal retaining walls |
|  |  |  |  | Install warning signs to include flashing beacons and speed feedback signs | 273.0 | - | ea | 1.0 | \$110,100 | \$3,000 | \$11,000 | \$0 | \$110,100 | \$124,100 | $\$ 68,800$ FC Unit Cost for Dynamic Speed Feedback sign, $\$ 41,300$ for flashing beacons |
|  |  |  |  |  |  |  |  |  | olution Total | \$825,000 | \$2,749,000 | \$2 | \$27,493,000 | \$31,067,000 |  |
| CS70\|60.6 | L26 | Globe Area Safety Improvements | M | Install speed feedback signs (2 EB and 2 WB ) | 246.0 | 250.0 | ea | 4.0 | \$68,800 | \$8,000 | \$28,000 | \$0 | \$275,200 | \$311,200 |  |
|  |  |  |  | High visibility striping | 243.0 | 255.0 | mi | 24.0 | \$29,700 | \$21,000 | \$71,000 | \$0 | \$712,800 | \$804,800 | 2 edge lines and lane line - one direction of travel |
|  |  |  |  | Install warning signs with beacons in advance of SR 188 intersection | 247.1 | - | ea | 2.0 | \$41,300 | \$2,000 | \$8,000 | \$0 | \$82,600 | \$92,600 |  |
|  |  |  |  | Construct passing lane in each direction MP 243-243.25 and MP 253.6255 | 243.0 | 255.0 | mi | 3.3 | \$5,742,000 | \$568,000 | \$1,895,000 | \$1 | \$18,948,600 | \$21,411,601 | In one direction; all costs except bridges; applicable to areas with small or moderate fills and cuts, minimal retaining walls |
|  |  |  |  | Solution Total |  |  |  |  |  | \$599,000 | \$2,022,000 | \$1 | \$20,019,000 | \$22,620,000 |  |
| CS70\|60.7 | L27 |  | E | Reconstruct <br> Pinal SPRR UP <br> to increase | 253.63 | 253.63 | SF | 11916.0 | \$610 | \$218,000 | \$727,000 | \$0 | \$7,268,760 | \$8,213,760 | Medium bridge |


|  |  | Globe Area Freight Improvements |  | $\begin{aligned} & \hline \text { vertical } \\ & \text { clearance } \end{aligned}$ |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Solution Total |  |  |  |  |  | \$218,000 | \$0 | \$0 | \$7,269,000 | \$8,214,000 |  |
|  |  |  | M | Reprofile mainline to increase vertical clearance | 253.63 | 253.634 | mi | 0.5000 | \$3,730,000 | \$56,000 | \$187,000 | \$1 | \$1,865,000 | \$2,108,001 |  |
|  |  |  |  | Solution Total |  |  |  |  |  | \$56,000 | \$0 | \$1 | \$1,865,000 | \$2,108,000 |  |
| CS60.8 | L32 | Miami Area West Safety Improvements | M | Consider installing speed feedback signs MP 229.9, 236, and 241 | 229.9 | 241.0 | ea | 3.0 | \$68,800 | \$6,000 | \$21,000 | \$0 | \$206,400 | \$233,400 |  |
|  |  |  |  | Install centerline rumble strips | 229.0 | 231.0 | mi | 2.0 | \$11,000 | \$1,000 | \$2,000 | \$0 | \$22,000 | \$25,000 |  |
|  |  |  |  | Install high visibility striping | 227.0 | 243.0 | mi | 32.0 | \$29,700 | \$29,000 | \$95,000 | \$0 | \$950,400 | \$1,074,400 | 2 edge lines and lane line - one direction of travel |
|  |  |  |  | Install high visibility delineators MP 228-228.3 \& MP 241-242 | 228.0 | 242.0 | mi | 2.6 | \$17,900 | \$1,000 | \$5,000 | \$1 | \$46,540 | \$52,541 | Both edges - one direction of travel |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  | Rehabilitate shoulder | 227.0 | 243.0 | mi | 32.0 | \$433,000 | \$416,000 | \$1,386,000 |  | \$13,856,000 | \$15,658,000 |  |
|  |  |  |  | Solution Total |  |  |  |  |  | \$453,000 | \$1,509,000 | \$1 | \$15,081,000 | \$17,043,000 |  |
| CS60.9 | L34 | Miami Area West Freight Improvements | E | Reconstruct Pinal Queen Creek Tunnel to increase vertical clearance | 228.47 |  | SF | 48341.0 | \$610 | \$885,000 | \$2,949,000 | \$0 | \$29,488,010 | \$33,322,010 | Medium bridge |
|  |  |  |  | Solution Total |  |  |  |  |  | \$885,000 | \$2,949,000 | \$0 | \$29,488,000 | \$33,322,000 |  |
|  |  |  | M | Reprofile mainline to increase vertical clearance | 228.47 | 228.7 | mi | 0.5 | \$3,730,000 | \$51,000 | \$172,000 |  | \$1,715,800 | \$1,938,800 |  |
|  |  |  |  | Solution Total |  |  |  |  |  | \$51,000 | \$172,000 | \$0 | \$1,716,000 | \$1,939,000 |  |
| CS60.10 | L39 | Superior Area Safety Improvements | M | Consider installing speed feedback signs (2 EB and 2 WB ) | 212.5 | - | ea | 4.0 | \$68,800 | \$8,000 | \$28,000 | \$0 | \$275,200 | \$311,200 |  |
|  |  |  |  | Install lighting at N Queen Valley Road and US 60 intersection | 214.2 | - | ea | 2.0 | \$38,300 | \$2,000 | \$8,000 | \$0 | \$76,600 | \$86,600 | 2 poles for the intersection - 1 each at WB and EB legs; not high-mast; solar power LED |
|  |  |  |  | Install chevrons | 219.3 | 219.7 | mi | 0.7 | \$50,600 | \$1,000 | \$4,000 | \$0 | \$37,444 | \$42,444 | On one side of road - includes signs, posts, and foundations |
|  |  |  |  | Install curve warning sign | 219.3 | - | ea | 1.0 | \$6,900 | \$0 | \$1,000 | \$0 | \$6,900 | \$7,900 | Includes 2 signs, posts, and foundations |
|  |  |  |  |  |  |  |  |  | solution Total | \$11,000 | \$41,000 | \$0 | \$396,000 | \$448,000 |  |
| CS60.11 | 143 | US-60 SW of Gold Canyon Safety | M | Consider installing speed feedback signs | 205.0 | 208.0 | ea | 2.0 | \$68,800 | \$4,000 | \$14,000 | \$0 | \$137,600 | \$155,600 |  |
|  |  |  |  | Install lighting | 205.0 | 207.0 | mi | 2.0 | \$1,034,000 | \$62,000 | \$207,000 | \$0 | \$2,068,000 | \$2,337,000 | One side of road only; offset lighting, not high-mast; does not include power supply; includes poles, luminaire, pull boxes, conduit, conductor |

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|  |  |  |  | Widen inside shoulder | 208.3 | 212.0 | mi | 1.3 | \$980,000 | \$38,000 | \$127,000 | \$0 | \$1,274,000 | \$1,439,000 | 4' of widening, including paving, striping, high visibility delineators, RPMs, safety edge, and rumble strip |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Solution Total |  |  |  |  |  | \$104,000 | \$348,000 | \$0 | \$3,480,000 | \$3,932,000 |  |
| CS60.12 | L44 | Gold Canyon Area Mobility and Safety Improvements | E | Add SB/EB through lane | 199.1 | 206.0 | mi | 6.9 | \$4,590,000 | \$947,000 | \$3,158,000 | \$437,184 | \$31,579,200 | \$36,121,384 | There are some guardrails in a few places that would need to get moved. Description from Appendix F: "For addition of 1 GP lane (AC) in one direction; includes all costs except bridges; for generally at-grade facility with minimal walls and no major drainage improvements" |
|  |  |  |  | Widen shoulders | 199.1 | 205.0 | mi | 5.9 | \$980,000 | \$173,000 | \$576,000 | \$124,608 | \$5,762,400 | \$6,636,008 | Assumes 10' of existing shoulder (combined left and right - must be confirmed), includes widening shoulder by a total of 4 '; new pavement for 4' width and mill and replace existing 10 ' width; includes pavement, minor earthwork, striping edge lines, RPMs, high-visibility delineators, safety edge, and rumble strips |
|  |  |  |  | Install speed feedback sign | 201.0 | - | еа | 1.0 | \$68,800 | \$2,000 | \$7,000 | \$0 | \$68,800 | \$77,800 |  |
|  |  |  |  | Install lighting | 201.0 | 202.0 | mi | 1.0 | \$1,034,000 | \$31,000 | \$103,000 | \$0 | \$1,034,000 | \$1,168,000 |  |
|  |  |  |  |  |  |  |  |  | Solution Total | \$1,153,000 | \$3,844,000 | \$562,000 | \$38,444,000 | \$44,003,000 |  |
| C560.13 | L46 | Apache Junction Area Mobility and Freight | M | Add through lane in NB/WB direction | 194.3 | 199.0 | mi | 4.7 | \$4,590,000 | \$647,000 | \$2,157,000 | \$297,792 | \$21,573,000 | \$24,674,792 | from Appendix F: "For addition of 1 GP lane (AC) in one direction; includes all costs except bridges; for generally at-grade facility with minimal walls and no major drainage improvements" Includes two places where existing road passes over surface street and existing structure may not be sufficient |
|  |  |  |  | Solution Total |  |  |  |  |  | \$647,000 | \$2,157,000 | \$297,792 | \$21,573,000 | \$24,675,000 |  |
| C560.14 | L47 | Apache Junction Area Safety Improvements | M | Consider installing speed feedback signs | 195.0 | - | ea | 2.0 | \$68,800 | \$4,000 | \$14,000 | \$0 | \$137,600 | \$155,600 |  |
|  |  |  |  | Install inside and edgeline rumble strips | 194.3 | 197.0 | mi | 5.4 | \$21,000 | \$3,000 | \$11,000 | \$0 | \$113,400 | \$127,400 |  |
|  |  |  |  |  |  |  |  |  | Solution Total | \$7,000 | \$25,000 | \$0 | \$251,000 | \$283,000 |  |

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Appendix I: Performance Effectiveness Scores

Need Reduction


|  | Solution \# | 60.10 | 60.11 | 60.12 | 60.13 | 60.14 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Description | Superior Area Safety Improvements | Us-60 SW of Gold Canyon Safety Improvements | Gold Canyon Area Mobility and Safety Imrovements | Apache Junction Area Mobility and Freight Improvements | Apache Junction Area Safety Improvements |
| LEGEND: | Project Beg MP | 212 | 208 | 205 | 199 | 199 |
|  | Project End MP | 219 | 206 | 199 | 194.3 | 194.3 |
|  | Project Length (miles) | 7 | 2 | 6.88 | 4.7 | 4.7 |
|  | Segment Beg MP | 212 | 205 | 199 | 194.3 | 194.3 |
|  | Segment End MP | 223 | 212 | 205 | 199 | 199 |
|  | Segment Length (miles) | 11 | 7 | 6 | 4.7 | 4.7 |
|  | Segment \# | 60-17 | ${ }^{60-18}$ | 60-19 | 60-20 | 60-20 |
|  | Current \# of Lanes (both directions) | 4 | 2 | 2 | 2 | 2 |
|  | Project Type (one-way or two-way) | wo-way | two-way | one-way | ne-way | two-way |
|  | Additional Lanes (one-way) |  |  | 1 | 1 |  |
|  | Pro-Rated \# of Lanes | 4.00 | 2.00 | 3.15 | 3.00 | 2.00 |


| 旁 |  | Orig segment Directional Safety Index (NB) | 1.820 | 0.910 | 1.620 | 1.890 | 1.890 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Orig Segment Directional Fatal Crashes (NB) | 3 | 1 | 2 | 3 | 3 |
|  |  | Oris Segment Directional Suspected Serious Crashes (NB) | 3 | 3 | 2 | 0 | 0 |
|  |  | Original Fatal Crashes in project limits (NB) | 3 | 1 | 2 | 3 | 3 |
|  |  | Original Suspected Serious Crashes in project limits (NB) CMF 1 (NB)(lowest CMF) | 3 | 2 | 2 | $\stackrel{0}{0.9}$ | 0 |
|  |  | CMF2 (NB) |  |  |  | 1 |  |
|  |  |  | TotaicmFle Calculated in Separate Workbook | TotarcmF Calcuated in Separate Workbook |  | 1 | Separate Workbook |
|  |  | CMF5 (NB) |  |  |  | 1 |  |
|  |  | Total CMF (NB) | 1.000 | 1.000 | 1.000 | 0.900 | 1.000 |
|  |  | Fatal Crash reduction (NB) | 0.250 | 0.060 | 0.250 | 0.300 | 0.330 |
|  |  | Suspected Serious Crash reduction (NB) | 0.060 | 0.940 | 0.250 | 0.000 | 0.000 |
|  |  | Post-Project Segment Directional Fatal Crashes (NB) | 2.750 | 0.940 | 1.750 | 2.700 | 2.670 |
|  |  | Post-Project Segment Directional Suspected Serious Crashes (NB) | 2.940 | 2.060 | 1.750 | 0.000 | 0.000 |
|  |  | Post-Project Segment Directional Safety Index (NB) | 1.670 | 0.790 | 1.420 | 1.700 | 1.690 |
|  |  | Post-Project Segment Directional Safety Index (NB) | 1.670 | 0.790 | 1.420 | 1.700 | 1.690 |
|  |  | Orig Segment Directional Safety Index (SB) | 0.650 | 0.090 | 0.270 | 0.690 | 0.690 |
|  |  | Orig Segment Directional Fatal Crashes (SB) | 1 | 0 | 0 | 1 | 1 |
|  |  | Orig Segment Directional Suspected Serious Crashes (SB) | 2 | 2 | 6 | 2 | 2 |
|  |  | Original Fatal Crashes in projectl limits (SB) | 1 | 0 | 0 | 1 | 1 |
|  |  | Original Suspected Serious Crashes in project limits (SB) CMF 1 (SB)(lowest CMF) | 2 | 1 | 6 | $\stackrel{2}{0.9}$ | 2 |
|  |  | CMF2 (SB) |  |  |  | 1 |  |
|  |  | CMF3 (SB) | (total CMFFCalculated in | Tota CMEMF Calculate in Separate Workbook | Totar CMF Calaluated in Separate Workbook | 1 | Total CMFMFCalculated in Separate Workbook |
|  |  | CMF 4 (SB) CMF 5 (SB) |  |  |  | $\begin{aligned} & 1 \\ & 1 \end{aligned}$ |  |
|  |  | Total CMF (SB) | 1.000 | 1.000 | 1.000 | 0.900 | 1.000 |
|  |  | Fatal Crash reduction (SB) | 0.527 | 0.000 | 0.000 | 0.100 | 0.110 |
|  |  | Suspected Serious Crash reduction (SB) | 0.000 | 0.630 | 2.580 | 0.200 | 0.280 |
|  |  | Post-Project Segment Directional Fatal Crashes (SB) | 0.473 | 0.000 | 0.000 | 0.900 | 0.890 |
|  |  | Post-Project Segment Directional Suspected Serious Crashes (SB) | 2.000 | 1.370 | 3.420 | 1.800 | 1.720 |
|  |  | Post-Project Segment Directional Safety Index (SB) | 0.330 | 0.090 | 0.150 | 0.620 | 0.610 |
|  |  | Post-Project Segment Directional Safety Index (SB) | 0.330 | 0.090 | 0.150 | 0.620 | 0.610 |
|  |  | Current Safety Index | 1.235 | 0.500 | 0.945 | 1.290 | 1.290 |
|  | 岕 | Post-Project Safety Index | 1.000 | 0.440 | 0.785 | 1.160 | 1.150 |
|  |  | Original Segment Safety Need | 3.111 | 0.318 | 1.023 | 2.515 | 2.515 |
|  |  | Post-Project Segment Safety Need | 1.858 | 0.281 | 0.812 | 2.440 | 2.038 |


| LEGENO： | Solution | ${ }^{19.1 .1 A}$ | 19.118 | 70.2 | 70．3A | ${ }^{20.38}$ | 70.4 | 70.5 | 70／60．6 | 70／60．7A | 70／60，78 | 60.8 | 60.94 | 60.98 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Descrition | US191 Pavement Preservation South of Safford |  | Shea Boulevard Safety Improvements | Bylas Area Pavement Preservation | Bylas Area Pavement Preservation | Bylas to Peridot Safety Improvements | East of Globe Safety | Globe Area Safety Improvements | Globe Area Freight Improvements | Globe Area Freight Improvements | Miami Area West Safety | Miami Area Freight Improvements | Miami Area Freight |
|  | $\xrightarrow[\substack{\text { Proiect be } \mathrm{MP} \\ \text { Proiecten } \\ \text { P }}]{ }$ | 88 <br> 104 | 88 <br> 104 |  | （298 | 298 <br> 293 | ${ }_{\substack{289.6 \\ 288.2}}^{\substack{\text { 2，}}}$ | $\underset{\substack{270 \\ 255}}{ }$ | ${ }_{\substack{255 \\ 24 \\ 125}}$ | ${ }_{2}^{255}$ | ${ }_{2}^{255}$ | ${ }_{227}^{223}$ | ${ }_{227}^{224}$ | ${ }_{227}^{223}$ |
|  | Projectierght（mies | ${ }_{10}^{104}$ | ${ }_{16}^{104}$ | ${ }_{\substack{33.722 \\ 0.21}}$ | ${ }_{5}^{293}$ | 5 | 288.2 1.4 | ${ }^{25}$ | 2.65 <br> 1.65 | ${ }_{12}^{243}$ | ${ }_{12}^{24}$ | ${ }_{16} 27$ | ${ }_{12} 12$ | ${ }_{16}^{27}$ |
|  |  | 87 104 10， | ${ }_{\substack{87 \\ 104}}$ | ¢ |  |  | 274 <br> 293 <br> 1 | 205 | $\underset{\substack{243 \\ 255}}{\substack{2 \\ \hline}}$ | $\underset{\substack{243 \\ 255}}{2}$ | $\underset{\substack{243 \\ 255}}{2}$ | ${ }_{24}^{227}$ | ${ }_{24}^{227}$ | ${ }_{\substack{227 \\ 223 \\ \hline}}$ |
|  |  | 104 101 19 | 104 101 19 | ${ }_{7} 9$ |  | cos 7 70.9 | 19 70.10 7 | 15 <br> 70.12 | 12 7060.13 | 12 7060.13 | 12 7060.13 | ${ }_{\substack{16 \\ 60.14}}^{129}$ | 16 <br> 60.14 <br> 6 | 16 <br> 60.14 |
|  | Currentt of tanes bioth firemetions | $\stackrel{191.3}{4}$ | $\stackrel{19}{19.3}$ | 70.6 4 | $\stackrel{70.9}{ }$ | $\stackrel{70.9}{2}$ | $\underset{\substack{70.10 \\ 2}}{ }$ | $\stackrel{70.12}{2}$ | 70／60．13 | \％0／60．13 | 70／60．13 | $\stackrel{60.14}{24}$ | $\stackrel{60.14}{24}$ | $\stackrel{60.14}{2}$ |
|  |  | swoway | twoway | twoway | ewoway | （wower | oneway |  | twoway | twoway | ewoway | ewoway | twowe | twowe |
|  | Pro．Ratedt fof tanes | 4.00 | 4.00 | 4.00 | 200 | 2.00 | 2.07 | 2.50 | 4.28 | 4.00 | 4.00 | 2.00 | 2.00 | 2.00 |


|  | Original Segnent Mobility ndex | 0.55 | 0.550 | 0.410 | ${ }^{0.200}$ | ${ }^{0.200}$ | 0.150 | 0.20 | 0.400 | 0.400 | 0.400 | 1.420 | 1.420 | 1.420 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 旁 | Piectu f f L neses（bot direction） | 4.00 | 4.00 | 4.00 | 2.00 | 2.00 | 2.07 | 2.50 | 4.28 | 4.00 | 4.00 | 2.00 | 200 | 2.00 |
|  | Postreproet segenent Mobility ndex | 0.05 | 0.05 | 0.41 | 0.24 | 0.24 | 0.15 | 0.19 | ${ }_{0} 0.37$ | 0.40 | 0.40 | ${ }_{1.42}$ | ${ }_{1.42}$ | ${ }_{1} 142$ |
|  |  | ${ }^{0.050}$ | 0.050 | 0.410 | 0.220 | 0．220 | 0.150 | 0.190 | 0.370 | 0.400 | 0.400 | 1.420 | 1.420 | 1.420 |
| 5\％ | Ond | $\substack{0.050 \\ \text { 0．050 }}$ | 0．0．5s 0．050 | （0．450 | （0．260 | ${ }^{0.260}$ | 0.170 0.170 | （0．200 | （0．450 | O． 0.450 | O．0．450 | 1.770 1.710 |  | 1．770 <br> 1.710 |
|  | Postriofiect sementen futue V／C | ${ }_{\substack{0.050}}^{0.005}$ | 0.00 | O．450 | － | 0.000 | ${ }_{0}^{0.170}$ |  | O．420 | O．450 | O．450 | ${ }_{1}^{1.710}$ | ${ }_{1}^{1.710}$ | ${ }_{1.710}^{1.70}$ |
| 边 |  | （0．0．030 | （i．0．030 |  | （0．160 | 0．0．60 | （0．110 | （0．150 | （0．250 | － | －0．200 | $\xrightarrow{0.790}$ | ${ }_{\substack{0.790 \\ 1.140}}^{\text {a }}$ |  |
|  | Adjused toall of t anes for usei in diectional peak hr | N／ | N／A | N／A | N／A | N／A | 2.15 | N／A | N／A | N／A | N／A | N／A | N／A | N／A |
|  | Postriforject Segement Peaktrv／（ NB） | 0.30 | 0.330 | 0.310 | 0.160 | 0.160 | 0.100 | 0.130 | 0.24 | 0.26 | 0.26 | 0.79 | 0.79 | 0.79 |
|  |  | ${ }_{0}^{0.030}$ | 0.300 | 0.290 | 0.120 | 0.120 | 0.070 | 0.140 | ${ }^{0.24}$ | ${ }^{0} 25$ | 0．25 | ${ }^{1.19}$ | ${ }_{1}^{1.19}$ | ${ }^{1.19}$ |
|  |  | （0．030 | （0．030 | － $\begin{aligned} & 0.310 \\ & 0.290\end{aligned}$ | 0.160 0.120 | （0．160 | （0．100 | （0．130 | － | （e．250 | （0．250 | －0．790 <br> 1.190 |  | 0．790 <br> 1.140 |
| 言 | Siter Peedection fator | （0．678 | ${ }_{\substack{0.703 \\ 0.297}}^{0.0}$ | 0．973 |  | $\substack{\text { Hopvol } \\ \text { noval }}$ | （0．745 | 0．179 | －0．747 | 1．000 <br> 0.000 |  | ． 0.599 | 1.000 <br> $\substack{1.000}$ <br> 1000 | 品， 0.700 |
|  | Sater neadection | ${ }_{\substack{0.322 \\ 1000}}^{\text {i，}}$ |  | ${ }^{0.027}$ | Hovvol | Hovvol | ． 0.255 | 0.0 .81 | 0．0．233 | ${ }^{0.000}$ |  | ${ }^{0.401}$ | ${ }^{0.000}$ | － 0.300 |
|  |  | （1．0．000 | （1．000 | ${ }_{\substack{1.000 \\ 0.000}}^{\substack{\text { a }}}$ | （1．000 | ${ }_{\substack{1.000 \\ 0.000}}$ | （1．000 | （0．0．928 | 0.925 0.075 | 1．000 <br> 0.000 |  | ${ }^{1.000} 0$ | （1．000 | 1.000 <br> 0.000 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | Orisinal irectional Segment LOTRR（（SE） | no data | nodata | no data | no data | no data | no data | no data | 1.160 1.150 1 | 11160 | 11160 | 1．120 | 1.120 1120 1 | ${ }^{1.120}$ |
|  |  | 0.097 | 0.089 | 0.008 | Hovvo | Hovvo | 0.076 | 0.288 | 0.091 | ${ }_{0}^{1.000}$ | 1.050 0.090 | 0.120 | 0.000 | ${ }_{0}^{1.090}$ |
|  | Post－Project iriectional Segmen Lotre（ MB） | mvaluel | Havuel | Hvaluel | Hvaluel | Hvaluel | Hvaluel | maalei | 1.055 | 1.160 | 1.056 | 1.060 | 1.120 | 1.019 |
|  | Post－Project Directional sement LOTR（S8） | mvaluel | Haluel | matuel | Hvaluel | Hvaluel | nodata | maauel | 1.045 | 1.150 | 1.096 | 1.029 | 1.17 | 1.06 |
| 宮 |  |  |  |  |  |  |  |  |  | －0．20 | －0．20 | （0．670 | （0．670 |  |
|  | Orig Segment Directional Closure Extent（SB） <br> Segment Closures with fatalities／injuries | 0.000 | 0.000 | 0.040 0 | 0.040 | co．0．000 | 0.050 | $\begin{aligned} & 0.000 \\ & 0.000 \end{aligned}$ | 0.350 | （1．3500.3 | 0.350 | 1.840 12 12 | 1.340 12 10 | 1.840 <br> 12 <br> 90 |
|  |  | 0.00 | 0.00 | 0.00 | ${ }_{0} 0.00$ | ${ }_{0} 0.00$ | 0.08 | 0.00 | 0.21 | ${ }^{29}$ | ${ }_{0}^{29}$ | 0.12 | ${ }_{0}^{0.12}$ | 0.12 |
|  | Cosure fediction | （i．000 | （0．000 | $c0000000$ | Hovvo | $\substack{\text { Hoivol } \\ \text { novyou }}$ | （0．021 | 0．000 | O．052 | － | 0．0．028 | 0．099 | 0．000 | 0.036 <br> 0.064 <br> 0.0 |
|  | Poos－rofect Segmen Directional Cossure Exeent（N8） | 0.020 | 0.020 | 0.020 | Hovvol | Hovvol | 0.069 | 0.170 | 0.208 | 0.220 | 0.206 | 0.637 | 0.670 | ${ }_{0}^{0.646}$ |
|  | Post－Project Segmen Directional Cosure Exent（SS） | 0.000 | 0.000 | 0.040 | \＃ovvo | \＃ovvo | 0．050 | 0.000 | 0.332 | 0.350 | 0.328 | 1．751 | 1.840 | 1.773 |
| $\stackrel{5}{6}$ |  |  | ${ }_{\text {c }}^{4.0 \% \%}$ | 46．0\％ | 26．0\％ |  | ${ }_{4}^{4.0 \%}$ | 23．0\％ | 54．0\％ | 54．0\％ | 54.08 | 49．0\％ | 49．0\％ | 49．0\％ |
|  |  | 10.3 <br> 103 <br> 1 | 10.3 <br> 103 | ${ }_{29}^{29}$ | 5.9 5.9 5.9 | 5.9 5.9 5， | ${ }_{5.1}^{5.1}$ | ${ }_{\substack{5.4 \\ 54 \\ \text { 5，}}}$ | 4.6 4.6 | 4.6 4.6 | 4.6 4.6 | 4.8 4.8 | ${ }_{\text {4，}}^{4.8}$ | 4.8 4.8 |
|  |  | 490\％ | 49．0\％ | 46．0\％ | $26.0 \%$ | 26．0\％ | $4.0 \%$ | 23．0\％ | 54．0\％ | $54.0 \%$ | 54，0\％ | 49．0\％ | 4．8\％ | 49．0\％ |
|  | Postripioet Segnent Bicrece Accomodation（\％） | 490\％ | 490\％ | 46．0\％ | $26.0 \%$ | $26.0 \%$ | 4．0\％ | 23．0\％ | 54．0\％ | $54.0 \%$ | 54．0\％ | \％ | $4.8 \%$ | 49．0\％ |
| veeds | Original Segment Mobilit Need | 0.562 | 0.562 | 0.888 | 0.863 | 0.863 | 0.862 | 0.834 | 0.983 | 0.983 | 0.983 | 10.54 | 10.954 | 10.959 |
|  | Postr．foriect Segnent Mobility wed | 0.562 | 0.562 | 0.888 | 0.863 | 0.863 | 0.860 | 0.839 | 0.946 | 0.983 | 0.970 | 10.91 | 54 | 913 |



|  | 0.260 | 0.530 | 1.010 | 1.310 | 1.310 |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4.00 | 2.00 | 3.15 | 3.00 | 2.00 |
|  | 0.26 | 0.53 | 0.64 | 0.87 | 1.31 |
|  | 0.260 | 0.530 | 0.640 | 0.870 | 1.310 |
|  | 0.370 | 0.660 | 0.860 | 1.450 | 1.450 |
| ${ }^{5}$ | 0.370 | 0.660 | 0.550 | 0.960 | 1.450 |
|  | 0.370 | 600 | 0.550 | 60 | 1.450 |
|  | ${ }_{0}^{0.150} 0$ | ${ }_{0}^{0.300} 0$ | 0.860 0.910 | 0.840 0.880 | ${ }_{0}^{0.840} 0$ |
|  | N/A | N/A | 4.29 | 4.00 | N/A |
|  | 0.15 | 0.30 | 0.54 | 0.56 | 0.84 |
|  | 0.14 | 0.32 | 0.58 | 0.58 | 0.88 |
|  | 0.150 | 0.300 | 0.540 | 0.560 | 0.840 |
|  | 0.140 | 0.320 | 0.580 | 0.580 | 0.880 |
| $\stackrel{\text { ¢ }}{\text { ¢ }}$ | 0.810 | 0.880 | 0.831 | 0.899 | 0.891 |
|  | 0.190 | 0.120 | 0.169 | 0.101 | 0.109 |
|  | 1.000 | 1.000 | 0.634 | 0.664 | 1.000 |
|  | 0.000 | 0.000 | 0.366 | 0.336 | 0.000 |
|  | 030 | 0.30 | 0.30 | 0.30 | 20 |
|  | 1.050 | 1.120 | 1.200 | 1.060 | 1.060 |
|  | 1.090 | 1.050 | 1.140 | 1.060 | 1.060 |
|  | 0.057 | 0.036 | 0.124 | 0.097 | 0.033 |
|  | 1.025 | 1.080 | 1.051 | 1.030 | 1.025 |
|  | 1.028 | 1.012 | 1.140 | 1.060 | 1.025 |
|  | 0.040 | 0.000 | 0.100 | 0.680 | 0.680 |
|  | 0.330 | 0.230 | 0.300 | 0.090 | 0.090 |
|  | 3 | 2 | 3 | 4 | 4 |
|  | 12 | 8 | 12 | 18 | 18 |
|  | 0.25 | 0.25 | 0.25 | 0.22 | 0.22 |
|  | 0.048 | 0.330 | 0.042 | 0.022 | 0.024 |
|  | 0.952 | 0.970 | 0.958 | 0.978 | 0.976 |
|  | 0.038 | 0.000 | 0.096 | 0.665 | 0.664 |
|  | 0.219 | 0.223 | 0.300 | 0.990 | 0.088 |
|  | 96.0\% | 100.0\% |  | 100.0\% | 100.0\% |
|  | 10.1 | ${ }_{8}^{8.1}$ | 7.5 7 7 | 10.0 | 10.0 |
|  | 10.1 | ${ }^{8.1}$ | 7.5 | 10.0 |  |
|  | 96.0\%\% | - | 42.0\% | 100.0\% | 0\% |
| Needs | 0.431 | 0.790 | 4.421 | 7.502 | 7.502 |
|  | 0.426 | 0.786 | 1.233 | 2.736 | 7.493 |








## Performance Effectiveness Scoring

| Candide | ${ }_{\substack{\text { candidate Solution } \\ \text { Name }}}^{\substack{\text { a }}}$ | (ictiepost | Estimated Cost (\$millions) | Pavement |  |  |  |  | Bridge |  |  |  |  | satery |  |  |  |  | Mobility |  |  |  |  | Freight |  |  |  |  | $\begin{gathered} \text { Total Risk Factored } \\ \text { Performance Area Benefit } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | $\begin{array}{\|c\|c\|c\|c\|c\|c\|c\|r\|r\|} \substack{\text { Segnean } \\ \text { Noed }} \end{array}$ | $\begin{array}{\|c\|} \hline \text { Post-Solution } \\ \text { Segment } \\ \text { Need } \\ \hline \end{array}$ | Raw | ${ }_{\text {R }}^{\substack{\text { Risk } \\ \text { fator }}}$ |  |  | Post-Solution Segment Need | $\substack{\text { Raw } \\ \text { score }}$ | ${ }_{\substack{\text { Risk } \\ \text { fator }}}^{\text {ar }}$ |  | $\underbrace{\text { Exsiting Segnent }}$ Need | Post-Solution Segment Need | ${ }_{\substack{\text { Raw } \\ \text { score }}}^{\text {a }}$ | ${ }_{\substack{\text { Risk } \\ \text { fator }}}^{\text {a }}$ |  |  | Postsoution | Rawsore | ${ }_{\substack{\text { Risk } \\ \text { fator }}}^{\text {arem }}$ | $\underbrace{\text { a }}_{\substack{\text { Fatored } \\ \text { scored }}}$ | $\begin{gathered} \text { Exsting } \\ \substack{\text { Segnent } \\ \text { Need }} \\ \hline \end{gathered}$ | $\begin{array}{\|c\|} \hline \text { Post-Solution } \\ \text { Segment } \\ \text { Need } \\ \hline \end{array}$ |  | ${ }_{\substack{\text { Risk } \\ \text { fator }}}$ | fatored |  |
| 19.1 .18 |  | ${ }_{88-104}$ | 200.326 | 1.802 | 0.600 | 1.202 | 1.54 | 1.851 |  |  | 0.000 |  | 0.000 | 0.509 | ${ }^{0.357}$ | 0.152 | 0.21 | 0.03 | 0.562 | 0.562 | 0.000 | 0.00 | 0.00 | 0.002 | 0.002 | 0.000 | 0.00 | 0.00 | 1.883 |
| 70.2 |  | ${ }^{3365.539}$ | ${ }^{0.1}$ |  |  | 0.000 |  | 0.000 |  |  | 0.000 |  | 0.000 | ${ }^{0.243}$ | ${ }^{0.233}$ | 0.010 | 4.72 | ${ }^{0.05}$ | ${ }^{0.888}$ | 0.888 | 0.000 | 0.00 | 0.00 | 0.004 | 0.004 | 0.000 | 0.00 | 0.00 | 0.047 |
| 70.4 | Bylas to Peridot Safety Improvements | 27-292 | 15.116 |  |  | 0.000 |  | 0.000 |  |  | 0.000 |  | 0.000 | 5.129 | ${ }^{3} 410$ | 1.719 | 2.17 | 3.74 | 0.862 | 0.860 | 0.002 | 8.28 | 0.02 | 0.008 | 0.008 | 0.000 | 0.00 | 0.00 | ${ }^{3.753}$ |
| 70.5 | East of Globe Safety Improvements | 255.270 | ${ }^{31.067}$ |  |  | 0.000 |  | 0.000 |  |  | 0.000 |  | 0.000 | ${ }^{9.593}$ | 0.291 | 9.302 | 0.40 | ${ }^{3.73}$ | ${ }^{0.894}$ | 0.839 | 0.055 | ${ }_{5}^{5.67}$ | ${ }^{0.31}$ | 0.012 | 0.012 | 0.000 | 0.00 | 0.00 | 4.000 |
| ${ }^{70160.6}$ |  | ${ }^{243-255}$ | 22.62 |  |  | 0.000 |  | 0.000 |  |  | 0.000 |  | ${ }^{0.000}$ | 11.096 | 7.783 | ${ }_{3.313}$ | 3.03 | 10.03 | ${ }^{0.983}$ | ${ }^{0.946}$ | 0.037 | 6.15 | 0.23 | 1.178 | 0.642 | 0.536 | 4.22 | 2.26 | 12.518 |
| 70160.7A | $\underbrace{}_{\substack{\text { GIobe Area freight } \\ \text { Impovenens }}}$ | ${ }^{253.63}$ | 8.21 |  |  | 0.000 |  | 0.000 |  |  | 0.000 |  | ${ }^{0.000}$ | ${ }^{11.096}$ | ${ }^{11.095}$ | 0.001 | 3.03 | 0.00 | ${ }^{0.983}$ | 0.983 | 0.000 | 0.00 | 0.00 | 1.178 | 0.728 | 0.450 | 4.22 | 1.90 | 1.901 |
| 70160.78 |  | ${ }^{253.63}$ | 2.11 |  |  | 0.000 |  | 0.000 |  |  | 0.000 |  | ${ }^{0.000}$ | ${ }^{11.096}$ | 10.41 | 0.655 | 3.03 | 1.98 | ${ }^{0.983}$ | 0.970 | 0.013 | ${ }^{3.58}$ | 0.05 | 1.178 | 0.818 | 0.360 | 4.22 | 1.52 | ${ }^{3.548}$ |
| 60.8 | Miami Area West Safety | 228-277.3 | 17.04 |  |  | 0.000 |  | 0.000 |  |  | 0.000 |  | ${ }^{0.000}$ | ${ }^{6.380}$ | 2.802 | 3.578 | 3.85 | ${ }^{13.77}$ | ${ }^{10.054}$ | 10.901 | ${ }^{0.053}$ | ${ }^{9.65}$ | 0.51 | 4.875 | 4.007 | 0.868 | 7.48 | ${ }^{6.49}$ | 20.772 |
| ${ }^{60.9 A}$ | Miami Area Freight mprovemen | ${ }^{228.47}$ | 33.32 |  |  | 0.000 |  | 0.000 |  |  | 0.000 |  | ${ }^{0.000}$ | ${ }^{6.380}$ | ${ }^{6.380}$ | 0.000 | 3.85 | 0.00 | ${ }^{10.054}$ | 10.900 | ${ }^{0.054}$ | 0.00 | 0.00 | 4.875 | 4.368 | 0.507 | 7.48 | 3.79 | ${ }^{3.793}$ |
| ${ }^{60.98}$ | Miami Area Freight <br> Improvements | 228.47 | 1.939 |  |  | 0.000 |  | 0.000 |  |  | 0.000 |  | 0.000 | ${ }^{6.380}$ | ${ }_{5} 594$ | 0.436 | 3.85 | ${ }^{1.68}$ | 10.954 | 10.913 | 0.041 | 6.77 | 0.28 | 4.875 | 4.218 | 0.557 | 7.48 | 4.91 | 6.870 |
| 60.1 |  | ${ }^{212-223}$ | 0.448 |  |  | 0.000 |  | 0.000 |  |  | 0.000 |  | 0.000 | ${ }^{3.111}$ | 1.858 | 1.253 | 0.69 | ${ }^{0.87}$ | 0.431 | 0.426 | 0.005 | 5.53 | ${ }^{0.03}$ | 0.473 | ${ }^{0.303}$ | 0.170 | 4.34 | 0.74 | 1.637 |
| 60.11 | US-60 SW of Gold Canyon Safety Improvements mprovement | 208.206 | 3.932 |  |  |  |  |  |  |  |  |  |  | ${ }^{0.318}$ | 0.881 | 0.037 | 1.60 | 0.06 | 0.790 | 0.786 | 0.004 | 6.09 | 0.02 | 0.827 | 0.633 | 0.94 | 5.84 | ${ }^{1.13}$ | 1.216 |
| 60.12 | Gold Canyon Area Mobility and Safety Improvement | 205.199 | 44.03 |  |  |  |  |  |  |  |  |  |  | 1.023 | 0.812 | 0.211 | 3.03 | 0.64 | 4.421 | 1.233 | 3.188 | ${ }^{6.12}$ | 19.52 | 1.096 | 0.355 | 0.461 | 4.68 | 2.16 | 22.314 |
| 60.13 | Apache Junction Area Mobility and Fre Improvements | 199.194.3 | 24.68 |  |  |  |  |  |  |  |  |  |  | 2.515 | 2.440 | 0.075 | 1.69 | 0.13 | 7.502 | 2.736 | 4.766 | 3.19 | 15.22 | 0.634 | ${ }^{0.124}$ | 0.510 | 2.22 | ${ }^{1.13}$ | 16.476 |
| 60.14 | Apache Junction Area Safety Improvements | 199.194.3 | 0.283 |  |  |  |  |  |  |  |  |  |  | 2.515 | 2.038 | 0.477 | 1.69 | 0.81 | 7.502 | 7.993 | 0.009 | 279 | 0.03 | 0.634 | 0.994 | 0.140 | 2.22 | 0.31 | ${ }^{1.144}$ |

Emphasis Area Scoring

|  |  |  |  | Safery Emphasis Area |  |  |  |  |  | Mobility Emphasis Area |  |  |  |  |  | Freight Emphasis Area |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Candidate Solution \# | Candidate Solution Name | Milepost Location | Estimated Cost (\$ millions) | $\begin{array}{\|l\|l\|} \hline \text { Existing } \\ \text { Coritor } \\ \text { Need } \end{array}$ | $\begin{array}{\|l\|l} \substack{\text { Post } \\ \text { Soutron } \\ \text { Coritidor }} \end{array}$ | Raw Score | Risk Factor | Emphasis factor | $\begin{aligned} & \text { Factored } \\ & \text { Score } \end{aligned}$ | $\begin{array}{\|c\|c\|c\|c\|c\|c\|c\|c\|c\|c\|c\|c\|c\|c\|c\|c\|c\|c\|c\|c\|c\|} \hline \text { Need } \end{array}$ | Post-Solution <br> Corridor <br> Need | Raw Score | Risk factor | $\begin{aligned} & \text { Emphasis } \\ & \text { Factor } \\ & \hline \end{aligned}$ | Fratiore | $\begin{gathered} \text { Existing } \\ \text { Corridor Need } \end{gathered}$ | Post-5olution Corrido Need | Raw Score | Risk Factor | Emphasis Factor | Factored Score |
| 191.18 | $\begin{gathered} \text { Ussig1 Pavement } \\ \text { Presenations ountrit of } \\ \text { Safford } \end{gathered}$ | ${ }^{88-104}$ | 200.3 | 1.801 | 1.801 | 0.000 | 0.21 | 1.50 | 0.00 | 0.310 | 0.310 | 0.000 | 0.00 | 1.50 | 0.00 | 2.995 | 2.995 | 0.000 | 0.00 | 1.50 | 0.00 |
| 70.2 | East Safford Safety <br> Improvements | 336.5-339 | ${ }^{0.1}$ | 1.801 | 1.796 | 0.005 | 4.72 | 1.50 | 0.04 | 0.310 | 0.310 | 0.000 | 0.00 | 1.50 | 0.00 | 2.995 | 2.995 | 0.000 | 0.00 | 1.50 | 0.00 |
| 70.4 | Bylas to Peridot Safety Improvements | 274-292 | ${ }^{15.1}$ | 1.801 | 1.571 | 0.230 | 2.17 | 1.50 | 0.75 | 0.310 | 0.310 | 0.000 | ${ }^{8.28}$ | 1.50 | 0.00 | ${ }^{2.995}$ | 2.995 | 0.000 | 0.00 | 1.50 | 0.00 |
| 70.5 | East of Globe Safety Improvements | $255-270$ | ${ }^{31.1}$ | 1.801 | 0.890 | ${ }^{0.911}$ | 0.40 | 1.50 | 0.55 | 0.310 | ${ }^{0.307}$ | ${ }^{0.003}$ | 5.67 | 1.50 | 0.03 | 2.995 | 2.995 | 0.000 | 0.00 | 1.50 | 0.00 |
| 70160.6 | Globe Area Safety Improvements | 243-255 | 22.6 | 1.801 | 1.538 | ${ }^{0.263}$ | ${ }^{3.03}$ | 1.50 | 1.19 | 0.310 | 0.309 | 0.001 | 6.15 | 1.50 | 0.01 | 2.995 | 2.817 | 0.178 | 4.22 | 1.50 | 1.13 |
| 70160.7A | Globe Area Freight Improvements | ${ }^{253.63}$ | 8.2 | 1.801 | 1.801 | 0.000 | 3.03 | 1.50 | 0.00 | 0.310 | 0.310 | 0.000 | 0.00 | 1.50 | 0.00 | 2.995 | 2.994 | 0.001 | 4.22 | 1.50 | 0.01 |
| 70160.78 | Globe Area Freigh Improvements | ${ }^{253.63}$ | 2.1 | 1.801 | 1.801 | 0.000 | ${ }^{3.03}$ | 1.50 | 0.00 | 0.310 | 0.310 | 0.000 | 3.58 | 1.50 | 0.00 | 2.995 | 2.94 | 0.055 | 4.22 | 1.50 | 0.35 |
| 60.8 | Miami Area West Safety Improvements | 228-247.3 | 17.0 | 1.801 | 1.470 | 0.331 | 3.85 | ${ }^{1.43}$ | 1.82 | 0.310 | 0.310 | 0.000 | 9.65 | 1.50 | 0.00 | 2.995 | 2.902 | 0.093 | 7.48 | 1.50 | 1.04 |
| ${ }^{60.94}$ | Miami Area Freight Improvements | ${ }^{228.47}$ | ${ }^{33.3}$ | 1.801 | 1.801 | 0.000 | ${ }^{3.85}$ | 1.50 | 0.00 | ${ }^{0.310}$ | 0.310 | 0.000 | 0.00 | 1.50 | 0.00 | 2.995 | 2.993 | 0.002 | 7.48 | ${ }^{1.50}$ | 0.02 |
| 60.98 | Miami Area Freight Improvements | 228.47 | 1.9 | 1.801 | 1.801 | 0.000 | 3.85 | 1.50 | 0.00 | 0.310 | 0.310 | 0.000 | 6.77 | 1.50 | 0.00 | 2.995 | 2.925 | 0.070 | 7.48 | 1.50 | 0.79 |
| 60.1 | Superior Area Safety Improvements | ${ }^{212-223}$ | 0.4 | 1.801 | 1.727 | 0.074 | 0.69 | 1.50 | 0.08 | 0.310 | 0.310 | 0.000 | 5.53 | 1.50 | 0.00 | 2.995 | 2.970 | 0.025 | 4.34 | 1.50 | 0.16 |
| 60.11 | US-60 SW of Gold Canyon Safety mprovements | 208-206 | 3.9 | 1.801 | 1.789 | 0.012 | 1.60 | 1.50 | 0.03 | 0.310 | 0.310 | 0.000 | 6.09 | 1.50 | 0.00 | 2.995 | 2.986 | 0.009 | 5.84 | 1.50 | 0.08 |
| 60.12 | Gold Canyon Area <br> Mobility and Safety <br> Improvements | 205-199 | 44.0 | 1.801 | 1.773 | 0.028 | ${ }^{3.03}$ | 1.50 | 0.13 | 0.310 | ${ }^{0.301}$ | 0.009 | 6.12 | 1.50 | 0.08 | 2.995 | 2.974 | 0.021 | 4.68 | 1.50 | 0.15 |
| 60.13 | Apache Junction Area Mobility and Freight Improvements | 199-194.3 | 24.7 | 1.801 | 1.801 | 0.000 | 1.69 | 1.50 | 0.00 | 0.310 | 0.302 | 0.008 | 3.19 | 1.50 | 0.04 | 2.995 | 2.928 | 0.067 | 2.22 | 1.50 | 0.22 |
| 60.14 | Apache Junction Area Safety Improvements | 199-194.3 | 0.3 | 1.801 | 1.781 | 0.020 | 1.69 | 1.50 | 0.05 | 0.310 | 0.310 | 0.000 | 2.79 | 1.50 | 0.00 | 2.995 | 2.989 | 0.006 | 2.22 | 1.50 | 0.02 |

## Performance Effectiveness Scoring

| Candidate Solution \# | Candidate Solution Name | Milepost Location | Estimated Cost (\$ millions) | $\begin{gathered} \substack{\text { Tratolal } \\ \text { Fanered } \\ \text { Benefit }} \end{gathered}$ | VMT Factor | NPV Fator | Performance Effectiveness Score | miles | 2020 At | $\begin{gathered} \text { 1-way or 2- } \\ \text { way } \end{gathered}$ | vмт |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| ${ }^{191.18}$ | $\left\|\begin{array}{c} \text { Usi91 Pavement } \\ \text { Presenaion South of } \\ \text { Safford } \end{array}\right\|$ | ${ }^{88-104}$ | 200.3 | 1.88 | 2.21 | 20.2 | 0.4 | 16.00 | 2617 | twoway | 41872 |
| 70.2 | East Safford Safety Improvements | 336.5.339 | 0.1 | 0.08 | 0.17 | 15.3 | 2.1 | 0.21 | 11553 | two-way | 2426.13 |
| 70.4 | Bylas to Peridot Safety Improvements | 27-292 | 15.1 | 4.50 | 0.13 | 20.2 | 0.8 | 1.40 | 2749 | 1 | 1924.3 |
| 70.5 | East of Globe Safety Improvements | $255-270$ | ${ }^{31.1}$ | 4.61 | ${ }^{3.50}$ | 20.20 | 10.5 | 15.00 | 5790 | twoway | ${ }^{86850}$ |
| 70160.6 | Globe Area Safety Improvements | 243.255 | 22.6 | 14.85 | ${ }^{1.13}$ | 20.20 | 15.0 | 1.65 | ${ }^{11143}$ | two-way | 18885.95 |
| 70160.7A | Globe Area Freight <br> Improvements | ${ }^{253.63}$ | 8.2 | 1.91 | 0.37 | 20.20 | 1.7 | 0.50 | ${ }^{11143}$ | twoway | ${ }^{5571.5}$ |
| 70160.78 | Globe Area Freight Improvements | ${ }^{253.63}$ | ${ }^{2.1}$ | 3.90 | ${ }^{0.37}$ | ${ }^{20.20}$ | 13.9 | 0.50 | ${ }^{11143}$ | twoway | 5571.5 |
| 60.8 | Miami Area West Safety Improvements | 228-247.3 | 17.0 | 23.63 | 4.47 | 8.80 | 54.6 | 16.00 | 10128 | twoway | 162048 |
| ${ }^{60.9 A}$ | Miami Area Freight Improvements | ${ }^{228.47}$ | ${ }^{33} 3$ | ${ }^{3.82}$ | 0.34 | 20.20 | ${ }^{0.8}$ | 0.50 | 10128 | twoway | 5064 |
| ${ }^{60.98}$ | Miami Area Freight Improvements | 228.47 | 1.9 | 7.66 | 0.34 | 20.20 | 27.1 | 0.50 | 10128 | twoway | 5064 |
| 60.1 | Superior Area Safety Improvements | ${ }^{212-223}$ | ${ }^{0.4}$ | ${ }^{1.88}$ | 3.29 | 8.80 | 121.4 | 7.00 | 11048 | ${ }^{\text {two-way }}$ | 77336 |
| 60.11 | US-60 SW of Gold Canyon Safety Improvement | 208.206 | 3.9 | ${ }^{1.32}$ | 1.61 | 15.30 | 8.3 | 2.00 | 14003 | twoway | 2800 |
| 60.12 | Gold Canyon Area Mobility and Safet mprovement | 205-199 | 44.0 | 22.67 | 4.16 | 20.20 | 43.3 | 6.90 | 18653 | o-wav | 128705.7 |
| 60.13 | Apache Junction Area Mobility and Freight Improvements | 199.194.3 | 24.7 | 16.74 | 3.97 | 20.20 | 54.5 | 4.70 | 48507 | 1 | ${ }^{113991.45}$ |
| 60.14 | Apache Junction Area Safety Improvements | 199-194.3 | ${ }^{0.3}$ | ${ }^{1.21}$ | 4.19 | ${ }^{8.80}$ | 158.2 | 2.70 | 48507 | twoway | 139968.9 |

## Aロロт

## Appendix J: Solution Prioritization Scores

| Candidate Solution \# | Candidate SolutionName | Milepost | $\begin{aligned} & \text { Estimated Cost (\$ } \\ & \text { millions) } \end{aligned}$ | Pavement |  | Bridge |  | Safety |  | Mobility |  | Freight |  | $\begin{gathered} \text { Total } \\ \text { Factored } \\ \text { Score } \\ \hline \end{gathered}$ | Risk Factors |  |  |  |  | Weighted Risk | $\begin{gathered} \text { Segment } \\ \text { Need } \end{gathered}$ | Prioritization Score |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Score | \% | Score | \% | Score | \% | Score | \% | Score | \% |  | Pavement | Bridge | Safety | Mobility | Freight |  |  |  |
| 191.18 | US191 Pavement Preservation South of Safford | 88-104 | 200.326 | 1.851 | 98.3\% | 0.000 | 0.0\% | 0.032 | 1.7\% | 0.000 | 0.0\% | 0.000 | 0.0\% | 1.883 | 1.14 | 1.51 | 1.78 | 1.36 | 1.36 | 1.15 | 0.77 | 0 |
| 70.2 | East Safford Safety Improvements | 336.5-339 | ${ }^{0.1}$ | 0.000 | 0.0\% | 0.000 | 0.0\% | 0.083 | 100.0\% | 0.000 | 0.0\% | 0.000 | 0.0\% | ${ }^{0.083}$ | 1.14 | ${ }^{1.51}$ | 1.78 | ${ }^{1.36}$ | ${ }^{1.36}$ | 1.78 | 0.92 | 3 |
| 70.4 | Bylas to Peridot Safety Improvements | 274-292 | 15.116 | 0.000 | 0.0\% | 0.000 | 0.0\% | 4.486 | 99.6\% | 0.017 | 0.4\% | 0.000 | 0.0\% | 4.503 | 1.14 | 1.51 | 1.78 | ${ }^{1.36}$ | ${ }^{1.36}$ | 1.78 | 1.38 | 2 |
| 70.5 | East of Globe Safety Improvements | 255-270 | 31.067 | 0.000 | 0.0\% | 0.000 | 0.0\% | 4.276 | 92.7\% | 0.337 | 7.3\% | 0.000 | 0.0\% | 4.614 | 1.14 | 1.51 | 1.78 | 1.36 | 1.36 | 1.75 | 1.23 | 23 |
| 70160.6 | Globe Area Safety Improvements | 243-255 | 22.62 | 0.000 | 0.0\% | 0.000 | 0.0\% | 11.225 | 75.6\% | 0.237 | 1.6\% | 3.386 | 22.8\% | 14.848 | 1.14 | 1.51 | 1.78 | 1.36 | 1.36 | 1.68 | 1.77 | 44 |
| 70160.7A | Globe Area Freight Improvements | 253.63 | 8.214 | 0.000 | 0.0\% | 0.000 | 0.0\% | 0.003 | 0.2\% | 0.000 | 0.0\% | 1.904 | 99.8\% | 1.907 | 1.14 | 1.51 | 1.78 | 1.36 | 1.36 | 1.36 | 1.77 | 4 |
| 70160.78 | $\underset{\substack{\text { Globe Area Freight } \\ \text { Improvements }}}{ }$ | 253.63 | 2.11 | 0.000 | 0.0\% | 0.000 | 0.0\% | 1.983 | 50.9\% | 0.047 | 1.2\% | 1.866 | 47.9\% | 3.896 | 1.14 | 1.51 | 1.78 | 1.36 | 1.36 | 1.57 | 1.77 | 39 |
| 60.8 | Miami Area West Safety Improvements | 228-247.3 | 17.043 | 0.000 | 0.0\% | 0.000 | 0.0\% | 15.585 | 65.9\% | 0.511 | 2.2\% | 7.537 | 31.9\% | ${ }^{23.634}$ | 1.14 | 1.51 | 1.78 | 1.36 | 1.36 | 1.64 | 2.54 | 227 |
| 60.9 A | $\underset{\substack{\text { Miami Area Freight } \\ \text { Improvements }}}{ }$ | 228.47 | 33.322 | 0.000 | 0.0\% | 0.000 | 0.0\% | 0.000 | 0.0\% | 0.000 | 0.0\% | 3.815 | 100.0\% | 3.815 | 1.14 | 1.51 | 1.78 | 1.36 | 1.36 | 1.36 | 2.54 | 3 |
| 60.98 | Miami Area Freight Improvements | 228.47 | 1.939 | 0.000 | 0.0\% | 0.000 | 0.0\% | 1.678 | 21.9\% | 0.278 | 3.6\% | 5.700 | 74.5\% | 7.656 | 1.14 | 1.51 | 1.78 | 1.36 | 1.36 | 1.45 | 2.54 | 100 |
| 60.1 | Superior Area Safety Improvements | ${ }^{212-223}$ | 0.448 | 0.000 | 0.0\% | 0.000 | 0.0\% | 0.948 | 50.5\% | 0.028 | 1.5\% | 0.901 | 48.0\% | 1.877 | 1.14 | ${ }^{1.51}$ | 1.78 | ${ }^{1.36}$ | ${ }^{1.36}$ | 1.57 | 1.00 | 191 |
| 60.11 | US-60 SW of Gold Canyon Safety Improvements | 208-206 | 3.93 | 0.000 | 0.0\% | 0.000 | 0.0\% | 0.088 | 6.7\% | 0.024 | 1.8\% | 1.211 | 91.5\% | 1.324 | 1.14 | 1.51 | 1.78 | 1.36 | 1.36 | 1.39 | 1.15 | 13 |
| 60.12 | Gold Canyon Area Mobility and Safety Improvement | 205-199 | 44.003 | 0.000 | 0.0\% | 0.000 | 0.0\% | 0.766 | 3.4\% | 19.602 | 86.5\% | 2.303 | 10.2\% | 22.671 | 1.14 | 1.51 | 1.78 | 1.36 | 1.36 | 1.37 | 1.69 | 101 |
| 60.13 | Apache Junction Area Mobility and Freight Improvements | 199-194.3 | 24.675 | 0.000 | 0.0\% | 0.000 | 0.0\% | 0.127 | 0.8\% | 15.255 | 91.1\% | 1.356 | 8.1\% | 16.738 | 1.14 | 1.51 | 1.78 | 1.36 | 1.36 | 1.36 | 1.38 | 102 |
| 60.14 | Apache Junction Area Safety Improvements | 199-194.3 | 0.283 | 0.000 | 0.0\% | 0.000 | 0.0\% | 0.858 | 70.7\% | 0.025 | 2.1\% | 0.331 | 27.3\% | 1.214 | 1.14 | ${ }^{1.51}$ | 1.78 | ${ }^{1.36}$ | ${ }^{1.36}$ | 1.66 | 1.38 | 362 |

## AロロT

Appendix K: Preliminary Scoping Reports for Prioritized Solutions

## Аロロт

PRELIMINARY SCOPING REPORT
GENERAL PROJECT INFORMATION

| Date：01－24－18 | ADOT Project Manager： |
| :---: | :---: |
| Project Name：Huachuca City Area Safety Improvements－Options A and B（CS90．1） |  |
| City／Town：Huachuca City | County：Cochise |
| COG／MPO：SEAGO and SVMPO | ADOT District：Southcentral |
| Primary Route／Street：SR 90 |  |
| Beginning Limit：MP 313 |  |
| End Limit：MP 317 |  |
| Project Length： 4 miles |  |
| Right－of－Way Ownership（s）（wh City／Town； $\square$ County：$\boxtimes_{A}$ | uction would occur）：（Check all that apply） $\square$ Tribal； $\square$ Other： |
| Adjacent Land Ownership（s）；（Ch <br> $\square$ city／Town；$\square$ county；$\square$ A <br> http：／／gis．azland．gov／webopps／parcel／ | al； $\square$ Tribal； $\square$ other： |


| LOCAL PUBLIC AGENCY（LPA）or TRIBAL GOVERNMENT INFORMATION （If applicable） |  |  |
| :---: | :---: | :---: |
| LPA／Tribal Name： |  |  |
| LPA／Tribal Contact： |  |  |
| Email Address： | Phone |  |
| Administration：$\square$ ADOT Administered | $\square$ Self－Administered | $\square$ Certification Acceptance |


| PROJECT NEED |
| :--- |
| Safety Need：Crash hot spots at MP 313－315 and MP 316－317 |
|  |
|  |

[^4]
## Аロロт

## PRELIMINARY SCOPING REPORT

| PROJECT RISKS |  |
| :--- | :--- |
| Check any risks identified that may impact the project＇s scope，schedule，or budget： |  |
| $\square$ Access／Traffic Control／Detour Issues | $\square$ Right－of－Way |
| $\square$ Constructability／Construction Window Issues | $\square$ Environmental |
| $\square$ stakeholder Issues | $\square$ Utilities |
| $\square$ structures \＆Geotech | $\square$ Other： |
| Risk Description：Will need to coordinate with adjacent properties regarding access management |  |
|  |  |
|  |  |
|  |  |
|  |  |


| POTENTIAL FUNDING SOURCE（S） |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Anticipated Project Design／Construction Funding Type：（Check all that apply） |  | $\square$ stBg | $\square$ | TAP | $\square$ HSIP | $\square$ state |
|  |  | $\square$ Local | $\square$ | Private | $\square$ Tribal | $\square$ other： |
| COST ESTIMATE |  |  |  |  |  |  |
| Preliminary | Design | Right－of－Way <br> \＄0（Option A） <br> $\$ 0$（Option B） | $\begin{aligned} & \text { Construction } \\ & \$ 810,000 \text { (Option A) } \\ & \$ 7,173,000 \text { (Option B) } \end{aligned}$ |  |  | Total <br> \＄916，000（Option A） $\$ 8,105,000$（Option B） |
| Engineering | \＄81，000（Option A） |  |  |  |  |  |
| \＄25，000（Option A） | \＄717，000（Option B） |  |  |  |  |  |
| \＄215，000（Option B） |  |  |  |  |  |  |


| RECOMMENDED PROJECT DELIVERY |  |  |
| :--- | :--- | :--- |
| Delivery：$\square$ Design－Bid－Build | $\square$ Design－Build $\quad \square$ other： |  |
| Design Program Year： FY |  |  |
| Construction Program Year： FY |  |  |

ATTACHMENTS
1）State Location Ma
3）Project Scope of Work

ATTACHMENT 1 - STATE LOCATION MAP


## ATTACHMENT 2 - PROJECT VICINITY MAP



## ATTACHMENT 3 - SCOPE OF WORK

| SCOPE OF WORK |  |  |  |  |  |  |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: |
| Option A: |  |  |  |  |  |  |
| - Install raised median, MP 313-314 |  |  |  |  |  |  |
| - Install centerline rumble strips, MP 314-317 |  |  |  |  |  |  |
| Option B: |  |  |  |  |  |  |
| - Install raised median, MP 313-314 |  |  |  |  |  |  |
| - Widen roadway to install raised median, MP 314-317 |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |

SCOPE ITEMS CONSIDERED, BUT NOT INCLUDED

Pursuant to 23 USC 409: Notwithstanding any other provision of law, reports, surveys, schedules, lists, or data compiled or collected for the purpose of identitying, evaluating, or planning the safety enhancement of poteniar accident sites, hazardous roadway conditions, or rail-way-highway crossings, pursuant to sections 130,144 , and 148 (152] of this title or for the
purpose of developing any highway safety construction improvement project which may be implemented utilizing Federa-aid purpose of developing any highway safety construction improvement project which may be implemented utilizing Federal-aid
highway funds shall not be subject to discovery or admitted into evidence in a Federal or State court proceeding or highway funds shall not be subject to. discovery or admitted into evidence in a Feederal or State court proceeding or
considered for other purposes in any action for damages arising from any occurrence at a location mentioned or addressed in such reports, surveys, schedules, lists, or data.

## Аロロт

PRELIMINARY SCOPING REPORT


| LOCAL PUBLIC AGENCY（LPA）or TRIBAL GOVERNMENT INFORMATION （If applicable） |  |  |
| :---: | :---: | :---: |
| LPA／Tribal Name： |  |  |
| LPA／Tribal Contact： |  |  |
| Email Address： | Phone |  |
| Administration：$\square$ ADOT Administered | $\square$ Self－Administered | $\square$ Certification Acceptance |


| PROJECT NEED |
| :--- |
| Safety Need：Crash hot spot at MP 319－323 |
| Freight Need：High level of need based on the overall Freight Index and SB／EB Directional TPTI ratings |

## PROJECT PURPOSE

[^5]
## AロロT

PRELIMINARY SCOPING REPORT

| PROJECT RISKS |  |
| :--- | :--- |
| Check any risks identified that may impact the project＇s scope，schedule，or budget： |  |
| $\boxtimes$ Access／Traffic Control／Detour Issues | $\square$ Right－of－Way |
| $\square$ constructability／Construction Window Issues | $\square$ Environmental |
| $\square$ stakeholder Issues | $\square$ Utilities |
| $\square$ structures \＆Geotech | $\square$ other： |
| Risk Description：Will need to coordinate with adjacent properties regarding access management |  |

finding $\quad \square$ STBG $\quad \square$ TAP

| POTENTIAL FUNDING SOURCE（S） |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- |
| Anticipated Project Design／Construction Funding <br> Type：（Check all that apply） | $\square$ sTBG | $\square$ TAP | $\square$ HSIP | $\square$ state |  |
|  | $\square$ Local | $\square$ Private | $\square$ Tribal | $\square$ other： |  |


| COST ESTIMATE |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- |
| Preliminary <br> Engineering <br> $\$ 78,000$ | Design <br> $\$ 260,000$ | Right－of－Way <br> $\$ 0$ | Construction <br> $\$ 2,581,000$ | Total <br> $\$ 2,919,000$ |


| RECOMMENDED PROJECT DELIVERY |  |  |  |
| :--- | :--- | :---: | :---: |
| Delivery：$\square$ Design－Bid－Build $\quad \square$ Design－Build $\quad \square$ other； |  |  |  |
| Design Program Year： FY |  |  |  |
| Construction Program Year： FY |  |  |  |

1）State Location Ma
2）Project Vicinity Map

ATTACHMENT 1 - STATE LOCATION MAP


ATTACHMENT 2 - PROJECT VICINITY MAP


## ATTACHMENT 3 - SCOPE OF WORK

- Implement signal coordination for 3 signals from Hatfield St/Buffalo Soldier Trail intersection (MP 317.2) to Coronado Implement signal coordination for 3 signals from Hattield St/Buffalo Soldier Trail intersection
Dr (MP 319.6), and for 6 signals from Campus Dr (MP 321.0) to Colonia De Salud (MP 323.0)
Install speed feedback and signal ahead signs, MP 318 EB and MP 320 WB
Install ispeed feedback and signal ahead signs, MP
Install centerline rumble strips, MP 317.2-320.8
Construct raised median, MP 321.5-323.7

SCOPE ITEMS CONSIDERED, BUT NOT INCLUDED

Pursuant to 23 USC 409: Notwithstanding any other provision of law, reports, surveys, schedules, lists, or data compiled or collected for the purpose of identifyying, evaluating, or planning the safety enhancement of potential accident sites, hazardous oadway conditions, or rail-way-highway crossings, pursuant to sections 130,144 , and 148 [152] of this title or for the highway funds shall not be subject to discovery or admitted into evidence in a Federal or State court proceeding or considered for other purposes in any action for damages arising from any occurrence at a location mentioned or addressed
in such reports, surveys, schedules, lists, or data.

## Аロロт

## PRELIMINARY SCOPING REPORT

| GENERAL PROJECT INFORMATION |  |
| :---: | :---: |
| Date：01－24－18 | ADOT Project Manager： |
| Project Name：San Pedro River Area Safety Improvement（CS90．3） |  |
| City／Town：－ | County：Cochise |
| COG／MPO：SVMPO and SEAGO | ADOT District：Southcentral |
| Primary Route／Street：SR 90 |  |
| Beginning Limit：MP 324 |  |
| End Limit：MP 336 |  |
| Project Length： $\mathbf{1 2}$ miles |  |
| Right－of－Way Ownership（s）（where proposed project construction would occur）：（Check all that apply）$\square$ City／Town； $\square$ County；$\triangle$ ADOT； $\square$ Private； $\square$ Federal； $\square$ Tribal； $\square$ Other： |  |
| Adjacent Land Ownership（s）；（Ch <br> $\square$ city／Town；$\square$ county；$\square$ <br> http：／／gis．azland．gov／webopps／（parcel／ | $\square$ Tribal；$\boxtimes$ other：State Trust |


| LOCAL PUBLIC AGENCY（LPA）or TRIBAL GOVERNMENT INFORMATION （If applicable） |  |  |
| :---: | :---: | :---: |
| LPA／Tribal Name： |  |  |
| LPA／Tribal Contact： |  |  |
| Email Address： | Phone |  |
| Administration：$\square$ ADOT Administered | $\square$ Self－Administered | $\square$ Certification Acceptance |


| PROJECT NEED |
| :--- | :--- |
| Safety Need：Safety Index and NB／WB Directional Safety Index above the statewide average，MP 324－336 |

[^6]
## Аロロт

## PRELIMINARY SCOPING REPORT

| PROJECT RISKS |  |
| :--- | :--- |
| Check any risks identified that may impact the project＇s scope，schedule，or budget： |  |
| $\square$ Access／Traffic Control／Detour Issues | $\square$ Right－of－Way |
| $\square$ constructability／Construction Window Issues | $\square$ Environmental |
| $\square$ stakeholder Issues | $\square$ Utilities |
| $\square$ structures \＆Geotech | $\square$ other： |
| Risk Description： |  |
|  |  |
|  |  |
|  |  |
|  |  |


| POTENTIAL FUNDING SOURCE（S） |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Anticipated Project Design／Construction Funding Type：（Check all that apply） | $\square$ STBG | $\square$ TAP | $\square$ HSIP | $\square$ state |
|  | $\square$ Local | $\square$ Private | $\square$ Tribal | $\square$ other： |
| COST ESTIMATE |  |  |  |  |
|   <br> Preliminary <br> Engineering Design <br> $\$ 260,000$ $\$ 867,000$ | $\begin{aligned} & \text { Right-of-Way } \\ & \$ 0 \end{aligned}$ | $\begin{aligned} & \text { Construct } \\ & \$ 8,675,00 \end{aligned}$ |  | Total \＄9，802，000 |
| RECOMMENDED PROJECT DELIVERY |  |  |  |  |
| Delivery：$\square$ Design－Bid－Build $\quad \square$ Design－Build $\quad \square$ Other： |  |  |  |  |
| Design Program Year：FY |  |  |  |  |
| Construction Program Year：FY |  |  |  |  |
| ATTACHMENTS |  |  |  |  |
| 1）State Location Map <br> 2）Project Vicinity Map <br> 3）Project Scope of Work |  |  |  |  |

ATTACHMENT 1 - STATE LOCATION MAP


## ATTACHMENT 2 - PROJECT VICINITY MAP



## ATTACHMENT 3 - SCOPE OF WORK

| SCOPE OF WORK |
| :--- | :--- |
| - Widen shoulders to 8 feet in both directions (striping, delineators, RPMs, safety edge, and rumble strips for both |
| shoulders), MP 324-336 |
| Install centerline rumble strips, MP 324-336 |


| SCOPE ITEMS CONSIDERED, BUT NOT INCLUDED |
| :--- |
|  |
|  |

Pursuant to 23 USC 409: Notwithstanding any other provision of law, reports, surveys, schedules, lists, or data compiled or collected for the purpose of identifying, evaluating, or planning the safety enhancement of potential accident sites, hazardous rail-way-highway crossings, pursuant to se. ns 130, 144, and 148 [152] of this title or for the highway funds shall not be subject to discovery or aditted into eldence in a Federal or State court proceeding or considered for other purposes in any action for damages arising from any occurrence al a location mentioned or addressed in such reports, surveys, schedules, lists, or data.

## Аロロт

## PRELIMINARY SCOPING REPORT

GENERAL PROJECT INFORMATION

| GENERAL PROJECT INFORMATION |  |
| :---: | :---: |
| Date：01－24－18 | ADOT Project Manager： |
| Project Name：Banning Creek Area Climbing Lane（CS80．4） |  |
| City／Town：－ | County：Cochise |
| COG／MPO：SEAGO | ADOT District：Southeast |
| Primary Route／Street：SR 80 |  |
| Beginning Limit：MP 336 |  |
| End Limit：MP 338 |  |
| Project Length： $\mathbf{2}$ miles |  |
| Right－of－Way Ownership（s）（where proposed project construction would occur）：（Check all that apply） <br> $\square$ city／Town；$\square$ $\square$ County； $\mathbb{Q}$ ADOT；$\square$ $\square$ Private； $\square$ Federal； $\square$ Tribal； $\square$ Other： |  |
| Adjacent Land Own $\square$ City／Town； $\square$ http：／／gis．azland．gov／w | $\square$ Tribal；$\boxtimes$ other：State Trust |


| LOCAL PUBLIC AGENCY（LPA）or TRIBAL GOVERNMENT INFORMATION （If applicable） |  |  |
| :---: | :---: | :---: |
| LPA／Tribal Name： |  |  |
| LPA／Tribal Contact： |  |  |
| Email Address： | Phone |  |
| Administration：$\square$ ADOT Administered | $\square$ Self－Administered | $\square$ Certification Acceptance |

PROJECT NEED

| PROJECT PURPOSE |  |  |  |
| :--- | :--- | :--- | :--- |
| What is the Primary Purpose of the Project？ | Preservation | Modernization 区 | Expansion $\square$ |
| Address identified Freight Need by constructing a climbing lane |  |  |  |
|  |  |  |  |
|  |  |  |  |
|  |  |  |  |
|  |  |  |  |
|  |  |  |  |

## Аロロт

## PRELIMINARY SCOPING REPORT

| PROJECT RISKS |  |
| :---: | :---: |
| Check any risks identified that may impact the project＇s scope，schedule，or budget： |  |
| $\square$ Access／Traffic Control／Detour Issues | $\square$ Right－of－Way |
| $\square$ Constructability／Construction Window Issues | $\square$ Environmental |
| $\square$ stakeholder Issues | $\square$ Utilities |
| $\square$ structures \＆Geotech | Other：Potential rock cuts |
| Risk Description：Could potentially require rock cuts |  |


| POTENTIAL FUNDING SOURCE（S） |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :---: |
| Anticipated Project Design／Construction Funding <br> Type：（Check all that apply） | $\square$ sTBG | $\square$ TAP | $\square$ HSIP | $\square$ state |  |
|  | $\square$ Local | $\square$ Private | $\square$ Tribal | $\square$ other： |  |


| COST ESTIMATE |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- |
| Preliminary <br> Engineering <br> $\$ 193,000$ | Design <br> $\$ 644,000$ | Right－of－Way <br> $\$ 0$ | Construction <br> $\$ 6,435,000$ | Total <br> $\$ 7,272,000$ |


| RECOMMENDED PROJECT DELIVERY |  |  |  |
| :--- | :--- | :---: | :---: |
| Delivery：$\square$ Design－Bid－Build | $\square$ Design－Build $\quad \square$ Other： |  |  |
| Design Program Year：FY |  |  |  |
| Construction Program Year：FY |  |  |  |

ATTACHMENTS
1）State Location Map
3）Project Scope of Work

ATTACHMENT 1 - STATE LOCATION MAP


ATTACHMENT 2 - PROJECT VICINITY MAP


## ATTACHMENT 3-SCOPE OF WORK



Pursuant to 23 USC 409: Notwithstanding any other provision of law, reports, surveys, schedules, lists, or data compiled or collected for the purpose of identifying, evaluating, or planning the safety enhancement of potential accident sites, hazardous oadway conditions, or rail-way-highway crossings, pursuant to sections 130, 144, and 148 [152] of this stitle or for the purpose of developing any highway safety construction improvement project which may be io court proceeding or ighway runds shall not be subject to discovery or admitted into evidence in a Federal or state court proceeding or
considered for other purposes in any action for damages arising from any occurrence at a location mentioned or addressed in such reports, surveys, schedules, lists, or data.

## Аロロт

## PRELIMINARY SCOPING REPORT

| GENERAL PROJECT INFORMATION |  |
| :---: | :---: |
| Date：01－24－18 | ADOT Project Manager： |
| Project Name：Banning Creek Area Freight Improvements（CS80．5） |  |
| City／Town：－ | County：Cochise |
| COG／MPO：SEAGO | ADOT District：Southeast |
| Primary Route／Street：SR 80 |  |
| Beginning Limit：MP 333 |  |
| End Limit：MP 339 |  |
| Project Length： 6 miles |  |
| Right－of－Way Ownership（s）（where proposed project construction would occur）：（Check all that apply）City／Town； $\square$ County；$\boxtimes$ ADOT；$\square$ $\square$ Private； $\square$ Federal； $\square$ Tribal； $\square$ Other： |  |
| Adjacent Land Ow City／Town $\square$ http：／／gis．azland．gov／ | $\text { al; } \square \text { Tribal; } \triangle \text { other: State Trust }$ |


| LOCAL PUBLIC AGENCY（LPA）or TRIBAL GOVERNMENT INFORMATION （If applicable） |  |  |
| :---: | :---: | :---: |
| LPA／Tribal Name： |  |  |
| LPA／Tribal Contact： |  |  |
| Email Address： | Phone |  |
| Administration：$\square$ ADOT Administered | $\square$ Self－Administered | $\square$ Certification Acceptance |

PROJECT NEED | Freight Need：High level of need based on the overall Freight Index and SB／EB Directional TPTI ratings |
| :--- |
|  |

[^7]
## AロロT

PRELIMINARY SCOPING REPORT

| PROJECT RISKS |  |  |  |
| :--- | :---: | :---: | :---: |
| Check any risks identified that may impact the project＇s scope，schedule，or budget： |  |  |  |
| $\square$ Access／Traffic Control／Detour Issues |  |  |  |
| $\square$ Right－of－Way |  |  |  |

$\square$ Constructability／Construction Window Issues $\quad \square$ Environmental
$\square$ stakeholder Issues $\square$ Utilities
$\square$ Structures \＆Geotech $\quad$ other：Potential rock cuts
Risk Description：Could potentially require rock cuts

| POTENTIAL FUNDING SOURCE（S） |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Anticipated Project Design／Construction Funding Type：（Check all that apply） |  | $\square$ STBG | $\square$ | TAP | $\square$ HSIP | $\square$ state |
|  |  | $\square$ Local | $\square$ | Private | $\square$ Tribal | $\square$ Other： |
| COST ESTIMATE |  |  |  |  |  |  |
| Preliminary Engineering \＄106，000 | $\begin{array}{\|l\|} \hline \text { Design } \\ \$ 355,000 \end{array}$ | $\begin{aligned} & \text { Right-of-Way } \\ & \$ 0 \end{aligned}$ |  | Construc |  | $\begin{array}{\|l\|} \hline \text { Total } \\ \$ 4,009,000 \end{array}$ |


| RECOMMENDED PROJECT DELIVERY |  |  |
| :--- | :--- | :--- |
| Delivery：$\square$ Design－Bid－Build | $\square$ Design－Build $\quad \square$ other： |  |
| Design Program Year： FY |  |  |
| Construction Program Year： FY |  |  |

ATTACHMENTS
1）State Location Map
3）Project Scope of Work

ATTACHMENT 1 - STATE LOCATION MAP


ATTACHMENT 2 - PROJECT VICINITY MAP


## ATTACHMENT 3 - SCOPE OF WORK

| SCOPE OF WORK |
| :--- |
| - Widen shoulders to 8 feet in both directions (striping, delineators, RPMs, safety edge, and rumble strips for both |
| shoulders), MP 333 -339 |


| SCOPE ITEMS CONSIDERED, BUT NOT INCLUDED |
| :---: |
|  |
|  |

Pursuant to 23 USC 409: Notwithstanding any other provision of law, reports, surveys, schedules, lists, or data compiled or collected for the purpose of identifying, evaluating, or M.wing the safety em 144 ant 1481152 or this roadway conditions, or rall-way-highway crossings, pursuant to sections 130,144 , and 148 (152] of this title or for the highway funds shall hot be subject to do discovery or admitted into evidence in a Federal or state court proceeding or considered for other purposes in any action for damages arising from any occurrence at a location mentioned or addresse considered for other purposes in any action for dar

## Аロロт

## PRELIMINARY SCOPING REPORT

GENERAL PROJECT INFORMATION

| GENERAL PROJECT INFORMATION |  |
| :---: | :---: |
| Date：01－24－18 | ADOT Project Manager： |
| Project Name：Mule Gulch Area Freight Improvements（CS80．6） |  |
| City／Town：－ | County：Cochise |
| COG／MPO：SEAGO | ADOT District：Southeast |
| Primary Route／Street：SR 80 |  |
| Beginning Limit：MP 345 |  |
| End Limit：MP 348 |  |
| Project Length： $\mathbf{3}$ miles |  |
| Right－of－Way Ownership（s）（where proposed project construction would occur）：（Check all that apply） $\square$ city／Town；$\square$$\square$ County：$\triangle$ ADOT；$\square$ $\square$ Private； $\square$ Federal； $\square$ Tribal； $\square$ Other： |  |
| Adjacent land Ow City／Town $\square$ <br> http：／／ais．azland．gov／ | $\square$ Tribal； $\square$ Other： |


| LOCAL PUBLIC AGENCY（LPA）or TRIBAL GOVERNMENT INFORMATION （If applicable） |  |  |
| :---: | :---: | :---: |
| LPA／Tribal Name： |  |  |
| LPA／Tribal Contact： |  |  |
| Email Address： | Phone |  |
| Administration：$\square$ ADOT Administered | $\square$ Self－Administered | $\square$ Certification Acceptance |

PROJECT NEED

| PROJECT PURPOSE |  |  |  |
| :--- | :--- | :--- | :--- |
| What is the Primary＇Purpose of the Project？ | Preservation $\square$ | Modernization $\boxtimes$ | Expansion $\square$ |
| Address Freight Need by constructing passing lanes |  |  |  |

## Аロロт

## PRELIMINARY SCOPING REPORT

| PROIECT RISKS |  |
| :--- | :--- |
| Check any risks identified that may impact the project＇s scope，schedule，or budget： |  |
| $\square$ Access／Traffic Control／Detour Issues | $\square$ Right－of－Way |
| $\square$ Constructability／Construction Window Issues | $\square$ Environmental |
| $\square$ stakeholder Issues | $\square$ Utilities |
| $\square$ structures \＆Geotech | $\square$ other： |
| Risk Description： |  |
|  |  |
|  |  |
|  |  |
|  |  |


| POTENTIAL FUNDING SOURCE（S） |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Anticipated Project Design／Construction Funding Type：（Check all that apply） | $\square$ STBG | $\square$ TAP | $\square \mathrm{HSIP}$ | $\square$ state |
|  | $\square$ Local | $\square$ Private | $\square$ Triba | $\square$ other： |
| COST ESTIMATE |  |  |  |  |
|   <br> Preliminary <br> Engineering Design <br> $\$ 119,000$ $\$ 396,000$ | $\begin{aligned} & \text { Right-of-Way } \\ & \$ 0 \end{aligned}$ | $\begin{aligned} & \text { Construct } \\ & \$ 3,960,00 \end{aligned}$ |  | Total $\$ 4,475,000$ |
| RECOMMENDED PROJECT DELIVERY |  |  |  |  |
| Delivery：$\square$ Design－Bid－Build $\quad \square$ Design－Build $\quad \square$ Other： |  |  |  |  |
| Design Program Year：FY |  |  |  |  |
| Construction Program Year：FY |  |  |  |  |
| ATTACHMENTS |  |  |  |  |
| 1）State Location Map <br> 2）Project Vicinity Map <br> 3）Project Scope of Work |  |  |  |  |

ATTACHMENT 1 - STATE LOCATION MAP


## ATTACHMENT 2 - PROJECT VICINITY MAP



## ATTACHMENT 3-SCOPE OF WORK



Pursuant to 23 USC 409: Notwithstanding any other provision of law, reports, surveys, schedules, lists, or data compiled or collected for the purpose of identifying, evaluating, or planning the safety enhancement of potential accident sites, hazardous roadway conditions, or rail-way-highway crossings, ponsintion purpose of developing any highway safety construction improvement project which may be implemented uttlizing Federal-aid highway funds shall not be subject to discovery or admitted into evidence in a Federal or State court proceeding or
considered for other purposes in any action for damages ansing from any occurrence at a location mentioned or addressed in such reports, surveys, schedules, lists, or data.


[^0]:    ${ }^{1}$ Urban or Fringe Urban Operating Environment
    ${ }^{2}$ Rural Operating Environment
    $\wedge$ Uninterrupted

    * Interrupted

[^1]:    *'-' indicates only one solution is being proposed and no option

[^2]:    * '-' indicates only one solution is being proposed and no option

[^3]:    ${ }^{1}$ HERS Support - 2011, Task 6: Procedures for Estimating Highway Capacity, draft Technical Memorandum. Cambridge Systematics. Prepared for the Federal Highway Administration. March 2013.

[^4]:    ## PROJECT PURPOS

    | What is the Primary Purpose of the Project？ | Preservation $\square$ | Modernization $\boxtimes$ | Expansion $\square$ |
    | :--- | :--- | :--- | :--- | Address identified Safety Need by installing a raised median barrier（MP 313－314）and by either installing centerline rumble strips or widening the roadway and installing a raised median（MP 314－317）

[^5]:    | What is the Primary Purpose of the Project？ | Preservation $\square$ |
    | :--- | :--- | and signal ahead signs，installing centerline rumble strips，and constructing a raised median

[^6]:    ## PROJECT PURPOSE

    | What is the Primary Purpose of the Project？ | Preservation $\square$ |
    | :--- | :--- |

    Modernization 区
    Expansion $\square$
    Address identified Safety Need by widening shoulders to 8 feet in both directions and installing centerline rumble strips

[^7]:    

    |  |  | Address identified Freight Need by widening shoulders to 8 feet in both directions |
    | :--- | :--- | :--- | :--- |

